

**APPENDIX T**



**Miner Flat Dam Left Abutment Ridge Seepage Analysis,  
April 1997**

**GOLDER ASSOCIATES,  
VOLUMES 1 THRU II OF II**

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**Golder Associates Inc.**

200 Union Boulevard, Suite 500  
Lakewood, CO USA 80228  
Telephone (303) 980-0540  
Fax (303) 985-2080



**MINER FLAT DAM  
LEFT ABUTMENT RIDGE  
SEEPAGE ANALYSIS**

***VOLUME I OF II***

*Prepared for:*

*Morrison-Maierle/CSSA  
910 Helena Avenue  
Helena, Montana*

*Prepared by:*

*Golder Associates Inc.  
200 Union Blvd., Suite 500  
Lakewood, Colorado 80228*

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**ACRONYMS AND ABBREVIATIONS**

ASTM	American Society for Testing and Materials
cfs	cubic feet per second
Golder	Golder Associates Inc.
gph	gallons per hour
gpm	gallons per minute
ISRM	International Society for Rock Mechanics
K	Hydraulic conductivity
MM/CSSA	Morrison-Maierle/CSSA
psi	pounds per square inch
RQD	rock quality designation
USCS	Unified Soil Classification System

## 1.0 INTRODUCTION

This report presents the preliminary results of the Miner Flat dam - Phase I exploration program performed in 1995. This investigation was performed in accordance with the scope of work approved by Morrison-Maierle (MM/CSSA) in 1994 under an Agreement with Golder Associates Inc. dated April 28, 1994. The field investigation included core drilling, logging and testing of core, percussion hammer drilling and logging, packer testing, and installation of piezometers in order to estimate the seepage through the left abutment ridge of the proposed Miner Flat dam. Seepage through the left abutment was numerically modeled using data from this and previous geotechnical investigations. The estimated loss from seepage is approximately 0.13 cubic feet per second.

The proposed Miner Flat dam is located on the North Fork of the White River east of Arizona State Highway 73 at Mile Post 350, about 8 miles south of McNary and about 11 miles north of Whiteriver, on the Fort Apache Indian Reservation. The Fort Apache Indian Reservation comprises about 1,700,000 acres that are south of the Mogollon Rim and north of the Salt River in north-central Arizona (Figure 1). The Phase I exploration investigation was preceded by three geotechnical investigations performed by Mineral Systems, Inc. in 1981, 1982, and in late 1985 and early 1986. The purpose of the 1982 study was to determine the feasibility of constructing a dam near Miner Flat and to select an approximate alignment for the dam. The 1982 study (Robinson 1982) consisted of preparing a preliminary geologic map of the dam and reservoir area and drilling along the recommended axis of the dam. The 1985-1986 investigation (Mineral Systems, Inc. 1986) was directed towards further defining the geology of the dam axis and abutment areas. The earlier investigations defined the general suitability of the site for a gravity dam and appurtenant structures. The present investigation was performed in order to evaluate the potential for seepage through the left abutment ridge of the proposed dam and reservoir.

## 2.0 GEOLOGY

The Fort Apache Indian Reservation is south of the Mogollon Rim, that is considered the southern boundary of the Colorado Plateau physiographic province. The Fort Apache Indian Reservation is in the transition zone between the Colorado Plateau and the Basin and Range physiographic provinces. The North Fork of the White River originates in the White Mountains that comprise the northeast corner of the reservation. The White Mountains are composed of Tertiary and Quaternary volcanic rocks, that extend to and south of the proposed Miner Flat dam site. The volcanic rocks, predominantly basalts and andesites, were deposited on Paleozoic and Mesozoic sedimentary rocks. The sedimentary rocks, in general, strike northwest and dip gently northeast, 2° to 8°. Locally the sedimentary rocks are cut by normal faults approximately parallel to the strike of the sedimentary formations. Small folds are locally superimposed on the regional dip.

The axis of the proposed Miner Flat dam crosses the North Fork of the White River where basalt exists in both the right and left abutments. The basalt is approximately 2 million years old (Condit 1983) and consists of a sequence of flows that came down an ancestral valley of the White River. The ancestral valley was then cut by the river into the Permian sandstones of the Coconino Sandstone and Supai Group and Tertiary gravel deposits, leaving a ridge of sandstone and basalt on the left abutment. Recent deposits include colluvium on the slopes and alluvium in the river valley.

The basalt is up to 260 feet thick in the right abutment and up to 211 feet thick on the left abutment. The basalt wraps around the left abutment ridge with Coconino Sandstone on the north side of the ridge and basalt on the south side of the ridge. East of the left abutment, the basalt thins out against the Coconino Sandstone.

The Coconino Sandstone, an aeolian deposit of Permian age, is a distinctive stratigraphic unit characterized by prominent cross-bedding, uniform grain size and general cliff-forming

character (McKee 1933). The typical Coconino Sandstone extends over a vast area without any appreciable change in type of cross-bedding, purity of quartz sand, kind of cement, or general massive character (McKee 1933).

At the proposed Miner Flat dam site, the Coconino consists of friable, fine-grained, subrounded, equigranular, and well sorted quartz grains poorly cemented by siliceous cement. The Coconino Sandstone ranges in color from pale reddish brown, grayish orange pink, moderate reddish orange and moderate orange pink. Characteristic of the Coconino Sandstone are quartz overgrowths on some grains, which make the grains sparkle in sunlight. The beds of the Coconino Sandstone range from less than a foot to as much as 10 feet in thickness.

Underlying the Coconino Sandstone is the Supai Group. The Supai Group is separated into four formations that range in age from Early Pennsylvanian to Early Permian. From oldest to youngest, these formations are designated as the Watahomigi, Manakacha, and Wescogame Formations and the Esplanade Sandstone (McKee 1975). At the Miner Flat dam site, the Supai Group is represented by the Esplanade Sandstone. The Esplanade consists primarily of very fine-grained cross-stratified sandstone that forms massive cliffs. It includes small amounts of siltstone and mudstone that weather into slopes, benches, or recesses and, contains extensive beds of gypsum. The Supai Group observed at the site consist of moderate reddish orange to moderate reddish gray, calcareous siltstone, fine-grained sandstone, claystone, and light gray gypsum.

The Tertiary gravel deposits partially mantel the east end of the east abutment ridge, and slope wash from these deposits mask the bedrock. The deposits consist of bouldery gravel, coarse sand, sand and silt. The clasts are comprised of quartzite, sandstone, limestone, and chert derived from younger Precambrian and Paleozoic formations. Locally, cobbles of granite and diabase may be found.



Colluvial deposits that overlay bedrock on the left abutment is material derived from the in-place weathering of bedrock. The colluvium derived from the basalt consists of reddish brown to dark brown silt and clay, with subangular sand, gravel, cobbles and boulders composed of basalt. Colluvium derived from the Coconino Sandstone and Supai Group consist of light brown to pale reddish brown fine to medium sand, silt, and angular pieces of gravel and cobble size sandstone.

The alluvial deposits occurring in the river valley of the North Fork of the White River consist of boulders, gravel, sand, and silt. The coarser materials are derived from the local outcrops--such as basalt and sandstone--and from the Tertiary gravels. The materials are unconsolidated in the modern stream beds and poorly to moderately cemented in the older stream terraces.

### 3.0 FIELD INVESTIGATION

The 1995 field investigation included core and percussion hammer drilling, downhole surveying, packer testing, and installing piezometers for the purpose of obtaining hydrogeologic data for seepage analysis of the eastern abutment. The drillholes were spaced approximately 200 feet apart located along the ridge and about midway between the upstream and downstream cliffs (Figure 2) in accordance with the work plan provided by MM/CSSA on January 31, 1994.

#### 3.1 Percussion Hammer Drilling

As a result of preliminary geologic mapping of the proposed dam site, and from earlier drilling, it was concluded that an abandoned stream channel may cross the left abutment. This channel would have developed after the deposition of the basalt and before the White River eroded its present channel. Previous drilling indicated that the bottom of the channel was below an elevation of 6,060 feet, that would be below the maximum elevation of the proposed reservoir. The material at the surface implied that the channel could be filled with highly permeable sand and gravel.

A total of 26 boreholes were drilled along the ridge between boreholes MF-102 and MF-218A (Figure 2).

Drilling was subcontracted to Layne Western Exploration (Layne) and North American Drilling (NAD), both from Phoenix, Arizona. An AP1000 percussion hammer drill rig with an open, 6 5/8-inch bit was used for the initial drilling program and an open, 9-inch bit was used for the later drilling program. The percussion hammer method uses dual wall drive pipe advanced by a single cylinder diesel pile drive hammer. Compressed air is forced down the annulus between the inner and outer drive pipe, and geologic samples are continuously returned up the center of the inner pipe. Percussion hammer drilling methods are best suited

and typically used in gravelly or cobbly materials where standard penetration tests (SPT) are unable to collect representative material and blow counts.

The blow counts from the percussion hammer can be correlated to SPT equivalent blow counts ( $N_{60}$ ) and relative density by recording the blow counts and pressure within the drive cylinder for the same interval. Allowing for altitude and type of hammer on the AP1000, an SPT equivalent blow count ( $N_{60}$ ) was determined for each 1-foot interval. The equivalent blow counts are shown on the borehole logs in Appendix A. The SPT equivalent blow count number is used to correlate relative density or cohesion of the soils. Because an open bit was used in order to collect samples, the SPT equivalent blow counts will typically be lower than when a closed bit is used (Harder and Seed 1986). The SPT equivalent blow counts for the 9-inch bit were correlated from Sy and Campanella (1994).

Bulk samples were collected, labeled, and bagged at 5 foot intervals. Selected bulk samples were transported to Golder's Lakewood, Colorado soils laboratory for further classification and testing. Attempts were made to collect undisturbed samples in Shelby tubes below elevation 6,060. However, the soil below elevation 6,060 was typically too dense or hard to collect sufficient samples for testing. Only one sample had sufficient material for testing.

### 3.1.1 Material Classification

Materials encountered during percussion hammer drilling were visually classified and logged by a geological engineer in accordance with American Society for Testing and Materials (ASTM) D 2488-93. Material descriptions shown on the borehole logs (Appendix A) include: ASTM designations; density (for cohesionless soils) or consistency (for cohesive soils); color (based on the Munsell System using a Geological Society of America rock color chart); textural descriptions; percentage of minor components; and qualitative moisture content. Representative samples were tested with dilute hydrochloric acid to estimate relative calcium carbonate content.

### 3.1.2 Laboratory Analysis

Samples obtained from the field program were analyzed at the Golder Associates Inc. laboratory in Lakewood, Colorado in accordance with applicable ASTM standards. Laboratory testing was performed on 11 bulk samples and 1 undisturbed sample. The samples were selected from boreholes where the channel material was below elevation 6,060. The bulk samples were tested for Unified Soil Classification System (USCS) soil classification, Atterberg limits, specific gravity, and moisture density relationship (standard Proctor). The undisturbed sample consisted of clay that was tested for permeability.

The results of the laboratory testing programs are presented in Appendix B. The majority of the soil below elevation 6,060 is classified as clay (CL); one sample was classified as clayey sand (SC), although the clay content was at 49.6 percent (greater than 50 percent clay would be classified as CL). The Atterberg limits ranged from liquid limits of 27 to 40 and plastic index of 15 to 23, which indicate the clay is low to medium plasticity.

The clay sample collected in the Shelby Tube had a natural dry density of 108.2 pounds per cubic foot and natural moisture of 9.2 percent. The average permeability from laboratory testing was  $2.7 \times 10^{-7}$  cm/sec.

## 3.2 Core Drilling

Boreholes were drilled and cored along the axis of the ridge that forms the left abutment of the proposed Miner Flat dam to investigate the potential for seepage through the Supai Formation as required by the work plan provided by MM/CSSA dated January 31, 1994. The locations of the boreholes, shown on Figure 2, were in conformance with the work plan provided by MM/CSSA. Final location of the boreholes was determined in the field based on site logistics and access. All locations were approved by MM/CSSA prior to the commencement of drilling. Eight boreholes were drilled using HQ (2.375 inch diameter

core) wire-line equipment with dual-tube core barrels. The core drilling was performed by Golden Drilling Company of Canon City, Colorado. The program began on June 1, 1995 and was completed on August 30, 1995. A total of 2,263.8 feet of borehole was drilled. This included 153.2 feet of surficial materials (Tertiary gravel and colluvium) and 2,110.6 feet of rock. Surficial materials were drilled using air-rotary methods using a 6-inch diameter tricone bit. Once competent rock was encountered, 4-inch steel casing was installed and fully grouted with neat cement grout. The boreholes were then cored to their final elevation, in most cases an elevation of 5,740, as specified the work plan.

Each borehole was logged for lithology, RQD, number of natural fractures, and other engineering geologic parameters as specified in the work plan provided by MM/CSSA. In addition, all the core was photographed and stored for future reference.

Clear water was the intended drilling fluid during the planning and initial stages of drilling. The source of the water was the North Fork of the White River, supplied to the drill sites through 1-inch diameter plastic pipe from the supply pump at the river. The drilling fluid was changed to a polymer based drilling mud when core recovery and borehole stability became a problem. During the drilling of borehole MF-218 core recovery and borehole stability became a problem at a depth of approximately 160 feet. In consultation and with the agreement of MM/CSSA the drilling fluid was modified to a polymer-bentonite-additive fluid the reduced fluid loss, friction, and bedrock wetting. The benefits of the switch to the polymer-based fluid included better borehole stability, increased core recovery and faster penetration rates. The disadvantage was that packer testing could not be conducted during the drilling of the borehole as recommended by MM/CSSA but had to be delayed until the borehole was drilled to its final depth.

The work plan included six boreholes aligned with the ridge that formed the south or southeast abutment spaced 200 feet apart. Two additional boreholes were drilled due to fluid loss and borehole stability problems. MF-250 located at the north end of the series of

boreholes (Figure 2) was the first borehole to be drilled in this program. The borehole encountered basalt at a depth of 11.8 feet. Fluid loss and borehole stability became an immediate problem and the upper portion of the borehole required grouting in order to maintain the borehole. An additional borehole, MF-250A located approximately 50 feet south of MF-250, was drilled to perform packer tests in the grouted portion of the basalt. Borehole MF-218 was abandoned after fluid loss and borehole stability problems became overwhelming at a depth of 160 feet. Borehole MF-218A was drilled approximately 50 feet southeast of MF-218 as a replacement.

The overall average of core recovery was 86.3 percent. The core recovery was highest in the basalt with an average of 95.7 percent. Core recovery in the Coconino Sandstone was 82.7 percent. The core recovery was 90.0 percent in the sandstones and siltstones of the Supai Group and 100 percent in the gypsiferous sections of the Supai. Core recoveries increased by 9.6 percent in the Coconino and by 4.1 percent in the Supai as a result of changing from clear water to a polymer based drilling fluid.

### 3.2.1 Core Logging

Logs of lithologic and engineering properties of the rock drilled were compiled at the completion of each core run. Golder personnel were on site continuously during the drilling program to record drilling conditions and to log the core as it was extracted from the core barrels. A detailed engineering geologic log was prepared on site at the time of drilling, a portion of which is presented in Appendix A. The parameters that are presented in Appendix A include:

- ▶ Weathering or Alteration
- ▶ Rock Description
- ▶ Recovery

- ▶ Rock Quality Designation
- ▶ Fractures Per Foot Recovered
- ▶ Total Fractures
- ▶ General Comments

Detailed lithologic descriptions and drilling conditions were recorded at the time of drilling and kept separately. These detailed descriptions are available upon request.

#### 3.2.1.1 Weathering or Alteration

Weathering or alteration describes the type and intensity of rockmass weathering or alteration classification using the International Society for Rock Mechanics (ISRM) classification system.

#### 3.2.1.2 Rock Description

The rock description includes the rock type, the major minerals present, the grain size of the minerals or rock constituents, the texture of the rock fabric, and the color based on the Munsell System.

#### 3.2.1.3 Recovery

The core recovery is recorded here as 100 times the length of core recovered in each core run. The percentage core recovery is calculated by dividing the recovered length by the run length. The mean core recovery for the basalt was 95.7 percent, 82.7 percent for the Coconino, and 93.1 percent for the Supai.

#### 3.2.1.4 Rock Quality Designation

The rock quality designation (RQD) is a method for classifying recovered core to reflect the fracturing and alteration of rock masses (Deere 1963). RQD is the summation of the lengths of recovered, hard, sound core whose length are greater than two times the core diameter. For convenience RQD is recorded in the logs as 100 times the summed length. The RQD (%) is the RQD length divided by the run length and expressed as a percentage of the run length. The mean RQD for the basalt was 62.1 percent, for the Coconino was 18.8 percent, and for the Supai was 69.9 percent.

#### 3.2.1.5 Fractures Per Foot Recovered

The fractures per foot recovered or fracture frequency is the total number of natural fractures observed in each core run divided by the total length of core recovered for that run. The mean fracture frequency for the basalt was 2.4 fractures per foot, for the Coconino was 5.9 fractures per foot, and for the Supai 2.8 fractures per foot.

#### 3.2.1.6 General Comments

The general comments include summary notes of drilling and rockmass conditions. Contacts between formations are generally noted in the general comments.

#### 3.2.2 Downhole Surveys

All the boreholes were surveyed to determine the accurate downhole location of the borehole. Seven of the boreholes were surveyed with an OWL TECHNICAL 780 series borehole deviation survey instrument to measure and record the path of the boreholes. Results of these surveys presented in Appendix C include the measured data of depth, inclination and bearing, and the calculated location of each survey measurement point.



### 3.3 Hydrological Testing Program

The objective of the hydrogeological testing was to obtain estimates of in situ hydraulic conductivity from the cored boreholes. Hydraulic conductivity was calculated from packer (injection) test data for specific intervals in the borehole.

#### 3.3.1 Packer Testing Equipment

The test equipment included a packer assembly consisting of two inflatable packers in a straddle configuration connected by a perforated pipe providing a test interval of approximately 23 feet. Medium-duty, sliding-end, 2.7-inch uninflated outside diameter Baski Inflatable Packers were used for most of the testing. Packers were inflated from a nitrogen cylinder at the surface via an inflation line that was strapped to the drill pipe. Flow rates for this assembly ranged from approximately 1 gallon per hour (gph) to approximately 30 gallons per minute (gpm).

For the high-flow test intervals, an additional packer configuration was developed. The high-flow packer assembly consisted of 2-inch, inside diameter plumbing and a 10-foot test interval between packers. The high-flow assembly allowed testing of intervals with flow rates up to approximately 100 gpm.

During injection, the water supply was kept at a constant pressure in a baffle tank. From the baffle tank, flow was diverted to the drill pipe or bypassed through alternative valves and flow meters. The flow meters included three Omega Rotameters capable of measuring flows between 0.017 to 0.117 gpm (1 and 7 gallons gph), 0.1 to 1 gpm, and 1 to 5 gpm. Higher flow rates were measured with a turbine flow meter and totalizer, calibrated to 10 gpm for one complete revolution of the dial. Flowmeters were connected in parallel and each fitted with a valve to allow selection of a particular meter and to regulate the flow.

The pressure data acquisition equipment consisted of a number of alternative strain gauge transducers including two with a maximum pressure reading of 30 pounds per square inch (psi) and one with a maximum reading of 100 psi. Depending on the anticipated maximum pressure, the appropriate transducer was placed in the drill pipe, either below the static water level when testing below the water table or just above the top packer when testing in the unsaturated zone. The transducer cable was connected to a data logging system at the surface which processed the signal and provided the digital input to a laptop computer.

### 3.3.2 Testing Procedures

Testing all boreholes over their entire depths (except for unconsolidated Tertiary gravel) provided an unbiased sample of estimates for hydraulic conductivity. Complete coverage was achieved by sequentially testing adjacent intervals, starting from the bottom of each borehole. Some test interval locations varied slightly due to practical limitations of packer seating locations and borehole conditions.

Two rounds of packer testing were conducted; one in which all intervals in all boreholes were tested, and a second round which involved retesting high-flow intervals only. For the first round of packer testing, a 23-foot perforated test interval was used. This spacing was small enough to provide sufficient hydraulic conductivity estimates for adequate technical assessment of the site, yet large enough to achieve adequate coverage within the practical constraints imposed by time and budget. A second round of packer testing, using the high-flow packer assembly, was conducted to retest smaller (10-foot) intervals.

Two types of packer tests were possible: constant head (steady-state) tests, or constant rate (transient) test. In the saturated zone, either type of test could be conducted. In the unsaturated zone however, only constant head tests were conducted because results from constant rate tests reflect the saturation of the aquifer instead of providing accurate hydraulic properties.

The packer testing procedure can be summarized as follows:

- a) Take manual water level measurement
- b) Set and inflate packers
- c) Place transducer approximately 3 to 5 feet above top packer
- d) Begin logging pressure data
- e) Add slug of water for falling head test and calculate an injection rate
- f) Decide on test type and conduct test
- g) Deflate and raise packer assembly to next interval

#### 3.3.2.1 Flow Rate Evaluation and Choice of Test Method

After placing the packer assembly, a water level measurement was taken (if the interval was below the water table) and the packers were inflated. Before starting data acquisition, the pre-test head was set to zero by adjusting an offset function in the data logging software. Acquisition of the pressure data began after packer inflation when in the unsaturated zone. When below the water table, it was preferable to begin acquisition before packer inflation. As a result of packer inflation, the test interval may be pressurized. Test initiation was delayed until the pressure pulse dissipated and the water level returned to the static condition.

A target flow rate for the test was estimated by conducting a short-term slug test. A volume of water sufficient to fill approximately 15 to 20 feet of drill pipe (2 to 3 gallons) was added 'instantaneously' and the rate at which the head decreased was recorded. In the unsaturated zone, prior to the slug test, it was necessary to fill the test interval and saturate the rock in the immediate vicinity of the borehole. The volume of water draining into the formation was then calculated by multiplying the head drop by the cross-sectional area of the drill pipe, and the flow rate was calculated by dividing by the time over which the head drop occurred.

The interval location and the flow rate calculated from the falling head are the two criteria that determine which test method to use. If the interval was below the water table and the flow rate is greater than 0.1 gpm, then it was possible to attempt a constant rate test. If the interval was in the unsaturated zone and/or the flow rate during the falling head period was less than 0.1 gpm, then the 'constant head' test method was used since flow rates less than 0.1 gpm are too low to measure with confidence over the short time of the test. Under these conditions, pressure was held constant and head changes were measured.

### 3.3.2.2 Constant Rate (Transient) Test Procedure

After the water level returned to static from the falling head test, an appropriate flow rate was selected and the water delivery was set while the valves were in the bypass position (*i.e.*, no down-hole flow). The valves were then switched to allow the constant flow to be directed down the drill pipe. Initially the pipe began to fill, as the head increased more flow entered the rock and pressure change decreased with time. The criterion for finishing the constant rate test was when the change of head with time was negligible or when the test duration reached three hours.

A practical difficulty with conducting constant rate tests was choosing an appropriate rate on the basis of a relatively small amount of information. The intent was to achieve a stable regime of radial flow in the adjacent formation (linear relationship on a semi-log plot) with the maximum possible head. It was difficult to maintain a truly constant rate over the duration of the test. If the injection of water flushed solid material from around the test interval, the shape of the pressure build-up curve could have been disturbed, and this may influence the quality of the data and analysis. Also, the injected water must travel approximately 150 feet down the drill pipe. This caused variations in the pressure data which makes analysis difficult. Although theoretically advantageous, the constant rate test method proved difficult to conduct. If the quality of the data after beginning a constant rate test was questionable, the decision was made to convert to the constant head method.

### 3.3.2.3 Constant Head (Steady State) Test Procedure

If a constant head is maintained, the change in rate will reduce logarithmically with time so that a steady-state analysis is acceptable with negligible error. Thiem (1906) and later Hvorslev (1951) developed equations and analysis procedures based on the assumption of steady-state radial flow. Their analyses require constant head values and corresponding pseudo-constant flow rates.

Three constant-head steps were generally achieved in three hours. Steps of equal head change, over the maximum range of available head, were attempted. The range of applicable heads were determined from the depth of the test interval or water table and the maximum practical flow rate. High permeability zones required high flow rates to achieve small increases in head thereby reducing the number of possible steps to one or two. The maximum flow rate was constrained by the available water supply and by the pressure losses in the packer assembly.

The procedure for obtaining a constant head step was to fill the drill pipe to the required head and then adjust the rate to maintain a constant head for the duration of the step. The rate was recorded at least every five minutes or when it was adjusted.

The ability to maintain a constant head varied according to the flow rate and the particular test conditions. When possible, a constant head was maintained with a variation of 0.5 feet or less. When the test interval was relatively deep, the flow rate was high and the applied head was low, the injected water had to travel over 100 feet to the water surface in the drill pipe causing short duration fluctuations in the head on the order of several feet.

If the test interval was close to the surface or the rock had low permeability, a head above the ground surface was imposed in order to obtain a high step or flow rate. This was achieved by attaching a sealed cap (stuffing box) on top of the drill pipe. This cap formed

a secure seal around the transducer cable. The water supply system in this configuration is completely closed and an increase in flow rate will increase the pressure and the effective head imposed on the test interval.

Once the final constant head step was completed, the flow was diverted to the bypass and the falling head in the drill pipe was recorded electronically (removing the pressure cap if necessary). This falling-head data was used to confirm the flow rate. After collecting several minutes of falling head data, the data acquisition was halted and the packers deflated.

### 3.3.3 Packer Test Analyses and Results

The Theis curve-matching method (Theis 1935) was used to analyze the constant rate (transient) tests, and the straight line fitting method (Thiem 1906 and Hvorslev 1951) was used to analyze the constant head (steady-state) test. The transient test can provide an estimate of the hydraulic conductivity and storativity as well as other information about the presence or absence of no flow or constant pressure boundaries. The accuracy of the assumptions of the transient test, however, are questionable within an unsaturated zone and it is often difficult to maintain a constant flow with sufficient accuracy to gain many of the benefits of this method.

Several constant rate tests were attempted, however, data quality was poor due to the physical constraints of maintaining consistent flow in the deeper, saturated test intervals. Once ascertained that the data being collected was not suitable for a high-quality transient analysis, the test was converted to a constant head test.

For subsequent use in seepage modeling, hydraulic conductivity values were compiled based on the lithology in which tests were conducted. Two lithologies were tested: basalt and sandstone.

Figures 6 and 7 present histograms and cumulative distribution curves of the basalt and hydraulic conductivity values, respectively. Conductivity values for the basalt range from  $21.6$  to  $5.2 \times 10^{-2}$  cm/sec with a geometric mean of  $5.86 \times 10^{-5}$  cm/sec. The sandstone conductivity values range from  $4.68 \times 10^{-3}$  to  $3.62 \times 10^{-8}$  cm/sec with a geometric mean of  $7.86 \times 10^{-5}$  cm/sec.

Figure 5 shows three cross sections with the packer test results posted along the borehole traces. In general the hydraulic conductivity decreases with depth.

### 3.4 Piezometers

After completion of the hydrological testing of the MF-218A, MF-250, MF-251, MF-252, MF-253, and MF-254 boreholes, piezometers were installed in these boreholes for long-term water-level monitoring. Piezometer construction summaries are provided in Appendix E.

Boreholes were backfilled with 3/4-inch rounded to sub-rounded gravel from the bottom of the borehole to the appropriate placement of the piezometer. Approximately 15 feet of bentonite "Holeplug" chips were placed on top of the gravel backfill. One foot of 10-12 grit silica sand was placed on top of the bentonite. The piezometer string consisted of 1.5-inch diameter, schedule-80, flush-threaded PVC with 20 feet of 0.020 inch slot for the screen. The filter pack consisted of 10-12 grit silica sand and was placed around the screen to 10 feet above the top of the screened section. Another 15 feet of bentonite "Holeplug" was placed on top of the filter pack and hydrated. The remainder of the borehole was backfilled with gravel to 10 feet below the surface except MF-218A, which was filled to 2 feet below ground surface. The installations were completed by grouting to the surface with Portland type I-II cement slurry with 5 percent Quickgel bentonite powder. Grout was allowed to settle overnight and then were topped off with additional grout the following day.

## 4.0 SEEPAGE MODEL

### 4.1 Objectives

The objective of the project as stated by the MM/CSSA work plan was to "Complete a six-hole transect on the left abutment for the purpose of investigating the potential for seepage through the Supai Formation. Specific objectives included using detailed site-specific geologic and hydrogeologic data to estimate aquifer water levels in the sandstone aquifer under boundary conditions defined by a full reservoir. Using this model, it is possible to estimate the seepage rate through the section of Coconino Sandstone, the formation that is most affected by the reservoir, that crops out along the sandstone ridge that forms the left abutment. The ridge consists of two distinct sections: 1) a "window" of Coconino Sandstone that outcrops across the ridge; and, 2) the tip of the ridge that is Coconino Sandstone overlain by Quaternary basalt. Both sections are considered to be potential preferred flow paths through the ridge. In addition, the potential for seepage through the "gravel channel" was evaluated.

### 4.2 "Gravel Channel" Seepage Potential

Previous core drilling and preliminary geologic mapping of the proposed dam site indicated that an abandoned river channel may cross the left abutment between boreholes MF-121 and MF-123 (Figure 2). From the previous drilling it was inferred that this channel, if it existed, would have been developed on top of the basalt surface prior to the erosion of the present channel of the North Fork of the White River. The bottom of the channel was interpreted to be at an elevation of approximately 6,055, below the maximum pool elevation of 6,060. The material at the surface implied that this channel could be filled with highly permeable sandy and cobbly gravel. If this channel exists then a potential for seepage from the reservoir exists. The "gravel channel" seepage potential was investigated by the use of the



percussion hammer drilling of 26 boreholes, logging of materials, laboratory analysis, and laboratory testing.

To evaluate the potential for seepage through the "gravel channel" a bedrock surface model was prepared from the following sources: previous drilling results, the results of the percussion hammer drilling, the bedrock contact from the 1995 core drilling program, and from selected surface outcrops. Figure 4 represents the results of the bedrock surface model. The surface model consisted of a 50 ft by 50 ft grid that covered the left abutment ridge. The bedrock elevation was estimated at each node using a Delaunay triangulation algorithm and then contoured with the 1 foot contour interval shown on Figure 4. The bedrock elevation data set contained 60 points. As with any surface modeling tool there are some inherent inaccuracies especially at the edges of the model and in areas of sparse data. Boreholes with the MF-B prefix have not been surveyed and are only approximately located. The bedrock contacts in these boreholes have been used as data for the surface modeling introducing another source of potential inaccuracy. The bedrock surface model therefore, should be used with discretion.

The surface model indicates that a true channel is not present on the left abutment. There is, however a relatively large westerly dipping bedrock surface bounded on the east by boreholes BA-1 and BA-7. The area between borehole MF-123 and BA-7 represents an area where water from the reservoir at elevation 6,060 could be lost through seepage above the bedrock surface.

Cross section A-A' (Figure 3) shows the surficial soil and top of bedrock along the existing access road between MF-218A and MF-102.

- ▶ Soils encountered in the boreholes consisted of silty or clayey sands with gravels and cobbles near the northern and southern part of the section and silty clay with none to very little gravels or cobbles in the middle of the section

- ▶ Bedrock consists of sandstone in MFB-20, MFB-21, MFB-23, MFB-32, BA-8 and BA-1 and in all the other boreholes

Cross section B-B' (Figure 3) shows the surficial soil and top of bedrock encountered in the coreholes between MF-218A and MF-102 (Figure 4). The cross section suggests the following:

- ▶ Surficial soil encountered in the coreholes consists of clayey silt, silty sand and clay across this section
- ▶ Bedrock along this section is basalt
- ▶ The basalt bedrock surface dip to the west

Cross section C-C' shows the surficial soil and top of bedrock between MF-218A and MF-218 (Figure 3). The cross section suggests the following:

- ▶ Surficial soil encountered in the boreholes consists of silty sand and sand
- ▶ Depth to bedrock is shallow

The materials encountered in boreholes with bedrock elevations below 6,060 are sandy clay (CL) with a small percentage of gravel and cobbles. The materials are stiff to very stiff. Laboratory permeability testing of remolded samples of this material averaged  $2.7 \times 10^{-7}$  cm/sec.

It is not likely that this area of a low bedrock surface will represent much of a seepage loss. Uncertainties exist as to the eastern extent of the basalt bedrock in the area between borehole MF-123 and BA-7 and in the in situ permeability of the materials. A more detailed evaluation of the necessity of seepage mitigation in this area must be completed prior to final design of the dam and appurtenant structures.

### 4.3 Conceptual Bedrock Model

The sandstone ridge to the south and east of the proposed dam abutment will serve to contain the proposed reservoir. This narrow strip of land is considered a potential seepage pathway from the proposed reservoir.

Geologically, the ridge is divided approximately in half; the northern portion of the ridge consists of Permian sandstones overlain by Quaternary basalt and the southern part is composed of only Permian sandstone (Figure 8). This southern sandstone "window" is bounded by the occurrence of overlying basalt to the north and south. In the north, the sandstone/basalt contact dips gently westward across the ridge and then dips steeply underneath the North Fork of the White River.

The model of the ridge is divided into two segments based on the geology described above; the "basalt" model representing the northern section of the ridge, and the "sandstone window" model, representing the southern portion of the ridge. Figure 9 shows the outlines of the models. Each model is designed to run on its own and results from each model summed for a total seepage flow rate through the ridge. The northern tip of the ridge (the left abutment of the proposed dam) is not included in the model. The seepage modeling for the dam abutment and foundation is not included in the scope of this investigation.

The basalt model consists of two layers: an upper layer of basalt and a lower layer of sandstone. Seepage flow through the ridge is estimated by simulating a full reservoir (pool elevation of 6,060 feet) on the eastern edge of the ridge and a river elevation of 5900 feet on the western edge of the ridge. The reservoir acts as a source for recharge to the ridge and the river acts as the discharge sink.

The sandstone window model is designed similarly to the basalt model except that it consists only of one layer and the river discharge elevation is defined at 5,880 feet.

## 5.0 MODELING METHOD

Modeling was conducted using VISUAL MODFLOW (WHS 1996), a user interface for MODFLOW. MODFLOW is a three-dimensional, finite-difference groundwater flow model developed by the USGS. Groundwater flow within the aquifer is simulated using a block-centered, finite-difference approach. Layers can be simulated as confined, unconfined, or a combination of confined and unconfined. The finite-difference equations can be solved using a variety of iterative solution methods (McDonald and Harbaugh 1988).

### 5.1 Sandstone Window Model

The model of the sandstone window consists of one layer with a base elevation of 5,856 feet, approximately 800 feet square. The base elevation of 5,856 represents the approximate contact elevation between the Coconino Sandstone and Supai Group. The top surface of the model is defined by the topographic surface. The model is bounded on both the north and south by the surficial occurrence of the basalt/sandstone contact. The contacts are represented in the model as no-flow boundaries. The window is bounded on the east by the pool elevation of the reservoir (6,060 feet) and on the west by the North Fork of the White River (5,880 feet). These are represented as constant head boundaries (Figure 1). The grid consists of 100 by 100 divisions, or 10,000 variably-sized cells.

Hydraulic conductivity for the sandstone window model was based on results of constant-head injection packer testing conducted in borehole intervals open to the Coconino Sandstone. Hydraulic conductivity values from tested intervals in boreholes MF-218A and MF-253 were averaged (geometric mean) yielding a value of  $1.94 \times 10^{-4}$  cm/sec (Figure 10).

Recharge and evapotranspiration were assumed to be negligible given the arid climate and the depth to water in the area. Since the model was run under a steady-state condition, specific storage, yield and porosity parameters for the aquifer were not necessary. The sandstone

aquifer was defined as unconfined. Output from the flow model was used by the zone budget module of MODFLOW to calculate seepage.

The zone budget module of MODFLOW allows the calculation of flow rates from one section of the modeled area to an adjacent section. Sections are defined by the user and can consist of any combination of unique areas within the model area. For this modeling exercise, the flow of interest was that through the Coconino Sandstone window to the North Fork of the White River to the west. A zone was defined therefore, as encompassing the sandstone window in the east and another zone beginning at the east edge of the river. Flows were estimated based on the resultant steady-state water level contour map predicted by the flow model.

## 5.2 Basalt Model

The basalt model has a base elevation of 5,880 feet, is approximately 600 feet square, and consists of two layers. The upper layer consists of basalt and the lower layer of undifferentiated sandstone. As with the sandstone window model, the top surface of the model is defined by the topography. The location of the basalt/sandstone contact was based on control points and geologic mapping on the east side of the model area and extrapolated to the western portion of the model area. The north and south edges of the model are represented by no-flow boundaries. The model is defined on the east by the pool elevation of the reservoir (6,060 feet) and on the west by the North Fork of the White River (5,900 feet). These are represented as constant head boundaries (Figure 9). The grid consists of 100 columns by 92 rows and two layers, or 18,400 cells.

Hydraulic conductivity values used in the model were based on constant-head injection packer testing conducted in basalt and sandstone. Hydraulic conductivity values from basalt intervals from nine boreholes were averaged (geometric mean) yielding values ranging five orders of magnitude, from  $10^{-4}$  to  $10^0$  cm/sec. A value was assigned to the unfractured basalt

using the geometric mean of the results in the  $10^{-3}$  to  $10^{-4}$  cm/sec range. The high-flow zones in the basalt were represented as three pipes with hydraulic conductivities defined by the geometric mean of the hydraulic conductivity results in the  $10^{-2}$ ,  $10^{-1}$  and  $10^0$  cm/sec ranges, respectively (Figure 10). Geometries of the pipes were determined from the relative percentages of total packer test interval represented by each conductivity value used. For example, the  $1.05 \times 10^{-2}$  cm/sec conductivity value used in the model represents testing done over 2.1 percent of the total 1,065 feet of basalt that was packer tested. A three-dimensional "pipe" representing 2.1 percent of the total volume of basalt in the model was defined. The "pipe" extended the full width and thickness of the basalt in the model and was assigned the  $1.05 \times 10^{-2}$  cm/sec conductivity value.

Recharge and evapotranspiration were assumed to be negligible given the arid climate and the depth to water in the area. Since the model was run using a steady-state condition, specific storage, yield and porosity information for the aquifer were not necessary. The basalt layer was defined as unconfined and the sandstone aquifer was defined as confined with variable storage and transmissivity. Output from the flow model was used by the zone budget module of MODFLOW to calculate seepage.

The zones for the zone budget calculations were defined in a similar manner to those for the sandstone window model. For the basalt model, the flow of interest was that from both the sandstone and basalt to the North Fork of the White River. One zone was defined, therefore, as encompassing both layers of the basalt and the sandstone in the east and another zone beginning at the east edge of the river. Flow was calculated based on the resultant steady-state water level contour map predicted by the flow model.

### 5.3 Modeling Assumptions and Limitations

Groundwater numerical modeling is a process of simulating the natural flow system. Due to the natural complexity and variability of the hydrogeologic system, certain simplifying

assumptions are necessary for defining the system for modeling purposes. These modeling assumptions include:

- ▶ Darcy flow conditions exist within the aquifer.
- ▶ No fracture flow occurs through the formation, that is, the ridge can be represented as a single-porosity flow system.

For the sandstone window model, the Darcy flow and no-fracture-flow assumptions are generally valid over the majority of the sandstone. Although some fracture flow may occur through some of the friable and weathered sandstone material, packer test and geologic logging results do not indicate that high-flow zones or fracture zones exist in the sandstone window. The assumption is uncertain in the basalt model since relatively discrete high-flow zones are known to exist. The "pipes" incorporated into the basalt model were designed to account for these discrete features in the basalt.

- ▶ Each model layer is homogeneous and isotropic.

For the sandstone window, the assumption that each layer is homogeneous and isotropic is generally valid as, according to the geologic logs, major variation within the Coconino Sandstone at the site is uncommon both laterally and with depth. This assumption holds less for the basalt, which contains discontinuities and structural features such as fractures and inter-basalt(lava)-flow-boundaries.

- ▶ Recharge and evaporation are negligible.

Net precipitation recharge is expected to be near zero for most of the year due to the arid climate in the region. Any recharge occurring in the model area is minor, and, compared to the groundwater flux driven by the reservoir head, can be considered negligible.

- ▶ Saturated K values used in the model are similar to measured unsaturated K values.

Hydraulic conductivities were calculated from packer test data over the entire zone predicted to be part of the flow regime under reservoir conditions. This zone includes both currently saturated and unsaturated material. Unsaturated packer test intervals were saturated by adding water

prior to commencing a test since unsaturated hydraulic conductivity values are generally higher than saturated values. This allowed greater confidence in the conductivity values obtained from unsaturated packer testing.

- ▶ No flow occurs between the sandstone and basalt (sandstone window model only).
- ▶ No flow occurs between the Coconino and Supai Group below (sandstone window model only).

The assumption that there exists no hydrologic communication between adjacent formations is necessary for defining model boundaries, however this may not be a completely valid assumption. The competent basalt bounding the sandstone window is generally less hydraulically conductive than the sandstone but also contains high-flow zones. The particular high-flow features may have a much higher conductivity than the sandstone, therefore, flow may occur from the sandstone to a fractured basalt zone. At the lower boundary of the model, the Coconino Sandstone grades into the underlying, finer-grained Supai Group. The Supai Group has a slightly lower hydraulic conductivity. Also, it is unlikely that a significant downward hydraulic gradient exists in the Supai. This suggests that hydrologic communication would be minimal, however, effects of a no-flow boundary at the contact are uncertain.

- ▶ Laterally continuous "pipes" of higher hydraulic conductivity are conservatively representative of discrete high-flow zones in the basalt (basalt model only).

The packer tests conducted by Golder in 1995 revealed zones of high hydraulic conductivity, presumably associated with flow through fractures or inter-(lava) flow boundaries. These discrete features, defined as "pipes" in the model, represent the most direct and continuous flow route through the ridge. The assumption that "pipes" through the basalt represent these discrete high-flow features, therefore, is a conservative assumption.

- ▶ The basalt/sandstone contact can be extrapolated from borehole logs and geologic mapping (basalt model only).

The basalt model was based on the assumption that the basalt/sandstone contact was laterally consistent and continuous and could be extrapolated



from the control points (boreholes). This assumption was necessary to extend the contact to the west of the known control points and agrees with the geologic model of the ridge. Additional information about the location of the contact is necessary to accurately define the geometry of the basalt and sandstone.

## 6.0 MODELING RESULTS AND DISCUSSION

### 6.1 Sandstone Window Model

#### 6.1.1 Results

Modeling results are summarized in Table 1. The water level contour map predicted by VMODFLOW is shown in Figure 11 and in cross sections in Figure 12. Water levels vary between the two defined constant-head boundaries of 6,060 and 5,880 feet elevation across the modeled area. Flow occurs generally to the west from the proposed reservoir to the river. Seepage into the river is estimated at 0.09 cubic feet per second (cfs) (7551.3 ft<sup>3</sup>/day) across the entire interface between the sandstone window aquifer and the river. The base flow of the North Fork of the White River has been estimated by MM/CSSA as 120 cfs (Morrison Maierle, Inc. 1986). The potential loss from the reservoir is estimated to be less than 0.1 percent of the base flow, an insignificant loss.

#### 6.1.2 Sensitivity Analysis

A sensitivity analysis was conducted to evaluate the effects on flow by changes in the hydraulic conductivity. The results of the analysis are presented in Table 2. The expected hydraulic conductivity represents the geometric mean of packer test results for the sandstone window. The model was re-run using the expected value plus one standard deviation and using the expected value minus one standard deviation. For the sandstone window model, these values are  $1.94 \times 10^{-4}$  cm/sec,  $6.86 \times 10^{-4}$  cm/sec and  $5.5 \times 10^{-5}$  cm/sec for the expected value, plus standard deviation and minus standard deviation, respectively. The sensitivity of the seepage rate to the hydraulic conductivity is linear (in agreement with Darcy's Law). Therefore, an order of magnitude increase in the hydraulic conductivity results in an order of magnitude increase in the seepage flow to the river.

### 6.1.3 Model Verification

The results of the sandstone model were verified by applying Darcy's Law:

$$Q = KiA$$

where:

Q = Flow Rate (ft<sup>3</sup>/day)

K = Hydraulic Conductivity = 0.000194 cm/sec = 0.55 ft/day

i = Hydraulic Gradient = 180 ft per 800 ft = 0.225 ft/ft

A = Cross-sectional Area = 1000 ft length x 35 feet depth = 35,000 ft<sup>2</sup>

resulting in a total flow of 4331 ft<sup>3</sup>/day (0.05 cfs). The calculation and the result is included in Table 2 for comparison with the modeling and sensitivity analysis results.

### 6.1.4 Discussion

The relatively well-understood geometry of the sandstone window allowed construction of a straight-forward model, however, some of the simplifications required for simulating the boundary conditions are uncertain. Consideration of flow from the sandstone to the basalt would refine the model as would the consideration of flow paths through the underlying Supai Group.

## 6.2 Basalt Model

### 6.2.1 Results

The water level contour map of layer two predicted by the model is shown in Figure 11 and in cross sections in Figure 13. Water levels vary between the two defined constant-head boundaries of 6,060 and 5,900 feet elevation across the modeled area. Flow is generally to the west from the proposed reservoir to the river. Seepage into the river is estimated at 0.04

cfs (3,451 ft<sup>3</sup>/day) across the entire interface between the basalt/sandstone aquifer and the saturated interval below the river. This represents a loss of approximately 0.03 percent of the base flow of the North Fork of the White River.

### 6.2.2 Sensitivity Analysis

As with the sandstone window model, a sensitivity analysis was conducted on the basalt model. Results are presented in Table 2. Hydraulic conductivity values were varied by one standard deviation from the mean for both the basalt and the sandstone layers. Simulations were run using the plus-one-standard-deviation values for both layers and the minus-one-standard-deviation values for both layers to estimate maximum and minimum flow rates. For the basalt, the expected conductivity value is  $2.88 \times 10^{-5}$  cm/sec and the plus- and minus-one-standard-deviation values are  $2.28 \times 10^{-4}$  cm/sec and  $3.63 \times 10^{-6}$  cm/sec, respectively. Sandstone values were  $1.13 \times 10^{-4}$  cm/sec,  $9.2 \times 10^{-4}$  cm/sec and  $1.4 \times 10^{-5}$  cm/sec for the expected, plus-one-standard-deviation and minus-one-standard-deviation, respectively.

### 6.2.3 Model Verification

A Darcy's Law calculation was conducted to verify the modeling and results included in Table 2. The following inputs were used:

$$K = \text{Hydraulic Conductivity (sandstone value)} = 0.000113 \text{ cm/sec} = 0.32 \text{ ft/day}$$

$$i = \text{Hydraulic Gradient} = 160 \text{ ft per } 600 \text{ ft} = 0.27 \text{ ft/ft}$$

$$A = \text{Cross-sectional Area} = 600 \text{ ft length} \times 35 \text{ feet depth} = 21,000 \text{ ft}^2$$

resulting in a total flow of 1,814 ft<sup>3</sup>/day (0.021 cfs).

#### 6.2.4 Discussion

The basalt model consisted of two layers, four different hydraulic conductivity values, two imported surfaces with high relief, cell rewetting and two constant head boundaries. When running the model with the cell-rewetting package included, appropriate convergence was not achieved. To achieve convergence, the cell-rewetting package was excluded and the model was run with an original water level of 6,060 feet elevation. Residual heads were damped (*i.e.*, convergence acceleration less than 1) such that cells were not dewatered prematurely.

A substantial amount of the basalt (upper layer) was dry since much of the elevation of the base of the basalt is above the pool elevation of 6,060. The high-conductivity "pipes" showed little effect on the overall resultant water table. The steady-state water table intersects the basalt -- and therefore the "pipes" -- just to the east of the river, thereby minimizing the potential high-flow effects of the "pipes" and defining the sandstone as the controlling aquifer in the system.

## 7.0 CONCLUSIONS

The ridge that forms the left or southeastern abutment of the proposed Miner Flat dam is composed of Coconino Sandstone that is overlain by Quaternary-Tertiary basalt and up to 50 feet of Quaternary-Tertiary gravel. Eight boreholes have been cored along the axis of the ridge and packer tests have been performed in the holes to investigate the potential for seepage through the ridge. These core holes show that the thickness of the basalt overlying the Coconino Sandstone is variable and that the eastern and western extent of the basalt is unknown as well as the extent of the basalt to the south. Rock quality indicators such as RQD and fracture frequency show that the basalt is of fair quality and not intensely fractured. Packer tests performed in the holes indicated that the sandstones of the Coconino and Supai formations have a permeability of approximately  $10^{-4}$  cm/sec. The permeability of the basalt is dependent on the number of hydraulically connected fractures intersected by the test interval. The majority of the packer tests performed in the basalt indicated a discrete feature hydraulic conductivity of  $10^{-5}$  cm/sec.

Twenty six additional boreholes have been drilled through the Quaternary-Tertiary gravel to determine the elevation of the bedrock and the bedrock type. The purpose of these additional holes was to investigate the potential of seepage through the surficial material overlying the bedrock. The drilling of the surficial materials has shown that the thickness of the Quaternary-Tertiary gravel deposits are variable and that a broad, flatly dipping bedrock surface exists about halfway along the axis of the ridge. The "gravel" deposits are primarily clay and the concentration of sands, gravels, and cobbles at the surface are lag deposits left by the removal of the finer silts and clays by erosion. The elevation of the flatly dipping bedrock surface is below the proposed pool elevation of 6,060 and could present a possible seepage pathway. Laboratory testing of the surficial materials collected from boreholes indicate that the permeability of these materials is approximately  $10^{-7}$  cm/sec.

Two hydrogeologic models were prepared to estimate the seepage losses through the bedrock of the ridge. The sandstone model representing the portion of the ridge that does not have a basalt cover is the most straight forward with the fewest simplifying assumptions. The results of the seepage modeling through the sandstone portion of the ridge indicate that the 1,000 feet of discharge length may yield approximately 0.09 cfs (7800 ft<sup>3</sup> per day) with a reservoir pool elevation of 6,060. The basalt model representing the portion of the ridge that is covered with the basalt over the sandstone is much more complicated with considerably more simplifying assumptions. The results of the seepage modeling through the sandstone/basalt portion of the ridge indicate that the 600 feet of discharge length may yield 0.04 cfs (3,450 ft<sup>3</sup> per day). The total seepage through the ridge is estimated to be approximately 0.13 cfs (11,000 ft<sup>3</sup> per day). Given that the base flow of the North Fork of the White River is estimated as 120 cfs the loss from the reservoir through the left abutment ridge is estimated at approximately 0.1 percent, an insignificant loss.

## 8.0 RECOMMENDATIONS

The purpose and scope of the left abutment seepage modeling were to investigate the potential for seepage through the Permian sandstones of the left abutment ridge. The scope was expanded from the original work plan to include the basalt section of the left abutment area and the seepage potential of the surficial material. The following recommendations are made in order to answer the unresolved potential seepage related to the operation of the proposed Miner Flat dam.

The investigation of the seepage potential through the surficial material covering the bedrock of the ridge requires only minor additional work at this time. Boreholes that have not been surveyed, those boreholes with the MF-B prefix, should be surveyed. Borehole location data would then be included in the bedrock surface model and a new bedrock surface would be estimated. The final bedrock surface map would bring to a close the present scope of work. It is recommended that prior to construction of the dam that the eastern extent of the basalt be determined and that in situ permeability testing of the surficial materials be performed. A design of seepage mitigation alternatives, if required, cannot be completed with the current information.

The investigation of the potential seepage through the Permian sandstone, in particular the Coconino Sandstone where it is not blanketed by basalt, is essentially complete at this time. The 'sandstone window' model should be integrated with the 'basalt' model to fully represent the hydrogeological interrelationship of the left abutment ridge.

The investigation of the seepage potential through the left abutment ridge where it is blanketed by basalt on the western or downstream side is incomplete and requires additional investigation. The seepage potential of the basalt model was not included as part of the original work plan. Additional data and flow modeling pertaining to the discrete feature component of this model is required to fully evaluate the seepage potential of this area. The



eastern and southern extent of the basalt must be determined to fully model the seepage potential. The western or downstream geometry of the basalt must be determined as this is the discharge area of the potential seepage through this section of the left abutment ridge. The material between the bottom of the basalt and the sandstone including Tertiary colluvium and Tertiary alluvium has not been investigated, tested, or included in the current model. This potential flow path must be included in a more inclusive model. The 'sandstone window' model must be included in a more comprehensive analysis of the seepage potential of the left abutment ridge.

The two discreet areas modeled for this investigation represent two of many possible seepage pathways that emanate from the proposed reservoir. They do not include: the specific area of the dam abutments or foundation; the north side of the reservoir through the basalt and Tertiary alluvium; or, seepage to the east of the sandstone window. Due to the complex nature of the regional and local geology, more detailed field and analytical investigations should be conducted, including:

- ▶ A seepage model of the dam abutments and foundation
- ▶ An investigation, including modeling, of the hydrogeologic properties and extent of Tertiary alluvium/colluvium deposits below the foundation and abutments and to the west
- ▶ A stream survey to define losing and gaining reaches of the North Fork of the White River and to define stream losses through the Tertiary alluvium
- ▶ Expanded data collection, in support of refining existing conceptual and numerical models of the basalt
- ▶ A large-scale seepage model incorporating existing model information and additional data on other potential flow pathways
- ▶ Exhaustive water-level data for all piezometers and wells in the area
- ▶ Hydrogeologic and geologic information on areas north of the proposed dam and east of the sandstone window including water levels, hydraulic and flow parameters, and structural, stratigraphic and lithological data.

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## TABLES

**TABLE 1**  
**SEEPAGE MODELING AND SENSITIVITY ANALYSIS**  
**MINER FLAT DAM SITE - ARIZONA**

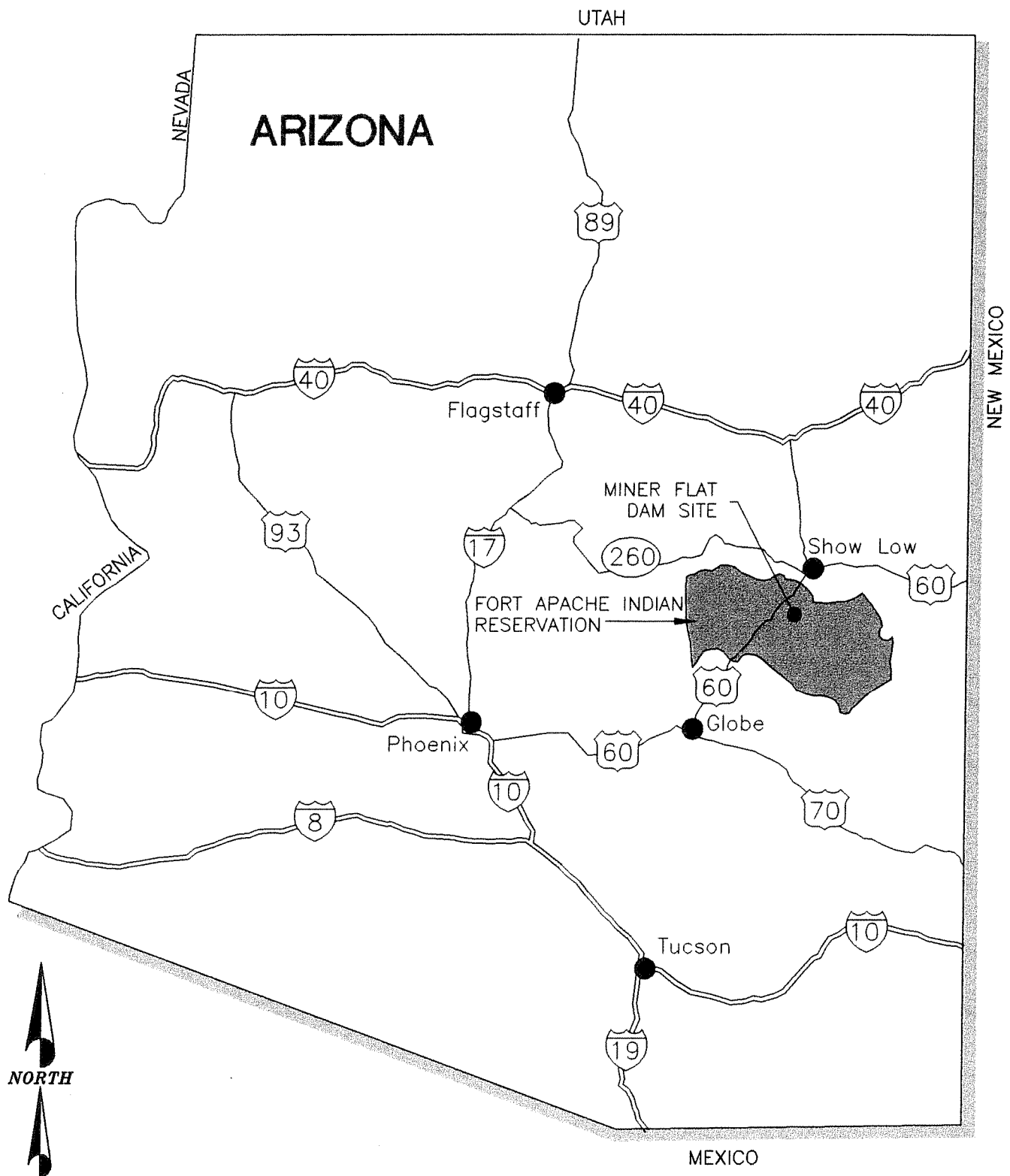
**Model Output Summary**

	<b>Seepage Rate (cfs)</b>	<b>Discharge Width (feet)</b>	<b>Flow per foot (cfs/foot)</b>
Sandstone Window	0.087	1000	8.70E-05
Basalt	0.040	600	6.65E-05
<b>Total</b>	<b>0.127</b>	<b>1600</b>	<b>7.94E-05</b>

**TABLE 2**  
**SEEPAGE RATES AT VARYING HYDRAULIC CONDUCTIVITY VALUES**  
**MINER FLAT DAM SITE - ARIZONA**

Model	Expected Value		+ 1 Standard Deviation		- 1 Standard Deviation		Darcy Calculation	
	K Value cm/sec	Seepage Rate (cfs)	K Value cm/sec	Seepage Rate (cfs)	K Value cm/sec	Seepage Rate (cfs)	K Value cm/sec	Seepage Rate (cfs)
Sandstone Window	1.94E-04	0.087	6.86E-04	0.309	5.50E-05	0.025	1.94E-04	0.050
Basalt	2.88E-05	0.040	2.28E-04	0.334	1.13E-04	0.005	2.88E-05	0.021
<b>Total</b>	<b>na</b>	<b>0.127</b>	<b>na</b>	<b>0.643</b>	<b>na</b>	<b>0.030</b>	<b>na</b>	<b>0.071</b>

**FIGURES**



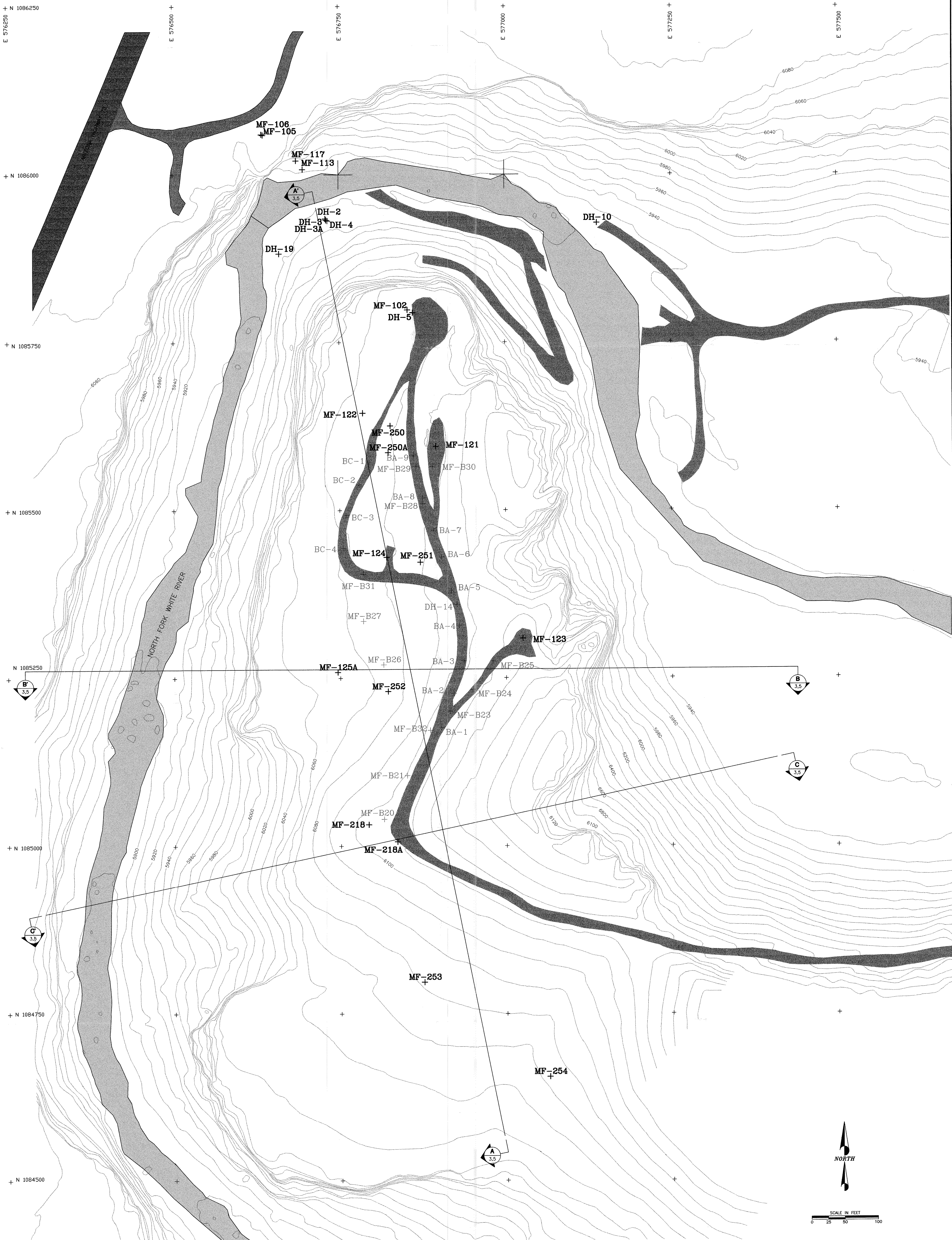
Golder Associates Inc.  
 200 Union Blvd. Suite 500  
 Lakewood, Colorado 80228  
 Phone: (303) 980-0540  
 FAX: (303) 985-2080

TITLE  
**SITE LOCATION  
 MAP**

CLIENT/PROJECT  
**MORRISON-MAIERLE / CSSA  
 MINER FLAT DAM**

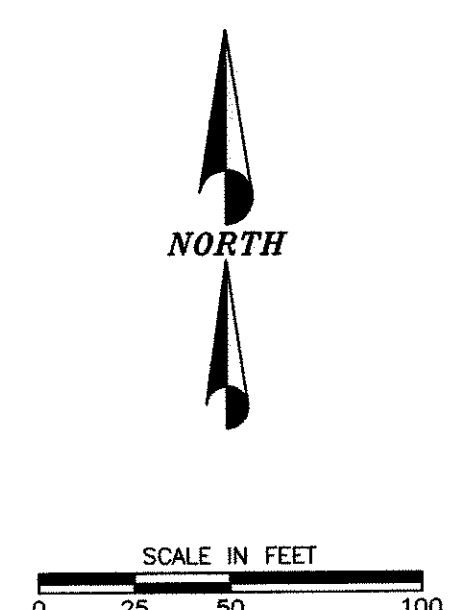
DRAWN	KAW	DATE	APRIL 1997	JOB NO.	943-2769
CHECKED	LB	SCALE	NTS	DWG NO./REV. NO.	A006
REVIEWED	CHR	FILE NO.	2769A006	FIGURE NO.	<b>1</b>





**EXPLANATION**

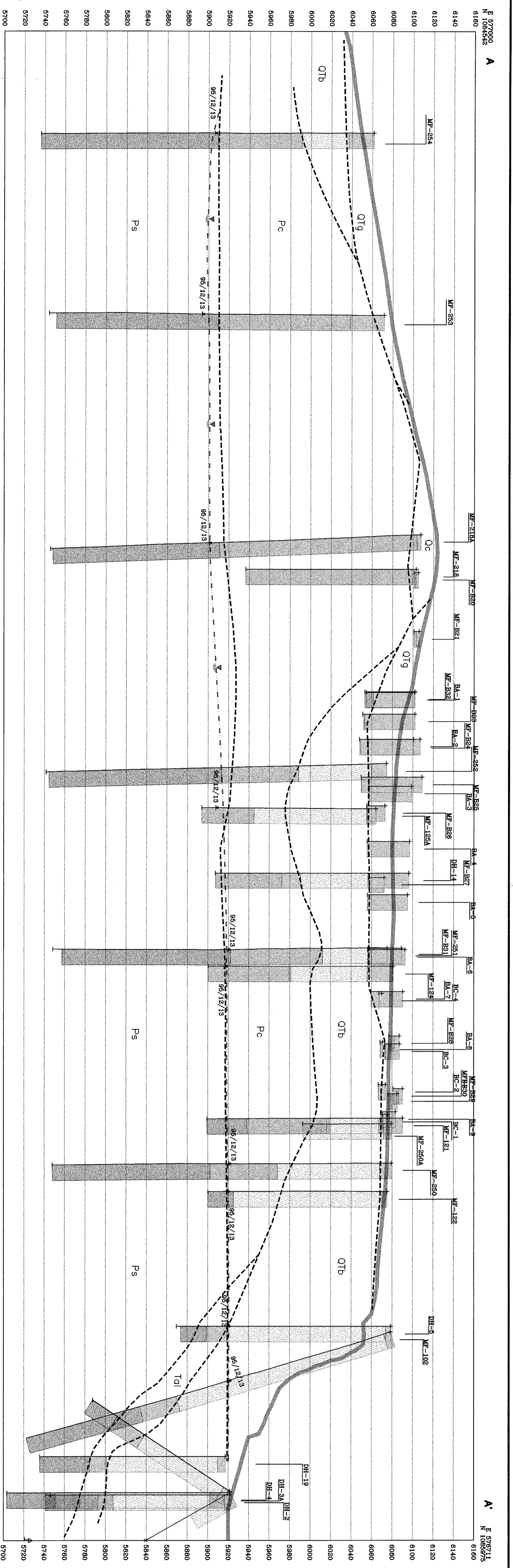
- |  |                          |          |  |
|--|--------------------------|----------|--|
|  | NORTH FORK WHITE RIVER   |          | BOREHOLE COLLAR LOCATIONS                          |
|  | SITE ACCESS ROAD         | MF-B26 + | PERCUSSION HAMMER BOREHOLE (APPROXIMATELY LOCATED) |
|  | ARIZONA STATE HIGHWAY 73 | BA-3 +   | PERCUSSION HAMMER BOREHOLE                         |
|  |                          | BC-4 +   | PERCUSSION HAMMER BOREHOLE                         |
|  |                          | DH-14 +  | COREHOLE   |
|  |                          | MF-102 + | COREHOLE   |



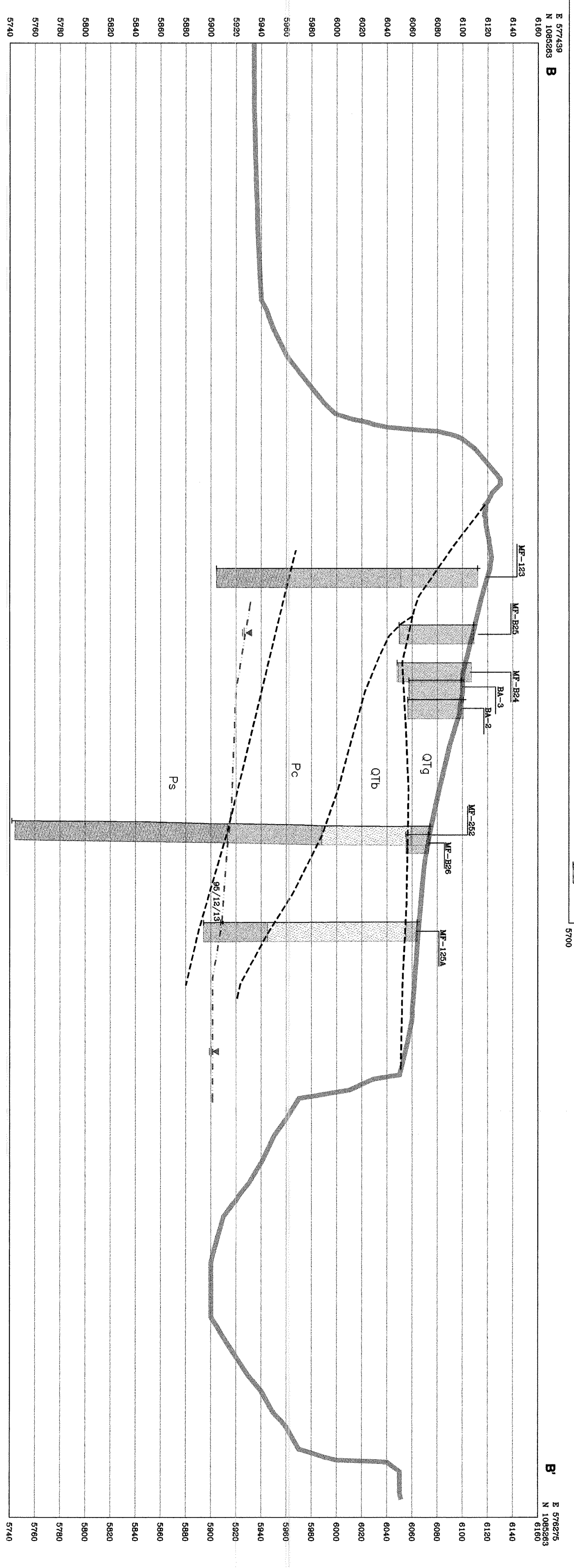
CLIENT/PROJECT		MORRISON-MAIERLE / CSSA MINER FLAT DAM	
TITLE		BOREHOLE LOCATION MAP	
Denver, Colorado		DRAWN	KAW
		CHECKED	LB
		REVIEWED	CHR
		APPROVED	CHR
		DATE	APRIL 1997
		SCALE	AS SHOWN
		JOB NO.	943-2769
		DWG. NO.	E007
		FILE NO.	2769E007
		FIGURE	2

REV.	DATE	DESCRIPTION	DR. BY	APP. BY

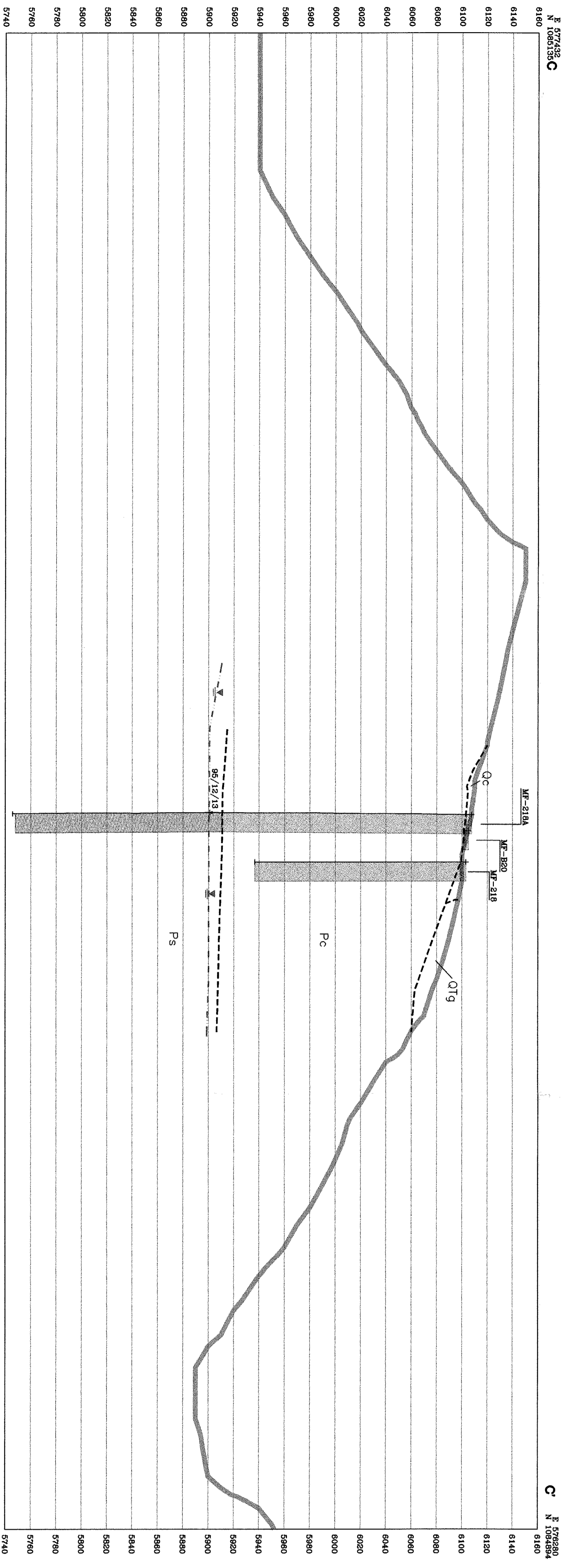




**A** CROSS SECTION A-A'  
 SCALE IN FEET  
 0 25 50 100



**B** CROSS SECTION B-B'  
 SCALE IN FEET  
 0 25 50 100



**C** CROSS SECTION C-C'  
 SCALE IN FEET  
 0 25 50 100

**EXPLANATION**

95/12/13	WATER LEVEL MEASURED DEC. 13, 1995
---	PERCUSSION COLLAR LOCATIONS
MF-826	PERCUSSION HAMMER BOREHOLE (APPROXIMATELY LOCATED)
BA-3	PERCUSSION HAMMER BOREHOLE
BC-4	PERCUSSION HAMMER BOREHOLE
DH-14	OREHOLE
MF-102	OREHOLE
OTb	QUATERNARY CLAY/LOAM, QUATERNARY-TEMPERARY
OTg	GRAVEL, TEMPORARY ALLUVIUM
Pc	QUATERNARY TEMPORARY DEPOSIT
Ps	COCONINO FORMATION
	SUPRA GROUP

**MORRISON-MARBLE / CSSA**  
**MINNER FLAT DAM**  
**GEOLOGIC CROSS SECTIONS**

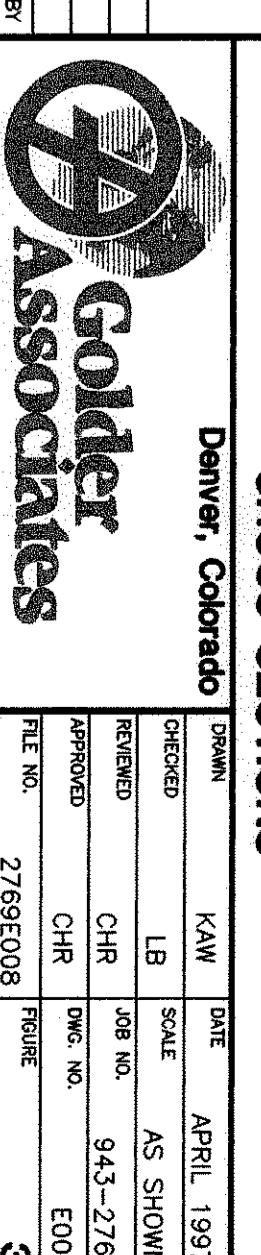
DESIGN/PROJECT: MORRISON-MARBLE / CSSA  
 MINNER FLAT DAM  
 GEOLOGIC CROSS SECTIONS

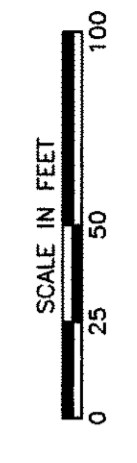
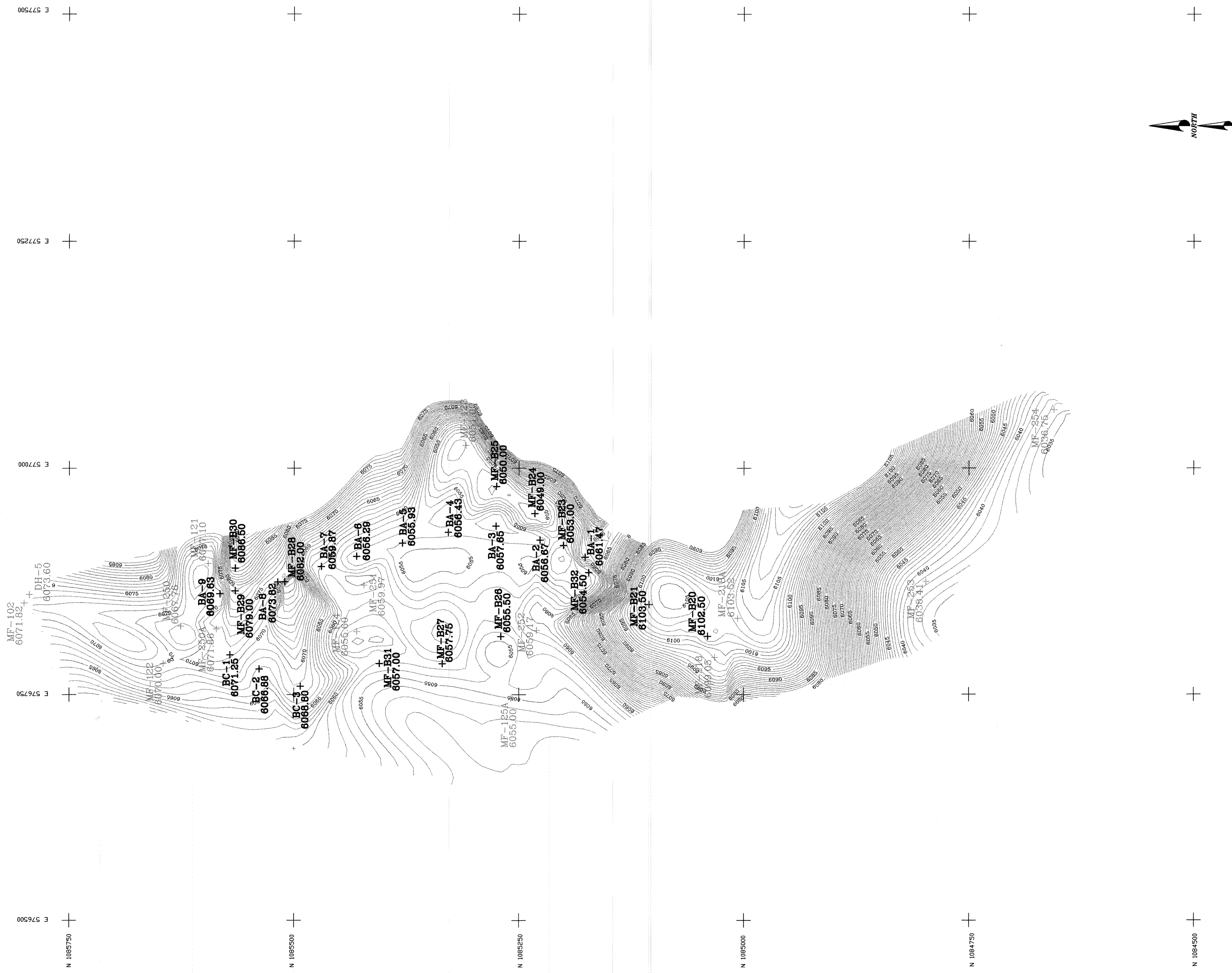
DENVER, COLORADO

DATE	APRIL 1997
BY	KAW
SCALE	AS SHOWN
JOB NO.	943-2769
PROJECT	MINNER FLAT DAM
DATE	APRIL 1997
BY	KAW
SCALE	AS SHOWN
JOB NO.	943-2769
PROJECT	MINNER FLAT DAM

REV. DATE DESCRIPTION


REV. DATE DESCRIPTION



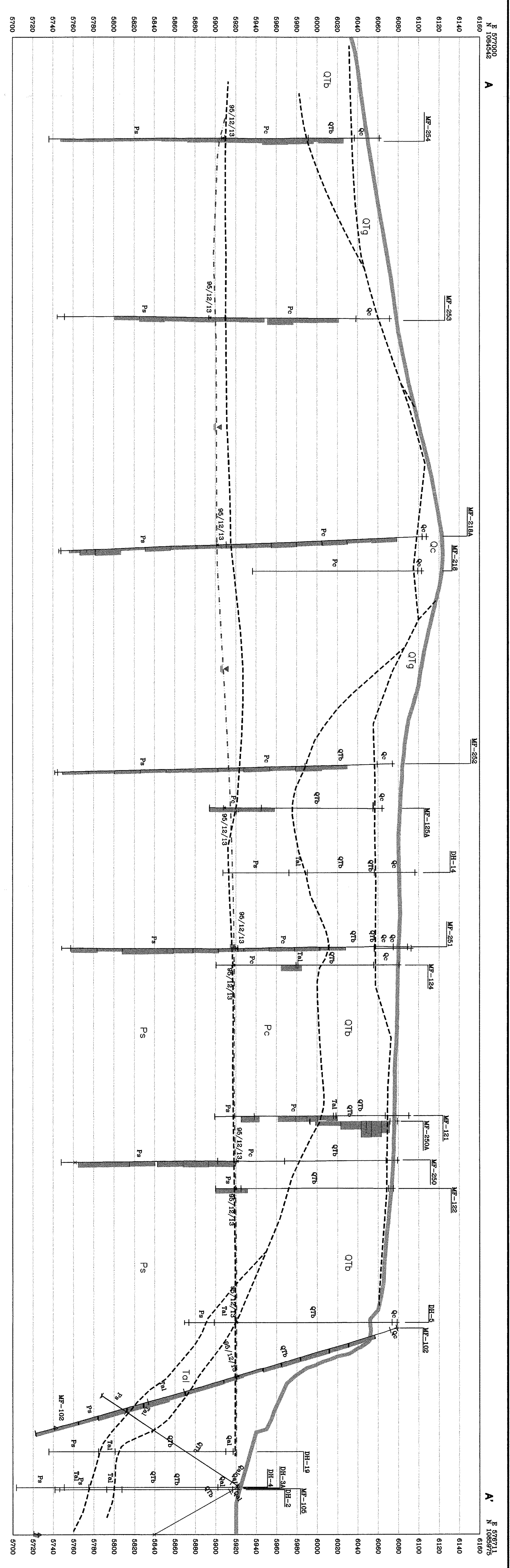


**EXPLANATION**

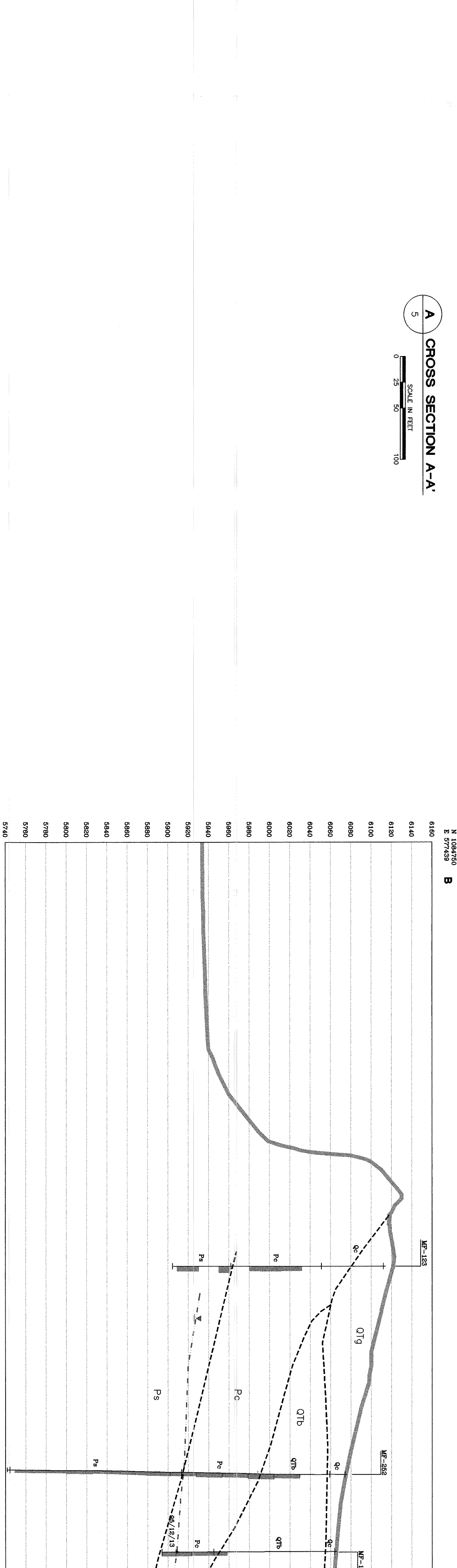
- BA-6**  
6056.29 + PERCUSSION HAMMER BOREHOLE  
BEDROCK ELEVATION
  - MF-251**  
6059.97 + COREHOLE BEDROCK  
ELEVATION
  - MF-B23**  
6053.00 + PERCUSSION HAMMER BOREHOLE  
BEDROCK ELEVATION
- Triangular Grid from 80 data points

<b>MORRISON-MAIERLE / CSSA MINER FLAT DAM</b>	
<b>Bedrock Surface Topography</b>	Denver, Colorado
<b>Golder Associates</b>	DATE: APRIL 1997
CHECKED: _____	SCALE: AS SHOWN
REVIEWED: _____	JOB NO. 943-2769
APPROVED: _____	CHR: _____
FILE NO. 276BED10	FIGURE NO. E010

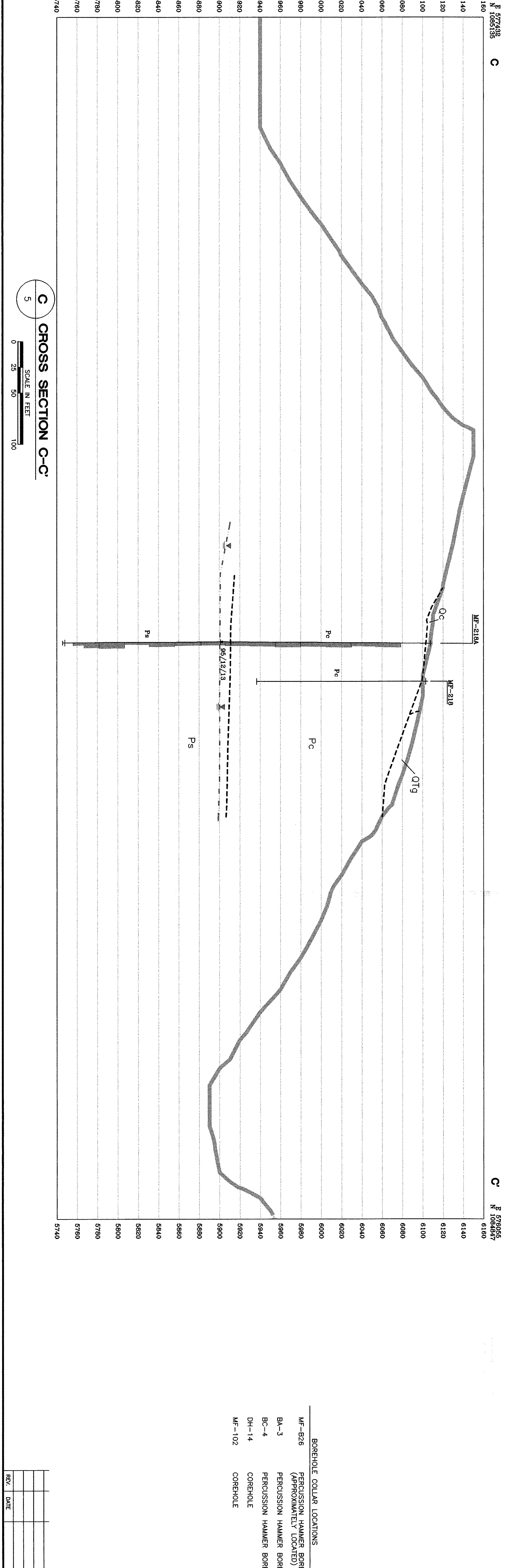
REV.	DATE	DESCRIPTION	DR. BY	APP. BY



**A** CROSS SECTION A-A'



**B** CROSS SECTION B-B'



**C** CROSS SECTION C-C'

**EXPLANATION**

MF-254	IDENTIFICATION LABEL
95/12/13	WATER LEVEL MEASURED DEC. 13, 1995
- - - - -	REZONING SURFACE
Oc	QUATERNARY-COLLUVIUM
Otg	QUATERNARY-TERTIARY GRAVEL
Tal	TERTIARY ALUMINUM
Qtb	QUATERNARY TERTIARY BEALTI
Pc	COCONINO TERTIARY BEALTI
Ps	SIPAI GROUP

MF-254

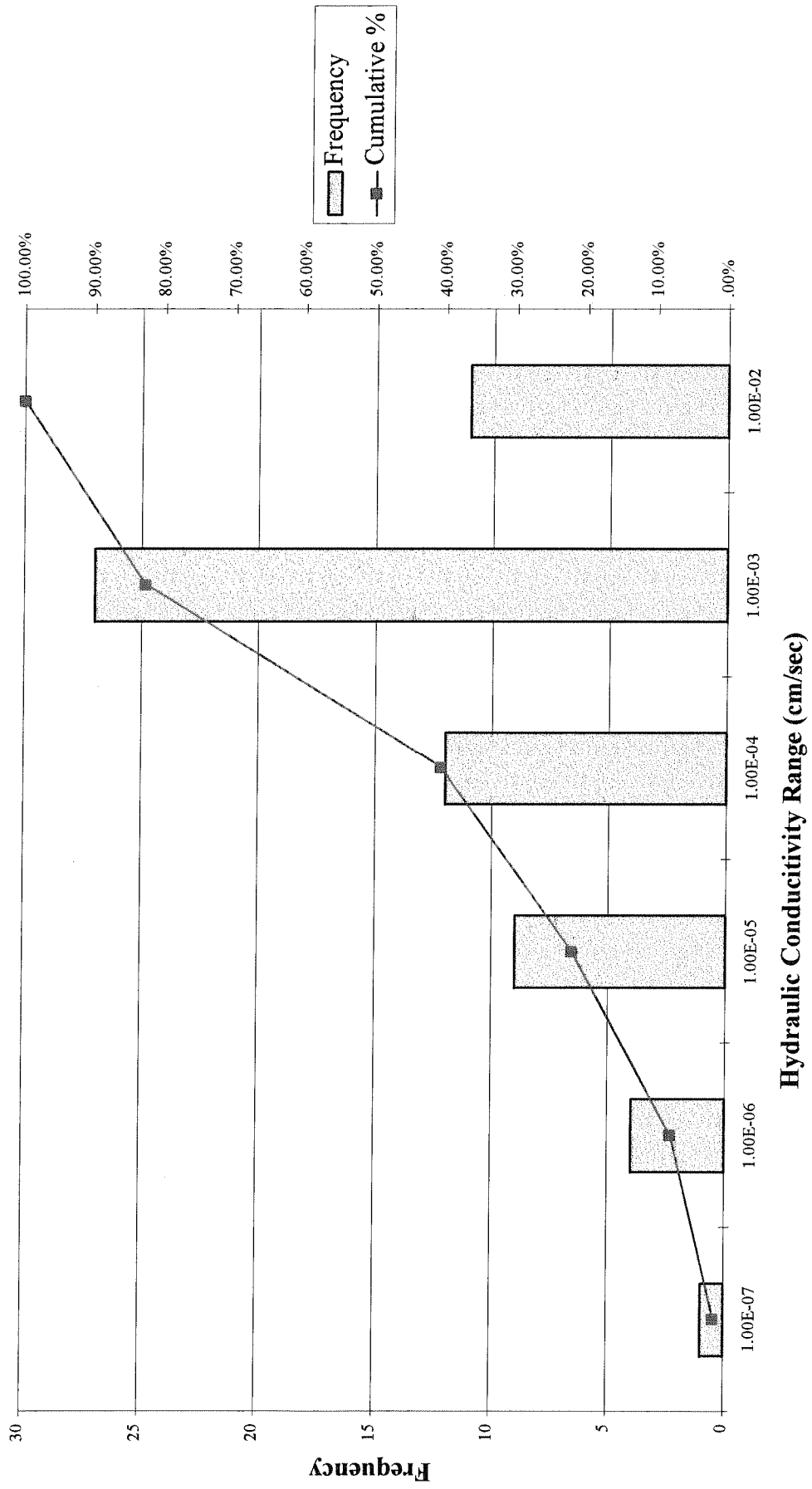
Hydraulic Conductivity from Packer Tests

**MORRISON-MARLE / OSSA**  
**MINER FLAT DAM**  
**HYDRAULIC CONDUCTIVITIES**  
**CROSS SECTIONS**  
 Denver, Colorado

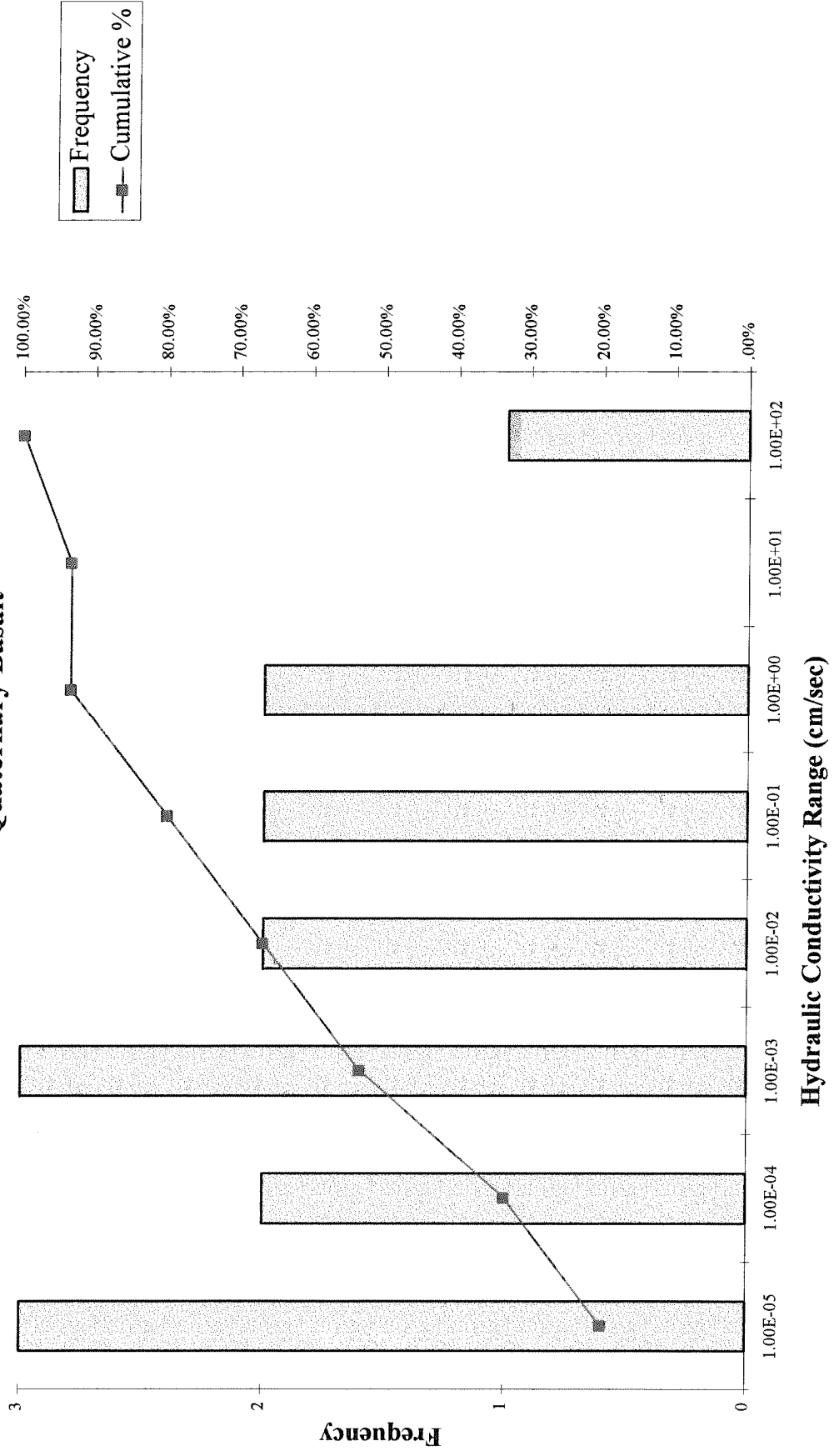
DATE	APRIL 1997
SCALE	AS SHOWN
PROJECT NO.	943-2769
CONTRACT NO.	E009
DATE	APRIL 1997
SCALE	AS SHOWN
PROJECT NO.	943-2769
CONTRACT NO.	E009

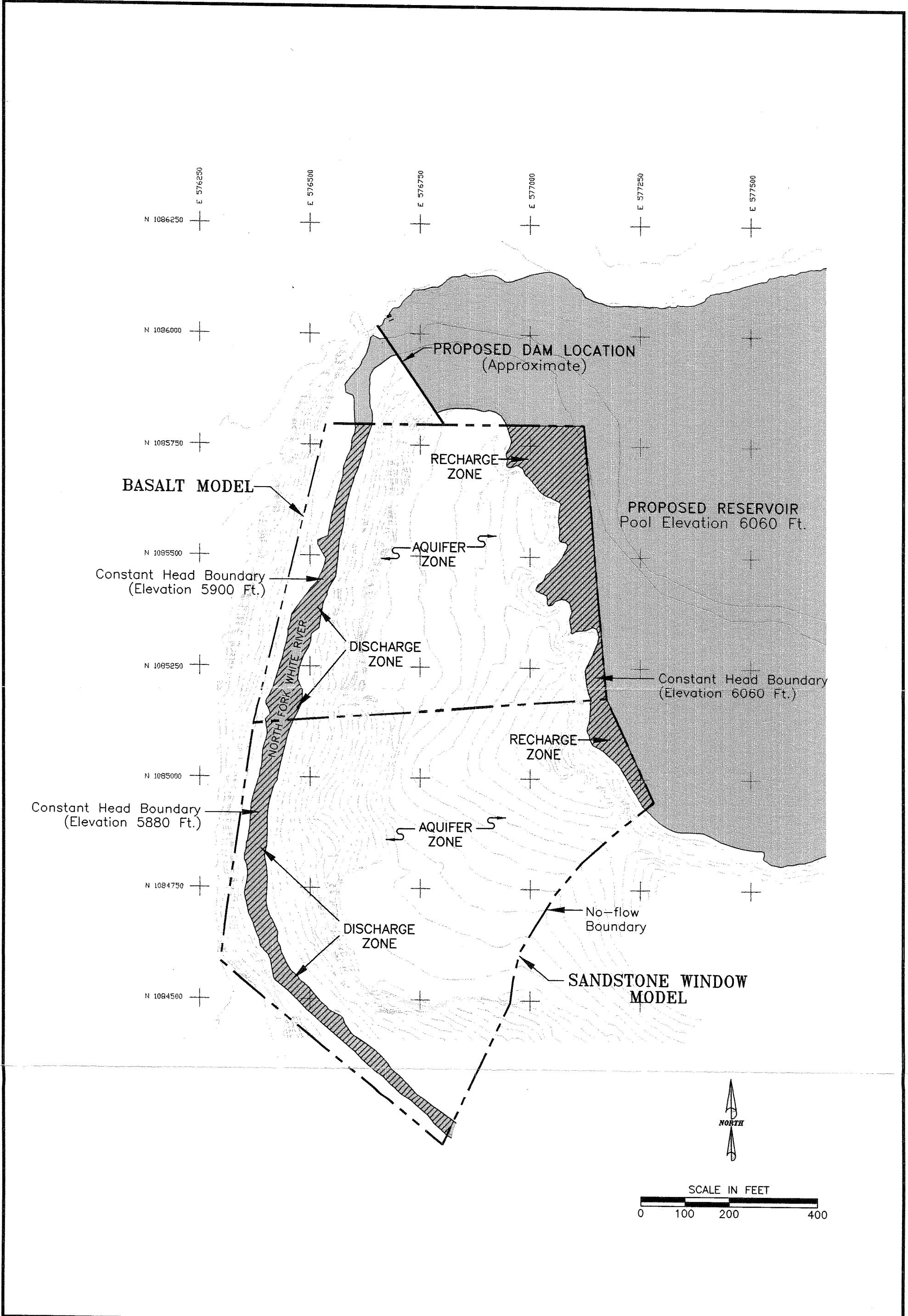
**Goldier Associates**


**Figure 6**  
**Histogram**  
**Hydraulic Conductivity Values**  
**Permian Sandstones**

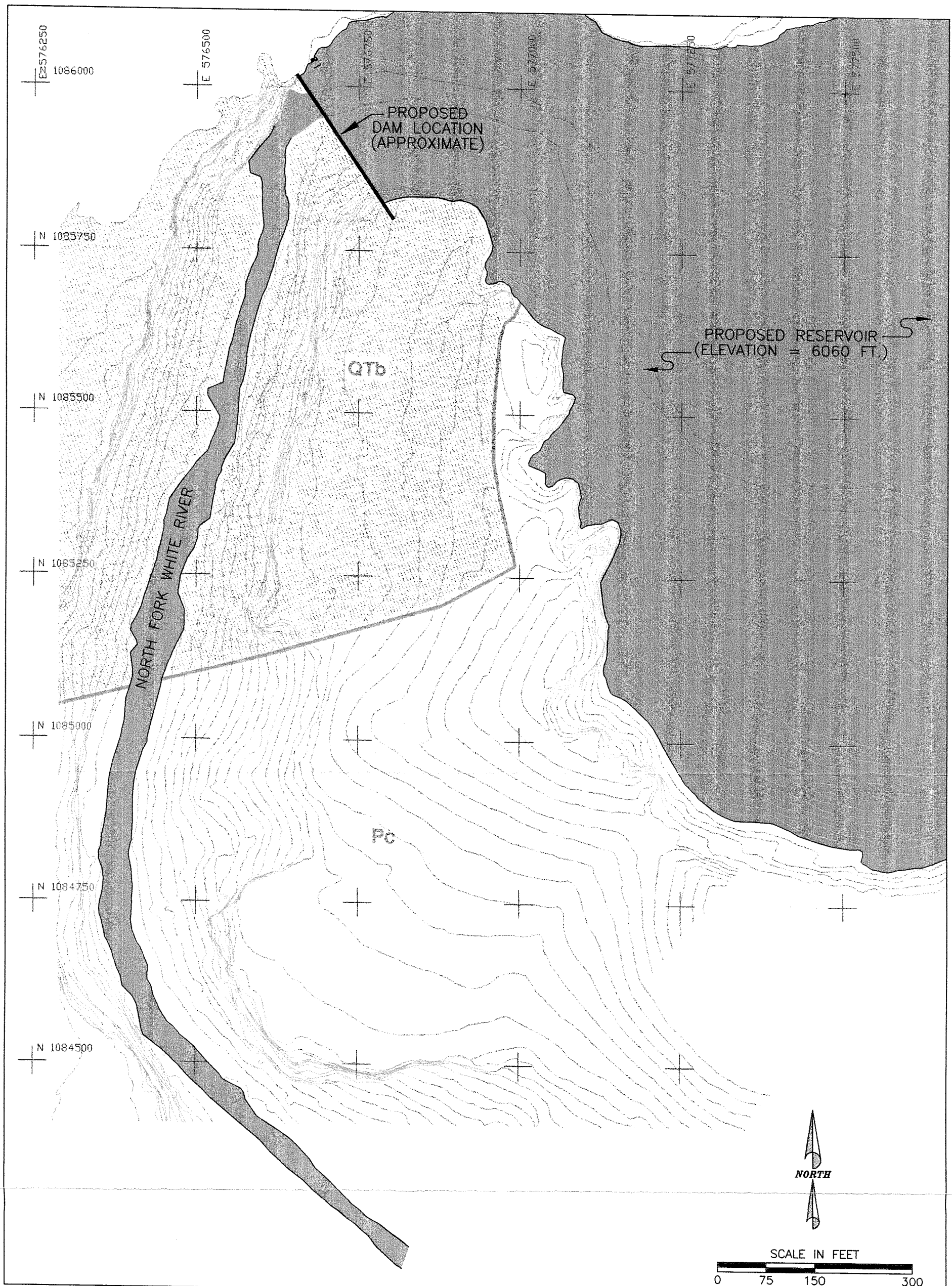


**Figure 7**  
**Histogram**  
**Hydraulic Conductivity Values**  
**Quaternary Basalt**

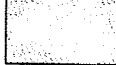






CLIENT/PROJECT			 Denver, Colorado			TITLE		
<b>MORRISON-MAIERLE / CSSA                  MINER FLAT DAM</b>						<b>SEEPAGE ANALYSIS                  MODEL DEFINITION</b>		
DRAWN	CHECKED	REVIEWED	DATE	SCALE	FILE NO.	JOB NO.	DWG NO./REV.NO.	FIGURE
KAW	BCJ	CHR	APRIL 1997	AS SHOWN	2769B022	943-2769	B022	8

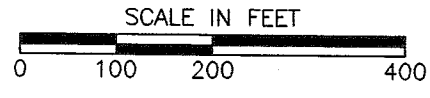
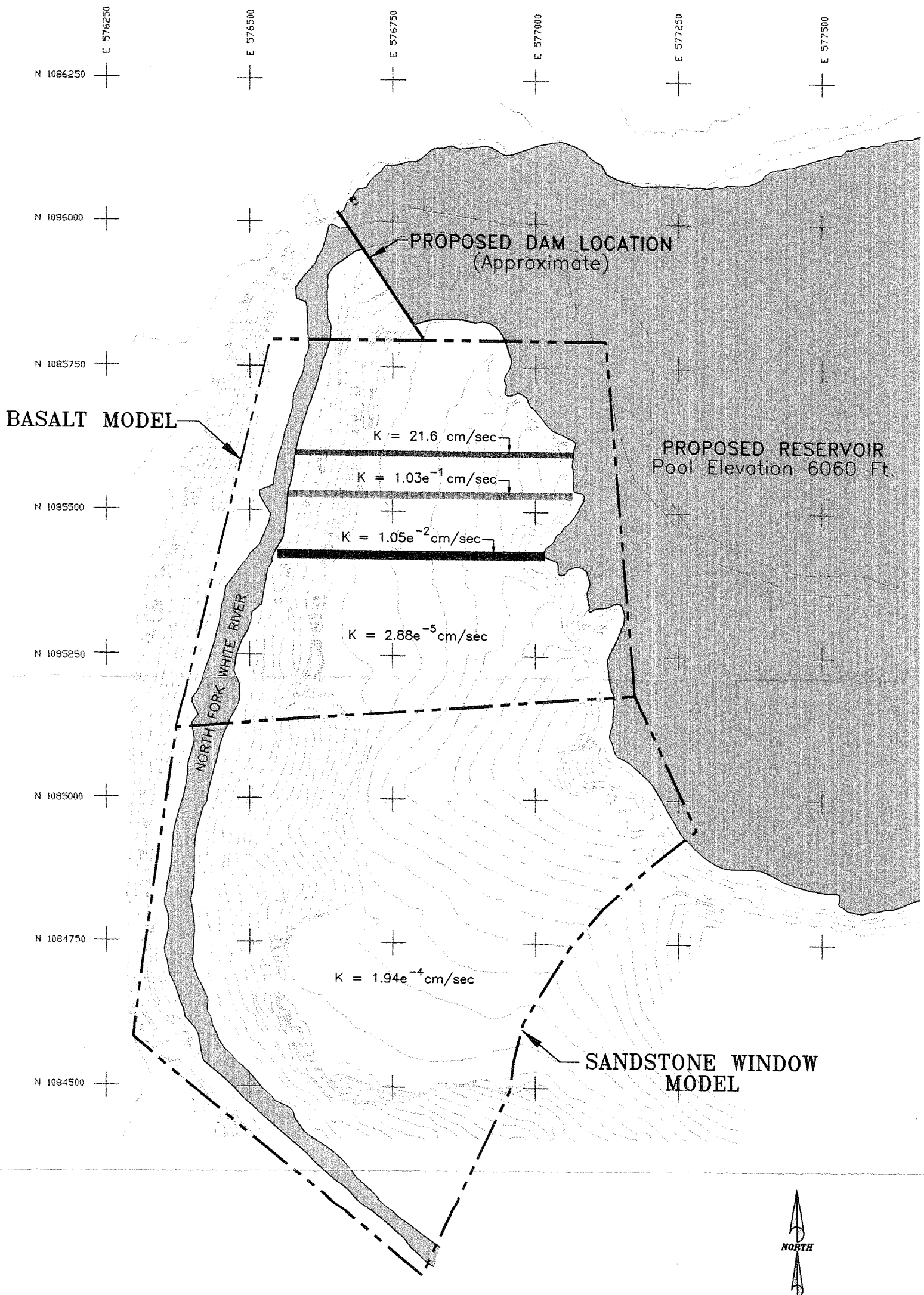



**EXPLANATION**

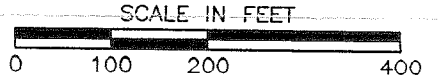
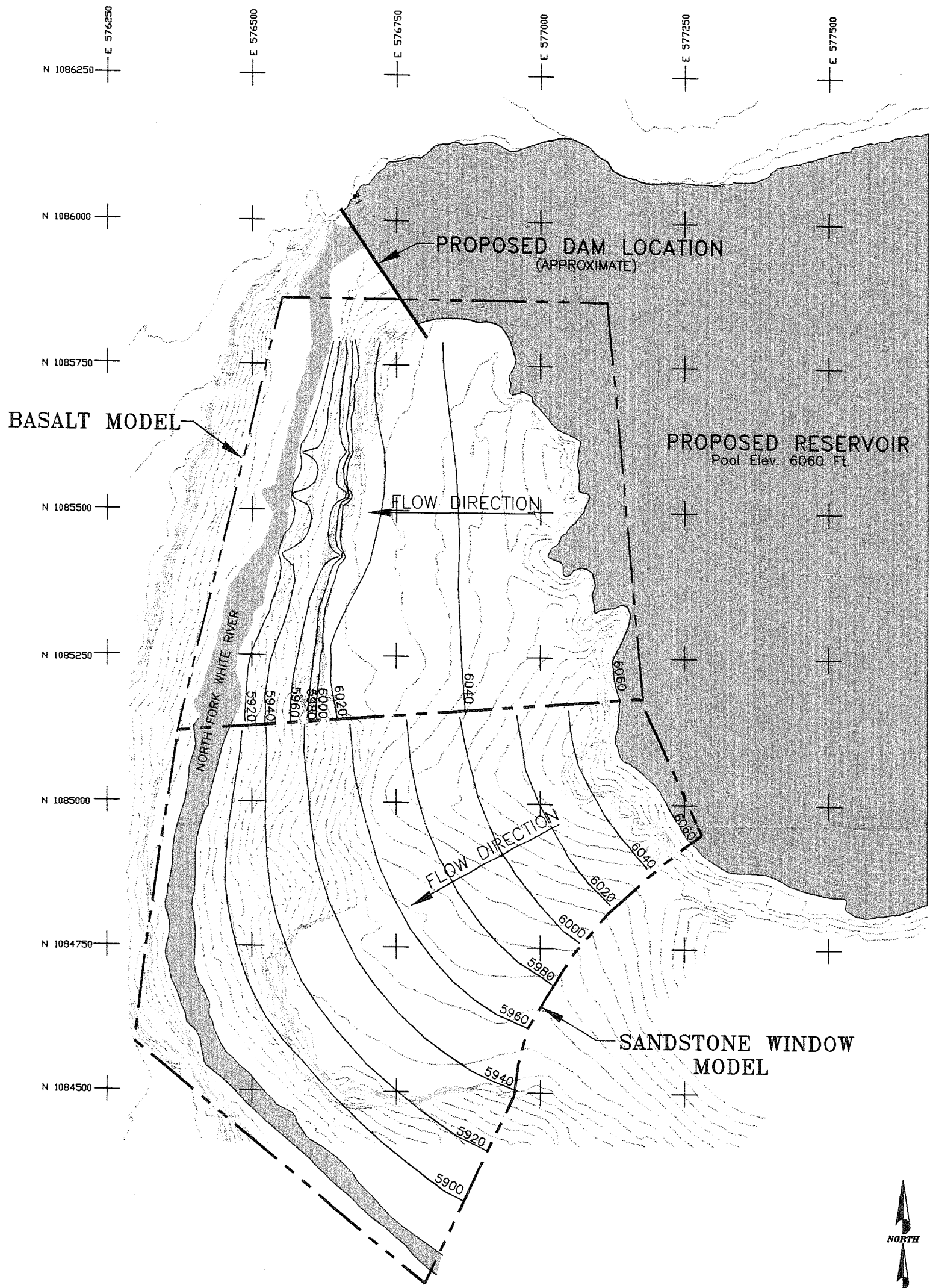
	QTb - QUATERNARY BASALT
	Pc - COCONINO FORMATION


CLIENT/PROJECT			 <b>Golder Associates</b> Denver, Colorado			TITLE		
<b>MORRISON-MAIERLE / CSSA</b> <b>MINER FLAT DAM</b>						<b>GENERALIZED</b> <b>GEOLOGY MAP</b>		
DRAWN	CHECKED	REVIEWED	DATE	SCALE	FILE NO.	JOB NO.	DWG NO./REV.NO.	FIGURE
KAW	BCJ	CHR	APRIL 1997	AS SHOWN	2769B021	943-2769	B021	<b>9</b>





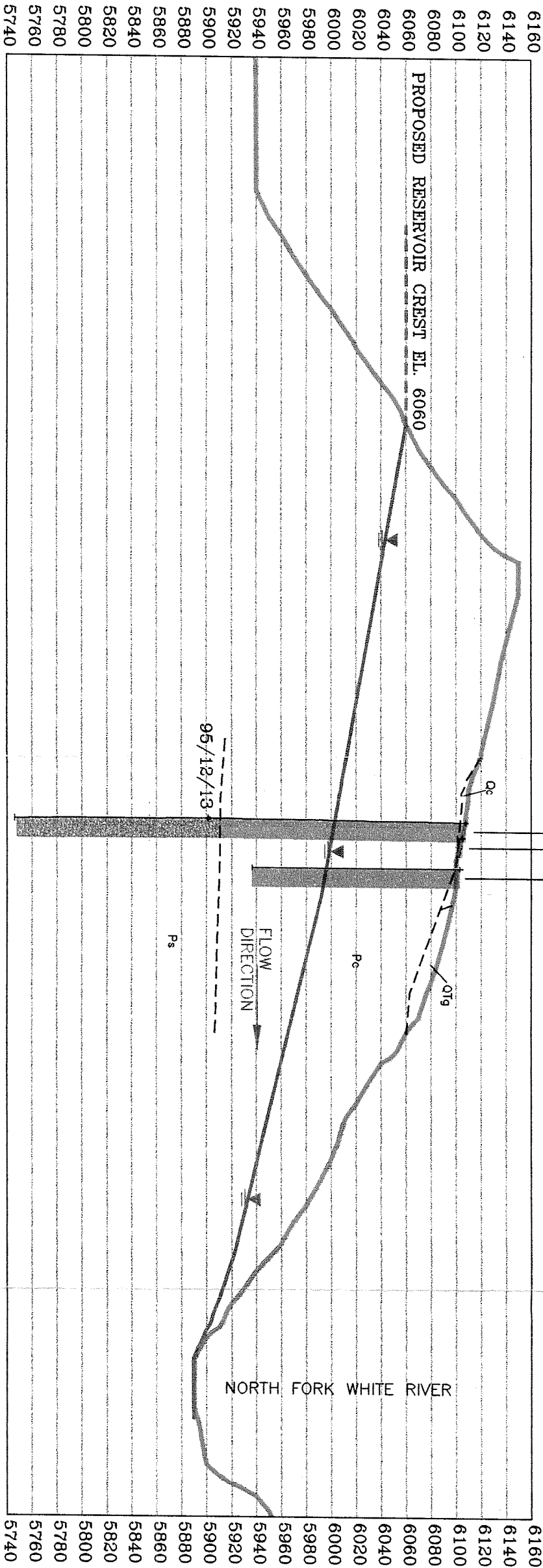
CLIENT/PROJECT			 <b>Golder Associates</b> Denver, Colorado			TITLE		
<b>MORRISON-MAIERLE / CSSA                  MINER FLAT DAM</b>						<b>SEEPAGE ANALYSIS                  MODEL INPUT</b>		
DRAWN	CHECKED	REVIEWED	DATE	SCALE	FILE NO.	JOB NO.	DWG NO./REV.NO.	FIGURE
KAW	BCJ	CHR	APRIL 1997	AS SHOWN	2769B023	943-2769	B023	10



CLIENT/PROJECT <b>MORRISON-MAIERLE / CSSA MINER FLAT DAM</b>			 Denver, Colorado			TITLE <b>SEEPAGE ANALYSIS MODEL OUTPUT</b>		
DRAWN KAW	CHECKED BCJ	REVIEWED CHR	DATE APRIL 1997	SCALE AS SHOWN	FILE NO. 2769B024	JOB NO. 943-2769	DWG NO./REV.NO. B024	FIGURE <b>11</b>

C  
 E 577432  
 N 1085136

C  
 E 576055  
 N 1084847

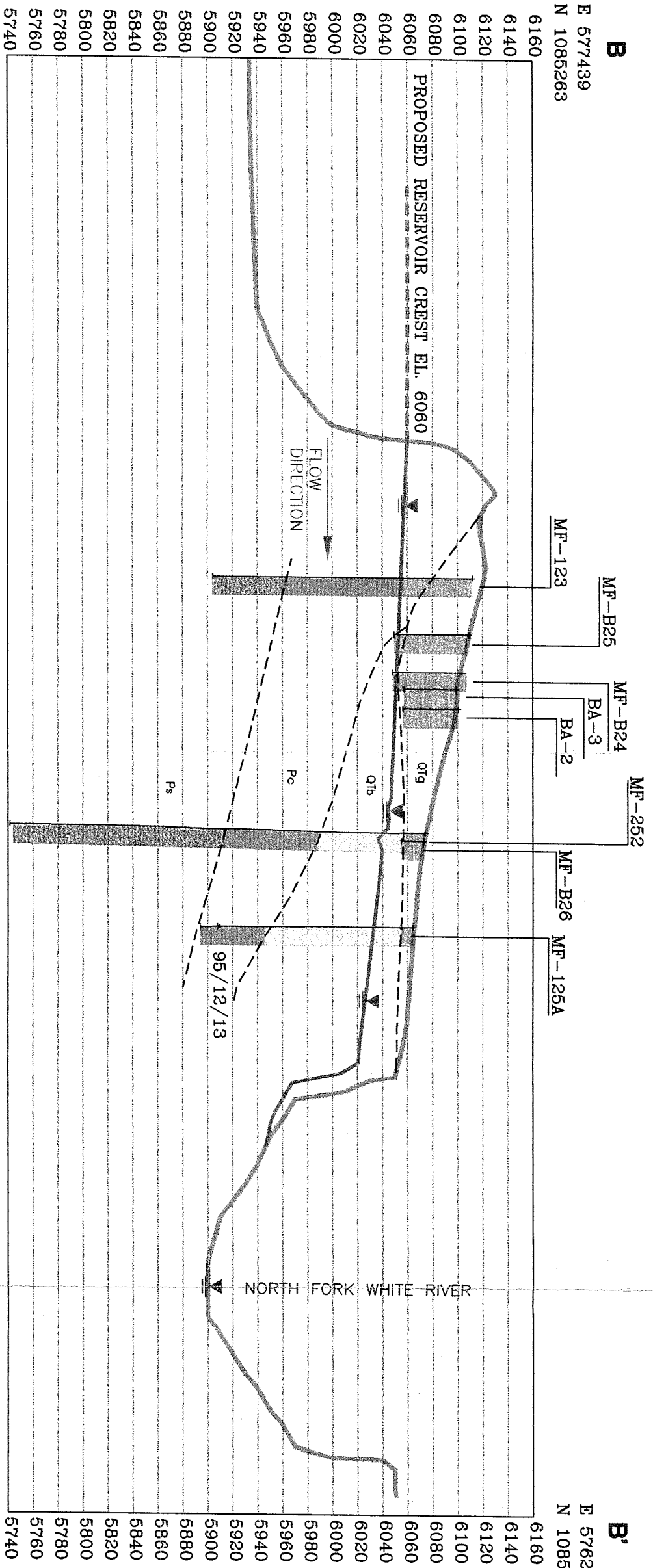


**EXPLANATION**

MF-B26	PERCUSSION HAMMER BOREHOLE (APPROXIMATELY LOCATED)	95/12/13	WATER LEVEL MEASURED DEC. 13, 1995
BA-3	PERCUSSION HAMMER BOREHOLE		PIEZOMETRIC SURFACE
BC-4	PERCUSSION HAMMER BOREHOLE		QUATERNARY COLLUVIUM, QUATERNARY-TERTIARY GRAVEL, TERTIARY ALLUVIUM
DH-14	COREHOLE		QUATERNARY TERTIARY BASALT
MF-102	COREHOLE		COCONINO FORMATION
			SUPAI GROUP



		CLIENT/PROJECT <b>MORRISON-MAIERLE / CSSA</b> <b>MINER FLAT DAM</b>	
		TITLE <b>SANDSTONE WINDOW MODEL</b> <b>ESTIMATED STEADY-STATE GROUND-</b> <b>WATER LEVEL CROSS-SECTION</b>	
DRAWN	KAW	DATE	APRIL 1997
CHECKED	BCJ	SCALE	AS SHOWN
REVIEWED	CHR	FILE NO.	2769B018
		JOB NO.	943-2769
		DWG. NO./REV. NO.	B018
		FIGURE NO.	<b>12</b>



**EXPLANATION**

- BOREHOLE COLLAR LOCATIONS**
- MF-B26 PERCUSSION HAMMER BOREHOLE (APPROXIMATELY LOCATED)
  - BA-3 PERCUSSION HAMMER BOREHOLE
  - BC-4 PERCUSSION HAMMER BOREHOLE
  - DH-14 COREHOLE
  - MF-102 COREHOLE
- 95/12/13**
- WATER LEVEL MEASURED DEC. 13, 1995
  - PIEZOMETRIC SURFACE
- EXPLANATION**
- Qc/Q1g/Td1 QUATERNARY COLLUVIUM, QUATERNARY-TERTIARY GRAVEL, TERTIARY ALLUVIUM
  - Q1b QUATERNARY TERTIARY BASALT
  - Pc COCONINO FORMATION
  - Ps SUPAI GROUP



<p><b>Golder Associates</b></p>		<p>CLIENT/PROJECT</p> <p><b>MORRISON-MAIERLE / CSSA</b></p> <p><b>MINER FLAT DAM</b></p>	
<p><b>BASALT MODEL</b></p> <p><b>ESTIMATED STEADY-STATE GROUND-WATER LEVEL CROSS-SECTION</b></p>		<p>TITLE</p>	<p>DATE</p>
<p>DRAWN</p>	<p>KAW</p>	<p>DATE</p>	<p>APRIL 1997</p>
<p>CHECKED</p>	<p>BCJ</p>	<p>SCALE</p>	<p>AS SHOWN</p>
<p>REVIEWED</p>	<p>CHR</p>	<p>FILE NO.</p>	<p>2769B015</p>
<p>JOB NO. 943-2769</p>		<p>DWG. NO./REV. NO.</p>	<p>B015</p>
<p>FIGURE NO. 13</p>			

**APPENDIX A**

**BOREHOLE LOGS**

# EXPLANATION

**DEPTH:**

The depth of the borehole at the end of each core run.

**WEATHERING/  
ALTERATION:**

A two letter code recording the type and intensity of the rock mass weathering or alteration.

The result of chemical weathering is decomposition of minerals.

Mechanical weathering results in the deterioration of the rock mass

Type

W	Weathering	The susceptibility of the rock or mineral to weathering
A	Argillic	Pertaining to clay or clay minerals
Q	Silicification	The introduction of, or replacement by, silica
B	Calcareous	Contains calcium carbonate

Intensity

F	Fresh	No visible sign of rock material weathering
S	Slightly	Discoloration indicates weathering of rock material and discontinuity surfaces.
M	Moderately	Less than half of the rock material is decomposed and/or disintegrated to a soil.
H	Highly	More than half of the rock material is decomposed and/or disintegrated to a soil.

**ROCK TYPE:**

A six letter code recording; Rock type, Major minerals, Grain size, and Texture, bedding or sorting.

<u>Type (First 2 letters)</u>		<u>Minerals (Second 2 letters)</u>		<u>Grain Size</u>		<u>Texture</u>	
SS	Sandstone	N	None	A	Aphanitic	V	Vesicular
SI	Siltstone	V	Olivine	B	Boulder	M	Massive
SM	Silty Sandstone	Q	Quartz	C	Cobble	A	Aphanitic
BA	Basalt	B	Biotite	G	Gravel	P	Porphyritic
GY	Gypsum	K	Potassium Feldspar	V	Very Fine	B	Bedded
		H	Hornblende	F	Fine	T	Thinly Bedded
		S	Sand	M	Medium	L	Laminated
		M	Silt	R	Coarse	R	Brecciated
		G	Gypsum	P	V.coarse	S	Scoracious
		C	Clay			X	Cross Bedded
		F	Iron				

**RECOVERY:**

The length of core recovered from each run. Expressed as length times 100.

Blank entries reflect no recovery for that interval.

**RQD:**

A quantitative index based on a core recovery procedure that is measured as the combined length of core greater than 2 times the diameter; expressed as the length times 100

Project		Miner Flat Dam Geotechnical Investigation - Phase I			Northing		1085626.0 Drilling Method		HQ Split Tube Core		Project #		943-27691 Task 120		Page					
Location		Sandstone Ridge			Easting		576826.4 Drilling Contractor		Golden Drilling		Logged By		L.Bush/C. Robinson		Borehole		MF-250			
					Elevation		8079.2		Inclination		90		Date Started		8/2/95		Date Completed		8/19/95	
Depth	Weathering		Rock Description				Recovery	RQD	Recovery %	RQD %	Fractures per foot rec.	Total Fractures	General Comments							
	Alteration	Type	Mineral	Gr. Size	Texture	Color														
11.8			OB																	
18.5	WS	BA	VQ	AF	V	N3	305	305	45.5	45.5	1.6	11								
22.6	WS	BA	VQ	AV	V	N3	387	387	94.4	94.4	2.4	10								
27.6	WS	BA	P	AF	VM	N3	470	470	94.0	94.0	1.8	9	Vascular to massive							
32.6	WS	BA	P	AF	MV	N3	260	260	52.0	52.0	0.8	4	massive to vascular							
33.6	WS	BA	P	AF	V	N3	100	100	100.0	100.0	5.0	5								
38.5	WS	BA	VP	AF	V	N3	435	435	88.8	88.8	1.8	9								
41.2	WS	BA	P	AF	V	N3	250	250	92.6	92.6	4.1	11								
46.2	WS	BA	PQ	AF	V	N3	510	510	102.0	102.0	1.6	8								
51.2	WF	BA	VP	AF	MV	N3	500	500	100.0	100.0	1.0	5								
56.2	WF	BA	VQ	AF	M	N3	500	500	100.0	100.0	0.4	2								
61.2	WS	BA	V	AF	M	N3	485	485	97.0	97.0	1.8	9	some joints fresh							
66.2	WF	BA	VQ	AF	M	N3	492	492	98.4	98.4	0.8	4								
71.2	WF	BA	VP	AF	M	N3	500	500	100.0	100.0	1.2	6								
76.2	WS	BA	VP	AF	MV	N3	460	460	92.0	92.0	1.4	7	fresh to 73.2							
81.2	WS	BA	VP	AF	MV	5R6/2	494	494	98.8	98.8	2.2	11								
86.2	WS	BA	VP	AF	V	N3	500	500	100.0	100.0	1.4	7								
91.3	WS	BA	VP	AF	V	N3	503	503	98.6	98.6	1.2	6								
96.3	WF	BA	VP	AF	M	N3	481	481	96.2	96.2	0.8	4								
101.3	WF	BA	VP	AF	M	N3	510	510	102.0	102.0	0.2	1								
106.3	WS	BA	VP	AF	S	N3	500	500	100.0	100.0	1.8	8								
111.3	WS	BA	V	AF	S	N3	310	310	62.0	62.0	1.2	6	highly fractured 106.7-108.6							
116.3	WF		Q	V		10R6/6	150	150	30.0	30.0	0.0	0	Basalt/sandstone contact at 109.3							
118.3	WF		Q	V		10R6/6	50	50	25.0	25.0	0.0	0	No Recovery							
120.8							0	0	0.0	0.0	0.0	0	No Recovery							
121.5							0	0	0.0	0.0	0.0	0	No Recovery							
124.6							15					0								
127.7	WF	SS	Q	V		5YR7/2	180	180	58.1	58.1	0.0	0								
131.2	WF	SS	Q	V		5YR7/1	180	180	51.4	51.4	0.0	0								
131.9	WF	SS	Q	V		10R5/4	35	35	50.0	50.0	2.9	2								
137.0	WF	SS	Q	V	X	10R5/4	360	360	70.6	70.6	2.2	11								
140.0	WF	SS	Q	V		10R5/4	312	312	104.0	104.0	4.7	14								
147.1	WF	SS	Q	V	X	10R5/4	190	190	26.8	26.8	0.8	4								
152.1	WF	SS	Q	V	X	10R5/4	470	470	94.0	94.0	3.8	19								
157.2	WF	SS	Q	V	B	10R6/6	436	436	85.5	85.5	2.4	12								
162.1	WF	SS	Q	V	B	10R6/6	110	110	22.4	22.4	0.8	4								
167.1	WF	SS	Q	V	B	10R6/6	230	230	46.0	46.0	2.8	14								
172.1	WF	SS	Q	V	XB	10R4/6	315	315	63.0	63.0	3.2	16								
177.1	WF	SS	Q	V	XB	10R4/6	280	280	52.0	52.0	3.0	15	Pc/Ps contact @177.1							
182.1	WF	SS	QM	V	B	10R4/6	165	165	33.0	33.0	2.0	10								
186.0	WF	SS	QM	V	B	10R4/6	320	320	82.1	82.1	3.1	12	Siltier @ 184							
191.1	WF	SS	QM	V	B	10R4/6	420	420	82.4	82.4	2.5	13	Sandy @ 187							
196.1	WF	SS	Q	V	B	10R4/6	450	450	90.0	90.0	1.6	8								
201.2	WF	SS	Q	V	BL	10R4/6	376	376	73.7	73.7	2.4	12	Silty & laminated							
206.2	WF	SS	Q	V	B	10R4/6	350	350	70.0	70.0	1.8	9	Highly fractured 203-205							
211.7	WF	SS	QM	V	B	10R4/6	270	270	49.1	49.1	0.0	0								
216.2	WF	SI	QM	V	B	10R4/6	430	430	95.6	95.6	2.2	10								
221.2	WF	SI	QM	V	B	10R4/6	492	492	98.4	98.4	1.2	6								
226.3	WF	SS	Q	V	B	10R4/6	200	200	39.2	39.2	1.6	8	Highly fractured							
231.3	WF	SS	Q	V		10R4/6	10	10	2.0	2.0	0.0	0	No Recovery							
233.3	WF	SS	QM	V		10YR7/1	20	20	10.0	10.0	0.0	0								
237.5	WF	SI	QM	V	MB	10R4/6	341	341	81.2	81.2	0.2	1								
242.6	WF	SI	QM	V	M	10R4/6	525	525	102.9	102.9	1.4	7								
247.5	WF	SS	QM	VF	M	10YR8/6	410	410	83.7	83.7	0.8	4	Calcite filled fracture @15 degrees							
248.1	WF	CS	MC	MC	BX	10YR6/6	4	4	6.7	6.7	0.0	0								
251.1	WF	SI	MC	MC	BX	10R3/4	286	286	95.3	95.3	0.7	2								
256.1	WF	SS	QM	VF	M	10R4/6	450	450	90.0	90.0	0.8	4								
260.5	WF	SS	QM	VF	M	10R4/6	75	75	17.0	17.0	0.0	0	Mislabel							
262.5	WF	SS	QM	VF	BX	10R4/6	200	200	100.0	100.0	1.5	3								
267.5	WF	SS	QM	VF	BX	10R4/6	475	475	95.0	95.0	1.4	7								
272.5	WF	SS	QM	VF	BX	10R4/6	497	497	99.4	99.4	2.6	13								
277.5	WF	SM	QM	V	BX	10R4/6	500	500	100.0	100.0	0.6	3								
282.5	WF	SM	QM	V	BX	10R4/6	500	500	100.0	100.0	1.0	5								
287.5	WF	SM	QM	V	BX	10R4/6	376	376	75.2	75.2	0.6	3								
291.5	WF	SI	MC	VF	BX	N6	364	364	91.0	91.0	0.5	2								
296.5	WF	SM	MC	VF	BX	N6	420	420	84.0	84.0	2.2	11								
301.5	WF	SM	CS	VF	BX	N6	515	515	103.0	103.0	1.4	7								
305.1	WF	SM	CS	VF	BX	N6	360	360	100.0	100.0	1.1	4								
310.0							0	0	0.0	0.0	0.0	0	Rods dropped 4.9'							
312.5	WF	SM	CS	VF	BX	N6	285	285	114.0	114.0	0.4	1								
317.5	WC	CL	GM			N6	490	490	98.0	98.0	0.0	0	Clay w/ 2.5' limestone boulder							
322.3	WC	CL	SC	BV		N6	345	345	71.9	71.9	0.0	0	Gypsum @ 322.6'							
327.3	WF	GY	GC	AV	M	N7	490	490	98.0	98.0	1.0	5								
331.3	WF	GY	GC	AV	M	N7	405	405	101.3	101.3	0.0	0								
ech													End of Hole							

Miner Flat Dam

Project		Miner Flat Dam Geotechnical Investigation - Phase I		Northing		1085586.0		Drilling Method		HQ Split Tube Core		Project #		943-27691 Task 120		Page	
Location		Sandstone Ridge		Easting		576823.1		Drilling Contractor		Golden Drilling		Logged By		D.Allowy		Borehole	
				Elevation		6079.2		Inclination		90		Date Started		8/29/95		Date Completed	
												8/30/95					
Depth	Weathering Alteration	Rock Description					Recovery	RQD	Recovery %	RQD %	Fractures per foot rec.	Total Fractures	General Comments				
		Type	Mineral	Gr. Size	Texture	Color											
8.7																	
12.3	WF	BA	QV	AV	V	N3	340	269	94.4	74.7	0.8	3					
17.3	WS	BA	QV	AV	V	N3	463	283	92.6	56.6	2.4	12					
22.3	WS	BA	QV	VF	V	N3	452	345	90.4	69.0	1.6	8					
27.3	WS	BA	QV	VF	V	N3	429	208	85.8	41.6	0.8	4					
31.3	WS	BA	QV	AV	V	N3	291	100	72.8	25.0	2.3	9					
36.3	WS	BA	QV	AF	V	N3	508	311	101.6	62.2	1.6	8					
41.3	WS	BA	QV	AF	V	N3	500	349	100.0	69.8	1.8	9					
46.3	WF	BA	QV	AF	V	N3	500	422	100.0	84.4	1.6	8					
51.3	WF	BA	QV	AF	V	N3	500	420	100.0	84.0	1.2	6					
56.4	WF	BA	QV	AF	M	N4	515	444	101.0	87.1	0.8	4					
61.5	WF	BA	QV	AF	M	N4	500	335	98.0	65.7	2.0	10					
66.5	WF	BA	QV	AF	M	N4	484	330	96.8	66.0	0.4	2					
71.5	WF	BA	QV	AF	M	N4	508	318	101.6	63.6	0.4	2					
76.5	WS	BA	QV	AF	V	N4	500	126	100.0	25.2	2.4	12					
81.5	WS	BA	QV	AF	V	N4	491	114	98.2	22.8	2.2	11					
86.5	WM	BA	QV	AF	V	N4	507	300	101.4	60.0	2.0	10					



Project		Miner Flat Dam Geotechnical Investigation - Phase I				Northing		1085422.0		Drilling Method		HQ Split Tube Core		Project #		943-27691 Task 120		Page	
Location		Sandstone Ridge				Easting		576870.9		Drilling Contractor		Golden Drilling		Logged By		L.Bush/C.Robinson		Borehole	
						Elevation		8089.0		Inclination		90		Data Started		6/30/95		MF-251	
												Data Completed		7/23/95					
Depth	Weathering		Rock Description				Recovery	RQD	Recovery %	RQD %	Fractures per foot rec.	Total Fractures	General Comments						
	Alteration	Type	Mineral	Gr. Size	Texture	Color													
30.0																			
31.5	WF	BA	VQ	FA	AV	N6	150	100	100.0	66.7	2.0	3.0							
36.5	WF	BA	VQ	FA	AM	N6	500	450	100.0	90.0	0.4	2.0							
41.6	WF	BA	VQ	FA	AM	N5	510	390	100.0	76.5	1.0	5.0							
45.9	WM	BA	VQ	AF	AV	5Y4/4	429	227	99.8	52.8	1.6	7.0	Clay filled fractures						
50.9	WS	BA	VQ	AF	AV	N4	498	255	99.6	51.0	1.4	7.0	Clay filled fractures						
55.9	WS	BA	VQ	AF	AV	N4	506	255	101.2	51.0	1.0	5.0	5-20 degree clay filled fracture						
60.9	WS	BA	VQ	AF	AV	N4	459	362	91.8	72.4	0.8	4.0	Clay zone at 58 ft						
65.9	WF	BA	VQ	AF	AV	N4	500	500	100.0	100.0	0.8	4.0	90 deg clay filled fracture						
70.9	WF	BA	VQ	AF	AV	N4	480	420	96.0	84.0	0.6	3.0							
75.9	WS	BA	VQ	AF	AV	5YR4/1	500	220	100.0	44.0	3.2	16.0							
80.9	WS	SS	Q	V	B	10R7/4	250	65	50.0	13.0	2.0	10.0	CONTACT basalt/sandstone						
84.4	WS	SS	Q	V	M	10R6/6	120	50	34.3	14.3	2.3	8.0							
89.4	WS	SS	Q	V	M	10R4/6	420	50	84.0	10.0	3.8	19.0							
92.3	WH	SS	Q	F	M	5YR5/6	274	83	94.5	28.6	0.7	2.0	Very friable						
97.3	WH	SS	Q	F	M	10R4/6	500	234	100.0	46.8	1.4	7.0	Very friable						
102.3	WH	SS	Q	VF	M	10R3/4	498	290	99.6	58.0	1.0	5.0	Very friable						
107.4	WM	SS	Q	FM	B	10YR6/6	465	218	91.2	42.7	1.8	9.0							
112.4	WM	SS	Q	FV	B	10R3/4	490	217	98.0	43.4	1.6	8.0							
117.4	WH	SS	Q	F	B	5YR3/4	486	0	97.2	0.0	1.2	6.0	Very friable						
122.4	WH	SS	Q	F	B	5YR3/4	479	0	95.8	0.0	1.4	7.0	Very friable						
127.4	WH	SS	Q	FV	B	5YR3/4	500	0	100.0	0.0	0.4	2.0	Very friable						
132.4	WH	SS	Q	FV	B	5YR4/4	498	212	99.6	42.4	0.6	3.0	Very friable						
137.4	WS	SS	Q	F	B	10R4/6	509	333	101.8	66.6	1.0	5.0	Very friable						
142.4	WM	SS	Q	F	B	10R4/6	490	198	98.0	39.6	0.8	4.0							
147.4	WS	SS	Q	F	B	10R5/4	510	194	102.0	38.8	1.6	8.0							
152.4	WH	SS	Q	F	B	10R5/4	494	94	98.8	18.8	0.2	1.0	Very friable						
157.4	WM	SS	Q	F	B	10R5/4	500	348	100.0	69.6	1.8	9.0							
162.4	WM	SS	Q	F	B	10R5/4	495	175	99.0	35.0	1.4	7.0							
167.4	WM	SS	Q	F	B	5YR5/6	500	159	100.0	31.8	2.4	12.0	Clay filled fracture @ 167						
172.4	WM	SS	Q	F	X	10YR6/6	474	306	94.8	61.2	1.6	8.0	Pc/Ps contact						
177.4	WM	SS	Q	F	X	10R4/6	505	140	101.0	28.0	1.4	7.0							
182.4	WM	SS	Q	F	B	10R4/6	491	451	98.2	90.2	0.8	4.0							
187.4	WM	SS	Q	F	B	10R4/6	504	294	100.8	58.8	1.4	7.0							
192.4	WM	SS	Q	F	B	10R4/6	498	430	99.6	86.0	0.8	3.0							
197.4	WH	SS	Q	F	B	10R4/6	478	161	95.6	32.2	0.4	2.0	Very friable zones						
202.4	WH	SS	Q	F	B	10R4/6	500	0	100.0	0.0	0.2	1.0	Very little cement						
207.4	WH	SS	Q	F	B	10R4/6	500	100	100.0	20.0	0.4	2.0							
211.2	WF	SM	MQ	VF	M	10R4/6	442	442	116.3	116.3	0.0	0.0							
215.8	WF	SM	MQ	VF	M	10R4/6	482	482	104.8	104.8	0.0	0.0	No nat. fractures						
220.7	WF	SS	MQ	VF	M	10R4/6	487	487	99.4	99.4	0.0	0.0	No nat. fractures						
225.0	WF	SS	MQ	VF	MB	10R4/6	491	346	114.2	80.5	1.4	6.0	SM to 223 R1 to 223						
229.8	WF	SS	Q	VF	B	10R5/4	475	175	99.0	36.5	1.3	6.0	no fract. in SM then R2						
234.0	WF	SM	Q	VF	M	10R5/4	320	42	76.2	10.0	1.7	7.0							
238.8	WF	SM	MQ	VF	M	10R5/4	407	65	84.8	13.5	1.3	6.0							
243.6	WF	SS	MQ	VF	BM	10R5/4	475	260	99.0	54.2	1.5	7.0							
248.3	WF	SS	Q	VF	BM	10R5/4	450	210	95.7	44.7	1.7	8.0							
253.0	WF	SS	Q	VF	M	10R5/4	444	312	94.5	66.4	0.4	2.0							
257.9							0	0	0.0	0.0	0.0	0.0	No recovery						
259.1							0	0	0.0	0.0	0.0	0.0	No recovery						
263.7	WF	CS	CM	V	M	10R5/4	453	310	98.5	67.4	0.7	3.0							
268.2	WF	SI	QM	V	BX	10R4/6	451	451	100.2	100.2	0.0	0.0							
273.0	WF	SI	QM	V	BX	10R4/6	480	395	100.0	82.3	0.8	4.0							
278.0	WF	SI	QM	V	BX	10R4/6	500	315	100.0	63.0	1.0	5.0	Silt like @ 276-277						
283.1	WF	SI	QM	V	BX	10R4/6	455	455	89.2	89.2	0.4	2.0							
287.9	WF	SI	QM	V	BX	10R4/6	455	430	94.8	89.6	0.2	1.0							
292.8	WF	SI	QM	V	BX	10R4/6	260	205	53.1	41.8	0.4	2.0							
295.7	WF	SI	QM	V	BX	10R4/6	480	480	165.5	165.5	0.0	0.0	Pick up lost core						
300.7	WF	SI	MQ	VF	BX	10R4/6	510	510	102.0	102.0	0.0	0.0	No nat. fractures						
305.6	WF	SI	MQ	V	BX	10R4/6	490	490	100.0	100.0	0.0	0.0	No nat. fractures						
310.6	WF	SI	MQ	V	BX	5Y6/4	475	475	95.0	95.0	0.2	1.0							
315.3	WF	CS	MC	A	BX	N5	485	485	103.2	103.2	0.0	0.0							
320.6	WF	CS	MC	A	BX	N5	258	232	48.7	43.8	0.4	2.0							
325.7	WF	SS	CQ	V	BX	N6	210	160	41.2	31.4	0.0	0.0	3/4" gravel 320-323						
326.9	WF	SS	MQ	V	BX	5Y7/6	150	150	125.0	125.0	0.8	1.0	Black soot-like dust on barrel						
332.0	WF	SS	CQ	V	BX	N6	400	400	78.4	78.4	0.2	1.0							
336.8	WF	MI	MQ	V	BX	10R4/6	405	330	84.4	68.7	0.8	4.0							
340.7	WF	MI	MQ	V	BX	10R4/6	445	445	114.1	114.1	0.5	2.0	End of hole						

Project		Miner Flat Dam Geotechnical Investigation - Phase I			Northing		1085422.0		Drilling Method		HQ Split Tube Core		Project #		943-27691 Task 120		Page			
Location		Sandstone Ridge			Easting		576870.9		Drilling Contractor		Golden Drilling		Logged By		L.Bush/D.Allaway		Borehole		MF-252	
					Elevation		6089.0		Inclination		90		Date Started		6/15/95		Date Completed		8/2/95	
Depth	Weathering Alteration	Rock Description					Recovery	RQD	Recovery %	RQD %	Fractures per foot rec.	Total Fractures	General Comments							
		Type	Mineral	Gr. Size	Texture	Color														
19.5																				
21.0	WS	BA	PV	A	V	N4	149	50	99.3	33.3	4	18								
26.0	WF	BA	PV	A	M	N3	500	480	100.0	96.0	4	8								
31.0	WF	BA	PV	A	M	N3	500	452	100.0	90.4	4	6								
36.0	WS	BA	PV	A	V	N3	475	183	95.0	36.8	11	26								
41.0	WS	BA	PV	A	V	N3	455	110	91.0	22.0	12	30								
46.0	WF	BA	PV	A	V	N3	500	355	100.0	71.0	5	30								
51.0	WF	BA	PV	A	MV	N3	500	440	100.0	88.0	2	16								
56.0	WF	BA	PV	A	M	N3	470	215	94.0	43.0	7	26								
61.0	WF	BA	PV	A	M	N3	498	498	99.6	99.8	1	8	V. Little clay in fractures							
66.0	WF	BA	PV	A	M	N3	500	333	100.0	66.6	7	14								
71.0	WF	BA	PV	A	M	N3	495	385	99.0	77.0	2	14								
76.0	WF	BA	PV	A	M	N3	500	390	100.0	78.0	5	8	@55, v. little clay							
81.0	WF	BA	PV	A	M	N3	500	185	100.0	37.0	11	28								
86.0	WF	BA	PV	A	M	N3	494	400	98.8	80.0	3	14	CONTACT basal/sandstone							
91.0	WF	SS	Q	VF	B	10R6/6	210	0	42.0	0.0	0	0								
96.0	WF	SS	CQ	VF	B	10R6/6	320	42	64.0	8.4	4	8	Gravelly							
101.0	WF	SS	CQ	VF	B	10R6/6	320	0	64.0	0.0	8	12	Highly fractured w/ clay							
106.0	WF	SS	Q	VF	B	10R8/2	480	175	96.0	35.0	15	22								
111.0	WF	SS	Q	VF	B	10YR8/2	496	180	99.2	36.0	3	4	Highly fractured							
116.0	WS	SS	Q	VF	B	10YR7/4	500	0	100.0	0.0	15	12	Highly fractured							
121.0	WS	SS	Q	VF	B	10YR7/4	390	0	78.0	0.0	29	28	Highly fractured							
122.6	WS	SS	Q	VF	B	10YR7/4	140	0	87.5	0.0	4	16	Highly fractured							
127.6	WF	SS	Q	VF	B	10YR7/4	480	50	96.0	10.0	8	14								
132.6	WF	SS	Q	VF	B	10YR8/2	480	90	96.0	18.0	11	20								
137.6	WS	SS	Q	VF	SO	10R5/4	325	0	65.0	0.0	3	6	Uncemented, soil like							
142.6	WS	SS	QC	VF	SO	10R5/4	355	0	71.0	0.0	2	8								
146.1	WS	SS	Q	VF	SO	10R5/4	275	0	78.6	0.0	0	0	Mostly soil like							
151.2	WF	SS	Q	VF	SO	10R5/4	360	0	70.6	0.0	4	6								
156.3	WF	SS	Q	F	BD	10R6/6	436	58	85.5	11.4	10	20								
161.5	WS	SS	Q	FV	BD	10R6/6	370	0	71.2	0.0	10	20	Pc/Ps contact							
166.6	WF	SS	Q	FV	M	10R6/6	321	59	62.9	11.6	6	9	Very fractured							
171.7	WS	SS	Q	FV	M	10R6/6	285	0	55.9	0.0	0	0	Very fractured many orientations							
176.8	WS	SM	QM	FV	BD	10R4/6	200	50	39.2	9.8	0	0	Very fractured many orientations							
181.0	WS	SM	QM	F	BD	10R4/6	162	0	38.6	0.0	0	0	Very fractured many orientations							
187.0	WF	SS	Q	F	BD	10R6/6	231	0	38.5	0.0	4	9	Very fractured many orientations							
192.0	WS	SS	Q	F	M	10YR10/7	294	0	58.8	0.0	2	5	Very fractured clay @ 192							
197.0	WF	SS	Q	F	BD	10R4/6	504	163	100.8	32.6	6	11								
202.0	WS	SS	Q	F	BD	10R4/6	509	345	101.8	69.0	3	4								
206.4	WF	SC	QC	FV	BD	10R4/6	344	189	78.2	43.0	2	10	Slipped 1' of core							
211.5	WF	SC	QC	FV	BD	10R4/6	402	302	78.8	59.2	5	10	Picked up core from prev.							
215.6	WF	SC	QC	FV	BD	10R4/6	500	444	122.0	108.3	5	15								
220.8	WF	SC	QC	FV	BD	10R4/6	518	457	99.6	87.9	2	8								
225.9	WF	SC	QC	F	BD	10R4/6	494	63	96.9	12.4	5	3								
231.0	WF	SC	QC	F	BD	10R4/6	512	512	100.4	100.4	0	0	No natural fractures							
236.1	WF	SC	QC	F	BD	10R4/6	460	154	90.2	30.2	5	9	Slipped core							
239.1	WF	SS	Q	F	BD	10R4/6	277	277	92.3	92.3	0	0	Picked up core from prev., slipped core							
242.6	WF	SC	QC	FV	BD	10R4/6	358	320	102.3	91.4	2	4	Gained 0.7' from prev., slipped 0.6'							
247.4	WF	SC	QC	F	BD	10R7/4	490	294	102.1	81.2	2	7								
252.5	WF	SS	Q	FV	BD	10YR7/4	502	461	98.4	90.4	1	2								
253.8	WF	SS	Q	F	BD	10R4/6	128	61	98.5	46.9	3	9								
257.5	WF	CS	CQ	F	BD	10R4/6	420	400	113.5	108.1	1	4								
262.6	WF	SC	CQ	F	BD	10R4/6	442	442	86.7	86.7	25	0	No natural fractures, slough in hole							
267.6	WF	SC	QC	F	BD	10R5/6	520	520	104.0	104.0	1	3								
272.6	WF	CS	CQ	F	BD	10R4/6	495	368	99.0	73.8	8	12								
277.7	WF	CS	CQ	F	BD	10R4/6	510	510	100.0	100.0	0	0	No natural fractures							
282.6	WF	CS	CQ	VF	BD	10R4/6	495	373	101.0	78.1	6	12								
287.7	WF	CS	CQ	FV	B	10R4/6	490	440	96.1	86.3	2	6								
292.6	WF	CS	CQ	VF	B	10YR4/2	500	301	102.0	81.4	6	20								
297.7	WF	CS	CQ	F	B	10YR4/2	502	502	98.4	98.4	1	5								
302.6	WF	CM	CM	V	B	10YR4/2	505	460	103.1	93.9	4	7								
307.7	WF	CM	CM	V	B	N5	505	375	99.0	73.5	0	0	No natural fractures, clay @ 304.7-306.2							
312.6	WF	MC	MC	V	B	N6	490	490	100.0	100.0	0	0	No natural fractures							
317.7	WF	MC	MC	V	B	N4	504	504	98.8	98.8	0	0	No natural fractures							
322.6	WF	MC	MC	V	B	N5	498	408	101.6	83.3	1	5								
327.7	WF	CM	CM	V	B	5Y4/4	500	500	98.0	98.0	0	0	No natural fractures							
332.7	WF	MC	MC	V	B	5Y6/4	496	496	99.2	99.2	0	0	No natural fractures							
EOH													End of Borehole							

Miner Flat Dam

Project		Miner Flat Dam Geotechnical Investigation - Phase I				Northing		1085032.0		Drilling Method		HQ Split Tube Core		Project #		943-27691 Task 120		Page	
Location		Sandstone Ridge				Easting		678791.7		Drilling Contractor		Golden Drilling		Logged By		L. Bush/D. Alloway		Borehole	
						Elevation		6103.1		Inclination		90.0		Date Started		6/3/95		Borehole	
														Date Completed		6/29/95			
Depth	Weathering Abrasion	Rock Description					Recovery	RQD	Recovery %	RQD %	Fractures per foot	Total Fractures	General Comments						
		Type	Mineral	Gr. Size	Texture	Color													
8.7																			
12.5	WF	SS	Q	F		10R4/6	130	0	34.2	0.0	0	0							
17.5							0	0	0.0	0.0	0	0							
21.2	WF	SS	Q	F	B	10R4/6	300	0	81.1	0.0	1	4	Highly fractured						
26.2	WF	SS	Q	F	B	10R4/6	460	0	92.0	0.0	5	25	Highly fractured						
28.7	WF	SS	Q	F		10R4/6	50	0	100.0	0.0	0	0	Highly fractured						
31.7	WF	SS	Q	F		10R4/6	340	0	68.0	0.0	2	9	Highly fractured						
33.7	WF	SS	Q	VF	B	10R4/6	168	0	84.0	0.0	0	0							
34.1	WS	SS	QC	VF	B	10R4/6	45	0	112.5	0.0	0	0							
37.5	WS	SS	QC	VF	B	10R5/4	200	0	58.8	0.0	0	0	Highly fractured						
42.5	WS	SS	QC	VF	B	10R5/4	475	42	95.0	8.4	1	7	Highly fractured						
47.0	WF	SS	Q	F	B	10R5/4	420	115	93.3	25.8	3	14	Highly fractured						
52.0	WS	SS	Q	F	B	10R5/4	505	62	101.0	12.4	4	22							
57.0	WF	SS	QC	F	B	10R5/4	510	118	102.0	23.6	4	21							
62.0	WF	SS	Q	F	B	10R5/4	490	60	98.0	12.0	3	16							
67.0	WF	SS	Q	F	B	10R6/6	475	328	95.0	65.6	2	9	Mod. fractured @ 66ft						
72.0	WS	SS	Q	F	X	10YR7/4	446	229	89.2	45.8	2	12	Very fractured @71 ft						
77.0	WS	SS	QC	F	X	10R5/4	493	250	98.6	50.0	2	10							
82.0	WS	SS	QC	F	B	10R5/4	352	223	70.4	44.6	1	4							
87.0	WS	SS	Q	F	M	10YR7/4	300	110	60.0	22.0	2	10							
91.6	WS	SS	Q	F	B	10YR6/6	400	91	87.0	19.8	3	15							
96.7	WS	SS	Q	F	M	10YR6/6	401	114	78.6	22.4	3	15							
101.8	WS	SS	Q	F	M	10YR5/6	451	87	88.4	17.1	3	14							
106.5	WS	SS	QC	F	B	10YR4/6	443	135	94.3	28.7	3	12							
111.6	WS	SS	Q	F	B	10YR7/4	431	133	84.5	26.1	2	11	Highly fractured						
116.7	WS	SS	QC	F	B	10YR4/6	285	60	55.9	11.8	2	8	Highly fractured						
121.7	WS	SS	Q	F	M	10YR4/6	55	0	11.0	0.0	0	0	Highly fractured						
126.8	WM	SS	QC	F	B	10YR4/6	110	0	21.8	0.0	0	1	Highly fractured						
131.8	WS	SS	Q	F	B	5YR5/6	112	0	22.4	0.0	1	4	Highly fractured						
136.8	WS	SS	Q	F	B	5YR5/6	220	0	44.0	0.0	1	3	Highly fractured						
141.9	WS	SS	Q	F	M	10R5/4	212	0	41.6	0.0	2	12	Highly fractured						
144.9	WF	SS	Q	F		10R5/4	40	0	13.3	0.0	0	0	Poor Recovery						
149.0	WF	SI	MQ	VF	BM	10R4/6	160	0	39.0	0.0	0	1	Poor Recovery						
154.0	WS	SS	MQ	F	M	10R5/4	210	86	42.0	17.2	0	2							
159.0	WF	SS	Q	VF	B	10R5/4	278	42	55.6	8.4	1	4	Highly fractured						
160.0							0		0.0	0.0	0	0	Rods dropped 159-160						
163.0	WF	SS	Q	F	B	10R5/4	246	49	82.0	16.3	2	5							
166.5	WF	SS	Q	F		10R5/4	45	0	12.9	0.0	0	0	End of Hole						

Project		Miner Flat Dam Geotechnical Investigation - Phase I					Northing		1085007.0		Drilling Method		HQ Split Tube Core		Project #		943-27891 Task 120		Page	
Location		Sandstone Ridge					Easting		578834.8		Drilling Contractor		Golden Drilling		Logged By		D.Alloway		Borehole	
							Elevation		8107.5		Inclination		90		Date Started		8/22/95		Date Completed	
Depth	Weathering Alteration	Rock Description					Recovery	RQD	Recovery %	RQD %	Fractures per foot rec.	Total Fractures	General Comments							
		Type	Mineral	Gr. Size	Texture	Color														
6.0																				
11.0	WF	SS	Q	VF	B	10R6/6	482	125	96.4	25.0	2.8	14								
16.0	WS	SS	Q	VF	B	10R6/6	512	0	102.4	0.0	4.6	23	Highly fractured, non cemented							
21.0	WS	SS	Q	VF	B	10R6/6	448	0	89.6	0.0	2.8	14	Highly fractured, non cemented							
22.0	WS	SS	Q	VF	B	10R6/6	80	0	80.0	0.0	2.0	2	Highly fractured, non cemented							
27.0	WS	SS	Q	VF	B	5YR5/6	458	0	91.6	0.0	2.4	12	Highly fractured, poorly cemented							
32.0	WS	SS	Q	VF	B	5YR5/6	492	0	98.4	0.0	4.8	24	Highly fractured, poorly cemented							
37.0	WS	SS	Q	VF	B	5YR5/6	500	0	100.0	0.0	4.2	21	Highly fractured, poorly cemented							
42.0	WS	SS	Q	VF	B	5YR5/6	500	0	100.0	0.0	3.2	16	Highly fractured, poorly cemented							
47.0	WS	SS	Q	VF	B	5YR5/6	491	0	98.2	0.0	2.4	12	Clayey silt @ 45.7-47.3							
52.0	WS	SS	MQ	VF	B	10R6/6	451	0	90.2	0.0	2.4	12	Highly fractured, poorly cemented							
57.0	WS	SS	Q	VF	B	10R6/6	461	49	92.2	9.8	5.0	25	Highly fractured, poorly cemented							
62.0	WS	SS	Q	FV	B	10YR8/6	512	246	102.4	49.2	2.8	14								
67.0	WF	SS	Q	FV	B	5YR5/6	486	0	97.2	0.0	11.0	55	V. Fractured, poorly cemented							
72.0	WS	SS	Q	FV	B	5YR5/6	486	74	97.2	14.8	2.8	14	poorly cemented							
77.0	WF	SS	Q	FV	B	5YR5/6	513	510	102.6	102.0	0.4	2								
82.0	WF	SS	Q	FV	B	5YR5/6	498	498	99.8	99.8	0.0	0	No natural fractures							
87.0	WF	SS	QC	FV	B	10R4/6	513	338	102.6	67.6	2.0	10								
92.0	WS	SS	Q	FV	B	5YR6/4	478	0	95.6	0.0	3.2	16	Highly fractured, poorly cemented							
97.0	WS	SS	Q	FV	B	10YR7/4	484	57	96.8	11.4	2.8	14								
102.0	WF	SS	Q	FV	B	10R6/6	500	240	100.0	48.0	1.6	8								
107.0	WF	SS	Q	VF	B	10R6/6	508	0	101.6	0.0	2.8	14	Highly fractured, poorly cemented							
112.0	WF	SS	Q	VF	B	10R6/6	500	143	100.0	28.8	2.8	14								
117.0	WF	SS	CQ	VF	B	10R4/6	512	400	102.4	80.0	0.8	4								
122.0	WS	SS	Q	VF	B	10R7/4	500	248	100.0	49.6	1.2	6								
127.0	WS	SS	CQ	VF	B	10R4/6	482	0	96.4	0.0	1.8	9								
132.0	WS	SS	Q	VF	B	10R6/6	464	0	92.8	0.0	0.4	2	V. poorly cemented							
137.0	WS	SS	Q	VF	B	10R6/6	450	0	90.0	0.0	1.6	8	Soil-like, non cemented							
142.0	WS	SS	Q	VF	B	10R6/6	489	146	97.8	29.2	1.6	8	Soil-like, non cemented							
147.0	WS	SS	Q	VF	B	10R6/6	475	0	95.0	0.0	1.8	9	V. poorly cemented							
152.0	WF	SS	Q	VF	B	10R6/6	500	203	100.0	40.6	2.0	10								
157.0	WS	SS	Q	VF	B	10R6/6	480	0	96.0	0.0	2.6	13	Soil like, non cemented							
162.0	WS	SS	Q	VF	B	10YR7/4	486	85	97.2	17.0	1.4	7	Soil like to poorly cemented							
167.0	WS	SS	Q	VF	B	10R6/6	477	0	95.4	0.0	1.2	6	soil like							
172.0	WS	SS	Q	VF	B	10R6/6	480	50	96.0	10.0	3.0	15	v. poorly cemented							
177.0	WF	SS	Q	VF	XB	10R6/6	490	75	98.0	15.0	1.4	7								
182.0	WF	SS	Q	VF	B	5YR5/6	484	54	96.8	10.8	1.8	9								
187.0	WF	SS	Q	VF	B	5YR5/6	497	0	99.4	0.0	1.6	8								
192.0	WF	SS	Q	VF	B	5YR5/6	475	78	95.0	15.6	2.6	13								
197.0	WF	SS	Q	VF	B	5YR5/6	500	218	100.0	43.6	2.2	11	Pc/Ps contact							
202.0	WF	SS	Q	VF	B	10R6/6	505	227	101.0	45.4	1.4	7	Weakly reactive to HCl to 232							
207.0	WF	SS	Q	VF	B	10R6/6	500	448	100.0	89.6	0.4	2								
212.0	WF	SS	Q	VF	B	10R6/6	500	350	100.0	70.0	1.4	7								
217.0	WF	SS	Q	VF	B	10R4/6	501	415	100.2	83.0	0.2	1								
222.0	WF	SS	Q	VF	B	10R6/6	500	490	100.0	98.0	0.2	1								
227.0	WF	SS	Q	VF	XB	10R6/6	495	400	99.0	80.0	0.2	1								
232.0	WS	SS	Q	VF	B	10R4/6	500	50	100.0	10.0	2.0	10	Mod HCl. No nat'l fract.							
237.0	WF	SS	Q	VF	B	10R4/6	511	506	102.2	101.2	0.2	1								
242.0	WF	SS	Q	VF	B	10R4/6	490	490	98.0	98.0	0.0	0	Weakly reactive, no natural fractures							
247.0	WF	SS	Q	VF	B	10R6/6	511	474	102.2	94.8	0.2	1								
252.0	WF	SS	Q	VF	B	10R6/6	508	402	101.6	80.4	0.6	3								
257.0	WS	SS	Q	VF	B	10R6/6	388	0	77.6	0.0	1.8	9								
262.0	WF	SS	Q	VF	B	10R6/6	490	314	98.0	62.8	5.0	25								
267.0	WF	SC	QC	VF	B	10R4/6	511	511	102.2	102.2	0.0	0	No natural fractures							
272.0	WF	SC	QC	VF	B	10R4/6	514	514	102.8	102.8	0.0	0	No natural fractures, HCl reactive							
277.0	WF	CM	CM	V	B	10R4/6	444	444	88.8	88.8	0.0	0	No natural fractures, lost core							
281.7	WF	CM	CM	V	B	10R4/6	533	533	113.4	113.4	0.0	0	Picked up core							
286.8	WF	CM	CM	V	B	10R4/6	471	471	92.4	92.4	0.0	0	No natural fractures							
291.9	WF	SI	MC	V	B	10R4/6	330	206	64.7	40.4	0.6	3	Slipped core							
293.9	WF	SI	MC	V	B	10R4/6	390	314	195.0	157.0	0.5	1	Broken zone, picked up core							
298.5	WF	SI	MC	V	B	5YR3/4	506	451	110.0	98.0	1.3	6								
302.0	WF	SI	MC	V	B	5YR3/4	335	335	95.7	95.7	0.0	0	no natural fractures							
307.0	WF	SI	MC	V	B	N7	522	430	104.4	86.0	1.2	6								
312.0	WF	SI	MC	V	B	N7	437	437	87.4	87.4	0.0	0	No natural fractures							
316.6	WF	SI	MC	V	B	N7	510	420	110.9	91.3	1.1	5								
321.7	WF	SI	MC	V	B	N7	508	466	99.6	91.4	0.8	4								
326.8	WF	SI	MC	V	B	N7	513	506	100.6	99.2	0.4	2								
331.9	WS	SI	MC	V	B	10YR6/2	490	130	96.1	25.5	0.4	2								
337.0	WF	SI	MC	V	B	N5	486	361	95.3	70.8	0.8	4								
342.0	WF	CM	MC	V	B	5YR4/4	510	469	102.0	93.8	0.6	3	Gypsum @ 345.9							
347.0	WF	CM	MC	V	B	5YR4/4	483	406	96.8	81.2	0.4	2								
352.0	WF	CM	MC	V	B	5YR4/4	518	451	103.6	90.2	0.6	3								
357.0	WF	G	G	V	B	N6	496	465	99.2	93.0	0.2	1								
362.0	WF	G	G	V	B	N6	512	495	102.4	99	0.8	4	End of Hole							

Project		Miner Flat Dam Geotechnical Investigation - Phase I					Northing		1084797.0		Drilling Method		HQ Split Tube Core		Project #		943-27691 Task 120		Page			
Location		Sandstone Ridge					Easting		578874.7		Drilling Contractor		Golden Drilling		Logged By		D.Alloway		Borehole		MF-253	
							Elevation		6071.4		Inclination		90		Date Started		6/3/95		Date Completed		8/21/95	
Depth	Weathering Alteration	Rock Description					Recovery	RQD	Recovery %	RQD %	Fractures per foot rec.	Total Fractures	General Comments									
		Type	Mineral	Gr. Size	Texture	Color																
34.8																						
37.2	WF	SC	QC	VF	B	10R6/6	239	70	99.6	29.2	2.1	5										
42.2	WF	SC	QC	VF	B	10R6/6	500	63	100.0	12.8	2.0	10										
47.2	WF	SS	Q	VF	B	10YR6/6	510	200	102.0	40.0	1.8	9										
52.2	WF	SS	Q	VF	B	10YR6/6	494	149	98.8	29.8	2.0	10										
57.2	WF	SS	Q	VF	B	10YR8/2	505	324	101.0	64.8	1.2	6										
62.2	WF	SC	QC	VF	B	10R4/6	520	296	104.0	59.2	1.6	8										
67.2	WF	SS	Q	VF	BX	10R6/6	504	325	100.8	65.0	1.0	5										
72.2	WF	SS	Q	VF	BX	10R6/6	490	400	98.0	80.0	0.4	2	Top 1' very friable									
77.2	WS	SS	Q	VF	B	10R6/6	493	154	98.6	30.8	1.0	5										
82.2	WS	SS	Q	VF	BX	10R6/6	485	182	97.0	36.4	1.4	7										
87.2	WS	SS	Q	VF	B	10R4/6	461	0	92.2	0.0	1.4	7										
92.0	WF	SS	Q	VF	B	10R4/6	498	288	103.8	60.0	1.0	5										
97.1	WF	SS	Q	VF	B	10YR8/2	510	203	100.0	39.8	2.2	11										
102.2	WF	SS	Q	VF	B	10R6/6	508	141	99.6	27.6	1.8	9										
107.2	WS	SS	Q	VF	B	10YR8/2	290	0	58.0	0.0	1.2	6	V. poorly cemented									
112.2	WS	SS	Q	VF	B	10R6/6	475	50	95.0	10.0	0.8	4	V. poorly cemented, v. fractured									
117.2	WF	SS	Q	VF	B	10R7/4	483	0	96.6	0.0	2.8	14	V. poorly cemented, v. fractured									
122.2	WS	SS	Q	VF	B	10YR8/2	460	0	92.0	0.0	1.4	7	V. poorly cemented									
127.2	WS	SS	Q	VF	B	10YR8/2	500	0	100.0	0.0	1.6	8	V. poorly cemented									
132.2	WS	SS	Q	VF	B	10YR8/2	458	0	91.6	0.0	1.8	9										
137.2	WF	SS	Q	VF	B	10R4/6	490	352	98.0	70.4	0.6	3										
142.2	WF	SS	Q	VF	B	10R6/6	497	366	99.4	73.2	0.4	2										
147.2	WF	SS	Q	VF	B	10R6/6	505	307	101.0	61.4	0.8	4										
152.2	WF	SS	Q	VF	B	10R6/6	500	89	100.0	17.8	1.0	5										
157.2	WF	SS	Q	VF	B	10R6/6	500	359	100.0	71.8	1.4	7										
162.2	WF	SS	Q	VF	B	10R6/6	505	262	101.0	52.4	1.0	5										
167.2	WF	SS	Q	VF	B	10R6/6	512	230	102.4	46.0	1.2	6	Pc/Ps contact									
172.2	WF	SS	Q	VF	B	10R6/6	480	224	96.0	44.8	1.6	8										
177.2	WF	SS	Q	VF	B	10R6/6	508	380	101.6	76.0	1.2	6										
182.2	WF	SS	Q	VF	B	10R6/6	492	449	98.4	89.8	0.6	3										
187.2	WF	SS	Q	VF	B	10R6/6	504	325	100.8	65.0	1.6	8										
192.2	WF	SS	Q	VF	B	5YR5/6	492	450	98.4	90.0	0.6	3										
197.2	WF	SS	Q	VF	B	5YR5/6	500	463	100.0	92.6	0.6	3										
202.2	WF	SS	Q	VF	B	5YR5/6	520	490	104.0	98.0	0.2	1										
207.2	WF	SS	Q	VF	B	5YR4/4	473	410	94.6	82.0	0.2	1										
212.2	WF	SS	Q	VF	B	5YR4/4	459	459	91.8	91.8	0.0	0	No natural fractures									
217.0	WF	SS	Q	VF	B	5YR4/4	515	515	107.3	107.3	0.0	0	No natural fractures									
222.1	WF	SS	Q	VF	B	5YR4/4	520	469	102.0	92.0	0.4	2										
227.2	WF	SS	Q	VF	B	5YR4/4	483	211	94.7	41.4	1.0	5										
231.7	WF	SS	Q	VF	B	5YR4/4	480	356	106.7	79.1	0.4	2										
236.8	WF	SS	Q	VF	B	5YR4/4	504	418	98.8	82.0	0.0	0										
241.9	WF	SS	Q	VF	B	10R7/4	497	497	97.5	97.5	0.0	0	No natural fractures									
247.0	WF	SS	Q	VF	B	5YR4/4	520	520	102.0	102.0	0.0	0	No natural fractures									
252.0	WF	SS	Q	VF	RB	10R7/4	500	115	100.0	23.0	1.0	5										
257.1	WF	SI	MQ	VF	RB	10R6/6	494	481	96.9	94.3	0.2	1										
262.1	WF	SI	MQ	VF	B	10R4/6	496	406	99.2	81.2	0.0	0	No natural fractures									
267.1	WF	SI	MQ	VF	B	10R4/6	523	512	104.6	102.4	0.2	1										
272.1	WF	SI	MQ	VF	B	10R4/6	510	510	102.0	102.0	0.0	0	No natural fractures									
277.1	WF	SC	MC	V	B	10R4/6	508	488	101.6	97.6	0.2	1										
282.1	WF	SC	MC	V	B	10R4/6	507	507	101.4	101.4	0.0	0										
287.1	WF	SC	MC	V	B	5YR4/4	445	412	89.0	82.4	0.6	3										
292.0	WF	CS	CM	V	RB	5YR4/4	515	165	105.1	33.7	5.1	25	V. fractured									
295.2	WF	SC	MC	V	RB	5YR5/2	300	94	93.8	29.4	0.6	2	Contact w/ Gypsum									
299.2	WF	SC	MC	V	B	5YR4/4	447	447	111.8	111.8	0.0	0	No natural fractures									
302.2	WF	SC	MC	V	B	5YR4/4	280	280	93.3	93.3	0.0	0	No natural fractures									
307.2	WF	GY	G	A	M	N3	504	504	100.8	100.8	0.0	0	No natural fractures									
312.2	WF	GY	G	A	M	N3	500	495	100.0	99.0	0.2	1										
317.2	WF	GY	G	A	B	N5	510	510	102.0	102.0	0.0	0	No natural fractures									
322.2	WF	GY	G	A	B	N5	500	500	100.0	100.0	0.0	0	No natural fractures									
327.2	WF	GY	G	A	B	N5	500	500	100.0	100.0	0.0	0	No natural fractures									
EOH													End of Borehole									

Project		Miner Flat Dam Geotechnical Investigation - Phase I					Northing		1084855.0		Drilling Method		HQ Split Tube Core		Project #		943-27691 Task 120		Page	
Location		Sandstone Ridge					Easting		577053.7		Drilling Contractor		Golden Drilling		Logged By		D.Allaway		Borehole	
							Elevation		8081.3		Inclination		90		Date Started		6/14/95		Date Completed	
																			8/7/95	
Depth	Weathering Alteration	Rock Description					Recovery	RQD	Recovery %	RQD %	Fractures per foot rec.	Total Fractures	General Comments							
		Type	Mineral	Gr. Size	Texture	Color														
29.3																				
32.2	WF	BA	QV	F	V	N4	289	289	99.7	99.7	0.0	0	No natural fractures							
37.2	WF	BA	QV	F	V	N4	488	272	97.6	54.4	1.0	5								
42.2	WF	BA	QV	F	V	N4	500	500	100.0	100.0	0.2	1								
47.2	WF	BA	QV	F	V	N4	504	410	100.8	82.0	0.4	2	1/2" clay filled fracture @ 15 deg 43.3-44.3, water loss							
52.2	WF	BA	QV	F	V	N4	478	400	95.6	80.0	0.6	3	Slipped core							
56.1	WF	BA	QV	F	M	N4	424	350	108.7	89.7	0.5	2	Begin mud use, picked core from previous							
61.1	WF	BA	QV	FM	M	N4	500	455	100.0	91.0	1.0	5								
66.1	WF	BA	QV	FM	M	N4	501	221	100.2	44.2	1.6	8	Basalt/Sandstone contact @ 69.9							
71.1	WF	BA	QV	FM	M	N4	470	278	94.0	55.6	1.0	5	Clay filled fractures, SS at 69.9, 100% water loss							
75.9	WS	SS	Q	VF	M	10R7/4	450	0	93.7	0.0	0.0	0	Non-cemented sand							
81.0	WS	SS	Q	VF	M	10R7/4	430	58	84.3	11.4	0.4	2	Non-cemented sand							
86.0	WS	SS	Q	VF	M	10R4/6	362	52	72.4	10.4	0.6	3								
90.1	WS	SS	Q	VF	M	10R6/6	350	51	85.4	12.4	1.7	7								
91.1	WS	SS	Q	VF	M	10R6/6	120	0	120.0	0.0	1.0	1	Slipped core							
96.1	WS	SS	Q	VF	M	10R6/6	510	212	102.0	42.4	1.0	5	Picked up core from previous							
101.1	WS	SS	Q	VF	B	10R6/6	485	0	97.0	0.0	0.2	1								
104.7	WS	SS	Q	VF	B	10R6/6	308	0	85.0	0.0	0.8	3								
109.7	WS	SS	Q	VF	B	10R6/6	445	138	89.0	27.6	1.0	5								
112.2	WS	SS	Q	VF	B	10R6/6	247	49	98.8	19.6	1.2	3								
117.2	WS	SS	Q	VF	B	10R6/6	510	128	102.0	25.6	1.8	9								
122.2	WS	SS	Q	VF	B	10R6/6	500	110	100.0	22.0	1.4	7	Slipped core							
127.2	WS	SS	Q	VF	B	10R6/6	512	144	102.4	28.8	0.8	4	Picked up core from previous							
132.2	WS	SS	Q	VF	B	10R6/6	459	0	91.8	0.0	1.6	8								
137.2	WS	SS	Q	VF	B	5YR6/4	505	118	101.0	23.6	1.0	5								
142.2	WS	SS	Q	VF	B	5YR6/4	462	0	92.4	0.0	1.8	9								
147.1	WS	SS	Q	VF	B	5YR6/4	492	0	100.4	0.0	2.2	11	Very fractured							
152.1	WS	SS	Q	VF	B	5YR5/6	475	132	95.0	26.4	1.2	6								
157.1	WS	SS	Q	VF	B	5YR5/6	512	372	102.4	74.4	0.6	3								
162.1	WF	SS	Q	VF	B	10R6/6	492	492	98.4	98.4	0.0	0	No natural fractures							
167.1	WF	SS	Q	VF	B	10R6/6	508	418	101.6	83.6	0.2	1	Soil like 164.8-165.7							
172.1	WS	SS	Q	VF	B	10R6/6	475	255	95.0	51.0	0.6	3	Very poorly cemented							
177.1	WS	SS	Q	VF	B	10R6/6	500	130	100.0	26.0	0.4	2	Very poorly cemented							
182.1	WF	SS	Q	VF	B	10YR7/4	503	356	100.6	71.2	0.6	3	Very poorly cemented							
187.1	WF	SS	Q	VF	B	10YR7/4	500	448	100.0	89.6	0.6	3								
192.1	WF	SS	Q	VF	B	10YR7/4	496	450	99.2	90.0	1.2	6								
197.1	WF	SS	Q	VF	B	10R4/6	502	378	100.4	75.6	0.6	3								
202.1	WF	SS	Q	VF	B	10R4/6	500	300	100.0	60.0	0.6	3								
207.1	WF	SS	QC	VF	B	10R6/6	504	455	100.8	91.0	0.2	1	Clayey zones throughout							
212.1	WF	SS	QC	VF	B	10R4/6	491	377	98.2	75.4	0.4	2	Lower part appears brecciated							
217.1	WF	CS	CQ	VF	B	10R4/6	510	510	102.0	102.0	0.0	0	No natural fractures							
222.1	WF	SC	QC	VF	B	10R4/6	500	393	100.0	78.6	0.4	2								
227.1	WF	SC	QC	VF	B	10R4/6	510	460	102.0	92.0	0.4	2								
232.1	WF	SC	QC	VF	B	10R4/6	507	365	101.4	73.0	0.6	3								
237.1	WF	CS	CQ	VF	B	10R4/6	498	498	99.6	99.6	0.0	0	No natural fractures							
242.1	WF	CS	CQ	VF	B	10R4/6	448	422	89.6	84.4	0.6	3	Slipped core							
246.7	WF	SC	QC	VF	B	10R4/6	486	448	105.7	97.4	0.9	4	Gained core from previous							
251.5	WF	SC	QC	VF	B	5YR4/4	508	497	105.8	103.5	0.2	1								
254.3	WF	CG	CG	VA	B	N5	291	144	103.9	51.4	0.4	1	Gypsum at 254.8							
258.3	WF	G	G	A	B	N5	398	398	99.5	99.5	0.0	0	No natural fractures							
262.2	WF	G	G	A	B	N5	389	375	99.7	96.2	0.3	1								
267.2	WS	GC	GC	AF	B	5YR4/4	505	0	101.0	0.0	5.0	25	Many horiz. fractures							
272.2	WS	C	C	V	B	N5	490	0	98.0	0.0	5.0	25	Soil like							
277.2	WS	SI	MS	VA	B	N3	504	212	100.6	42.4	5.0	25	As above, layered clay and gypsum							
282.2	WF	SC	MC	VA	B	N5	496	416	99.2	83.2	1.0	5								
287.2	WF	GC	GC	VA	B	5YR4/4	512	418	102.4	83.6	1.2	6								
292.2	WS	G	G	A	B	N3	508	508	101.6	101.6	0.6	3								
297.2	WS	GC	GC	AV	B	N5	495	472	99.0	94.4	0.6	3								
302.2	WS	GC	GC	AV	B	N5	499	499	99.8	99.8	0.6	3								
307.2	WS	GC	GC	AV	B	N5	513	513	102.8	102.8	0.4	2								
312.2	WS	G	G	A	B	N5	500	500	100.0	100.0	0.0	0	No natural fractures							
317.2	WS	G	G	A	B	N5	502	502	100.4	100.4	0.0	0	No natural fractures							
322.2	WS	CG	CG	VA	B	10R4/6	386	386	77.2	77.2	0.0	0	No natural fractures, slipped 1.2 ft of core							
325.2	WS	CG	CG	VA	B	10R4/6	459	459	153.0	153.0	0.0	0	No natural fractures, picked up 1.1 ft of core							
EOH													End of Borehole							

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE  NORTHING: EASTING: _____ ELEVATION _____	DRILLING METHOD: BECKER HAMMER	BORING NO. BA-1
	SAMPLING METHOD: BULK	SHEET 1 OF 4
	DRILLING	
	START TIME 11:00	FINISH TIME 13:30
	DATE 7/25/95	DATE 7/25/95
	DRILL RIG AP1000 BECKER HAMMER	

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD
ANGLE VERTICAL BEARING NA	SAMPLE HAMMER

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	60			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

1						(0.0-5.0 ft.) Light brown (5YR 5/6), loose to compact, CLAYEY SAND with GRAVEL, 35% fine to medium subrounded sand, 20% silty clay, 20% subangular cobbles and 25% fine to coarse subangular gravel, <5% coarse subangular sand sized sandstone, dry, (SC)								
2														
3														
4														
5														
6														
7														
8														
9														
10														
11														
12														
13														
14														

JOB NO.: 243-27691170  
 FILENAME: BA-1  
 LOGGED BY: M.D. ALLOVAY  
 DATE: SEPTEMBER 1995  
 CHK'D BY: L. BUSH  
 DRILLING CONTR: LAYNE WESTERN EXPLORATION

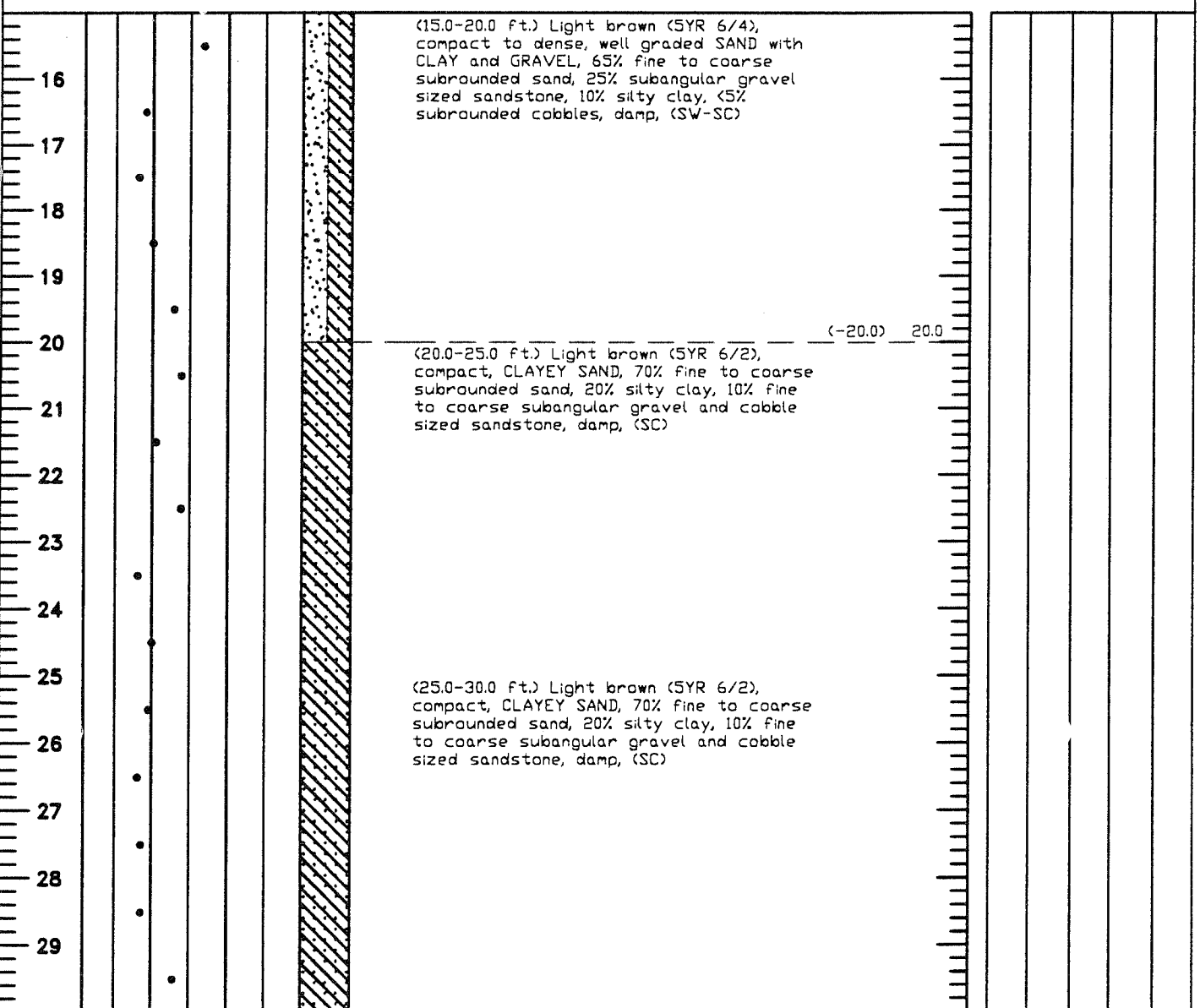
# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. BA-1	
	SAMPLING METHOD: BULK				SHEET 2 OF 4	
	DRILLING					
					START TIME 11:00	FINISH TIME 13:30
					DATE 7/25/95	DATE 7/25/95
	NORTHING: EASTING:				ELEVATION	

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS



DRILLING CONTR. LAYNE WESTERN EXPLORATION  
  
 LOGGED BY M.D. ALLOVAY  
 DATE SEPTEMBER 1995  
 JOB NO.: 243-27691170  
 FILENAME: BA-1  
 CHK'D BY L. BUSH



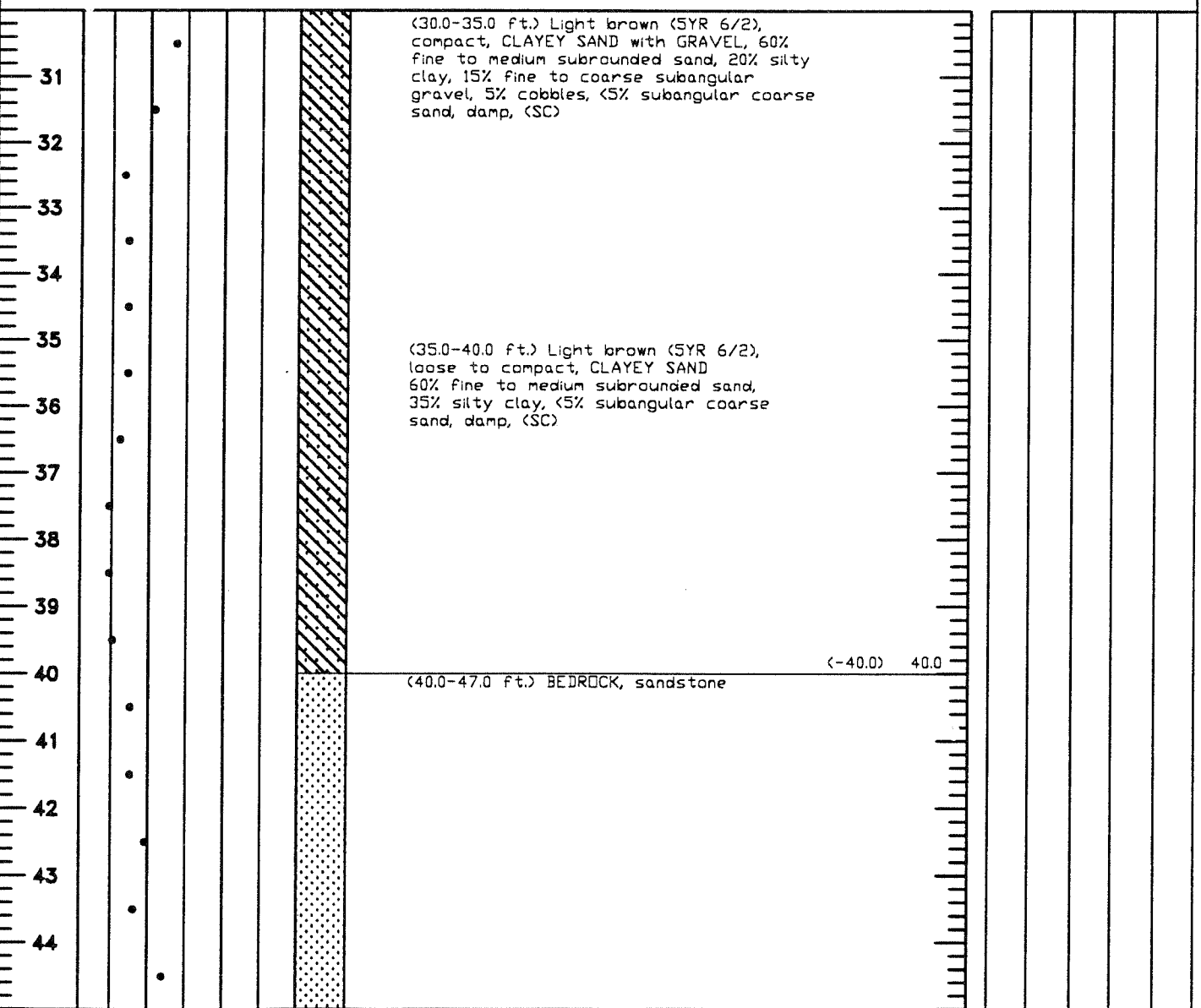
# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER	BORING NO. BA-1		
	SAMPLING METHOD: BULK	SHEET 3 OF 4		
	DRILLING			
			START TIME 11:00	FINISH TIME 13:30
			DATE 7/25/95	DATE 7/25/95
	NORTHING: EASTING:	ELEVATION		

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	60			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS



JOB NO.: 943-27691170  
 FILENAME: BA-1  
 LOGGED BY: M.D. ALLOWAY  
 DATE: SEPTEMBER 1995  
 CHK'D BY: L. BUSH  
 DRILLING CONTR: LAYNE WESTERN EXPLORATION

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER	BORING NO. BA-1		
	SAMPLING METHOD: BULK	SHEET 4 OF 4		
	DRILLING			
			START TIME 11:00	FINISH TIME 13:30
			DATE 7/25/95	DATE 7/25/95
	NORTHING: EASTING:	ELEVATION		

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

46			(40.0-47.0 ft.) BEDROCK, sandstone	
47			End of Borehole at 47.0 ft.	(-47.0) 47.0
48				
49				
50				
51				
52				
53				
54				
55				
56				
57				
58				
59				

DRILLING CONTR LAYNE WESTERN EXPLORATION  
 LOGGED BY M.D. ALLOWAY  
 DATE SEPTEMBER 1995 CHK'D BY L. BUSH  
 JOB NO.: 243-27691170  
 FILENAME: BA-1

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  <p style="text-align: center;">MINER FLAT DAM SANDSTONE RIDGE</p>	DRILLING METHOD: BECKER HAMMER	BORING NO. BA-2
	SAMPLING METHOD: BULK	SHEET 1 OF 3
	DRILLING	
	START TIME 13:30	FINISH TIME 15:20
	DATE 7/25/95	DATE 7/25/95

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS	SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
				WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

1			(0.0-5.0 ft.) Pale yellowish brown (10YR 6/2), stiff, SANDY LEAN CLAY, 60% silty clay, 40% fine to medium subrounded sand, <5% coarse subangular sand sized basalt, dry, (CL)	
2				
3				
4				
5				
6			(5.0-10.0 ft.) Pale yellowish brown (10YR 6/2), stiff to very stiff, SANDY LEAN CLAY, 60% silty clay, 40% fine to coarse subrounded sand, dry, (CL)	
7				
8				
9				
10				
11			(10.0-15.0 ft.) Pale yellowish brown (10YR 6/2), stiff to hard, SANDY LEAN CLAY, 50% silty clay, 40% fine to medium subrounded sand, 10% fine to coarse subangular gravel, <5% subangular cobbles, dry, (CL)	
12				
13				
14				

DRILLING CONTR LAYNE WESTERN EXPLORATION  
 LOGGED BY M.D. ALLOWAY  
 DATE SEPTEMBER 1995 CHK'D BY L. BUSH  
 JOB NO.: 243-27691170  
 FILENAME: BA-2

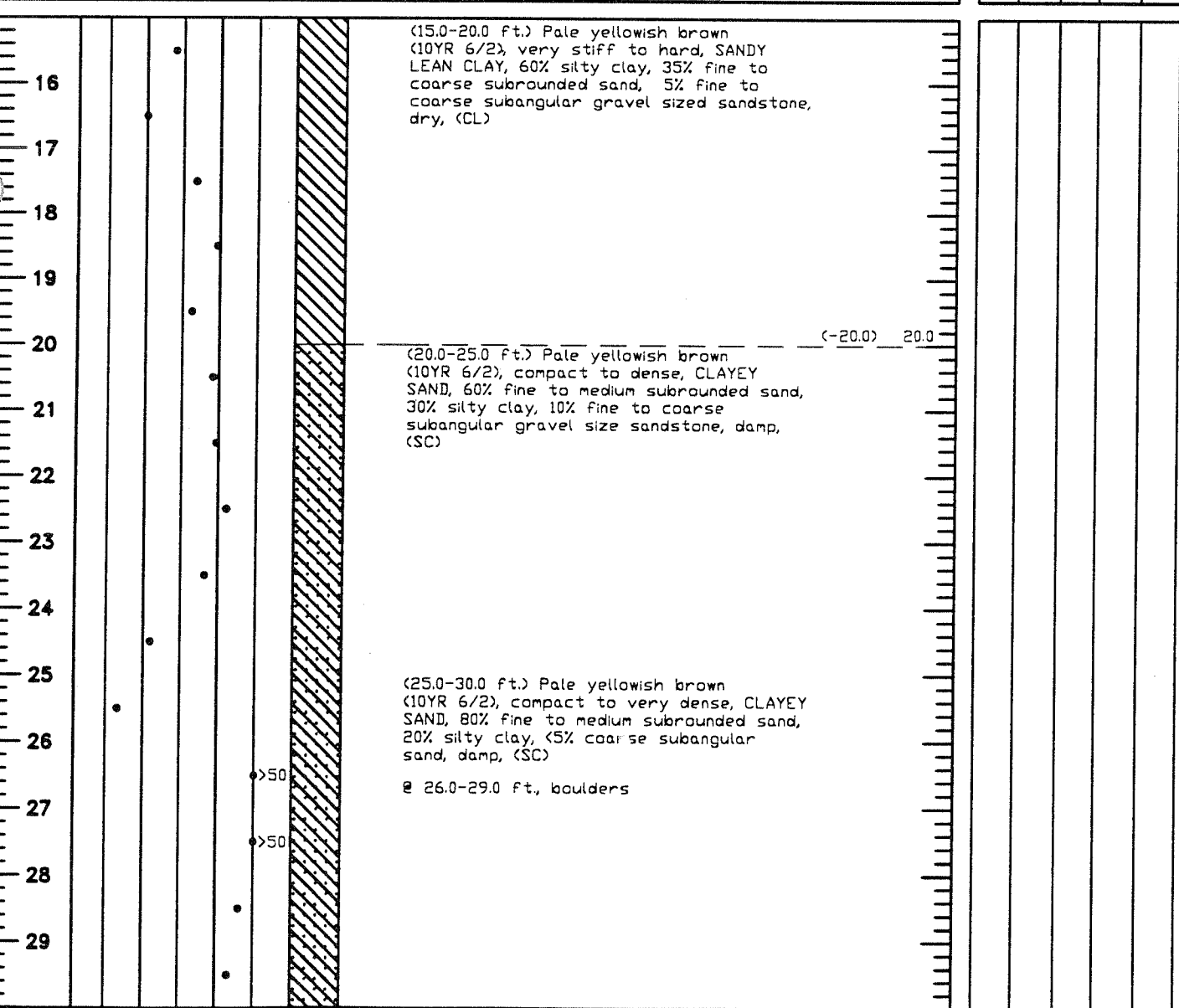
# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER	BORING NO. BA-2
	SAMPLING METHOD: BULK	SHEET 2 OF 3
	DRILLING	
	START TIME 13:30	FINISH TIME 15:20
	DATE 7/25/95	DATE 7/25/95

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD
ANGLE VERTICAL BEARING NA	SAMPLE HAMMER

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS



DRILLING CONTR LAYNE WESTERN EXPLORATION  
  
 LOGGED BY M.D. ALLOWAY  
 DATE SEPTEMBER 1995  
 CHK'D BY L. BRUSH  
  
 JOB NO.: 943-27691170  
 FILENAME: BA-2

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER	BORING NO. BA-2
	SAMPLING METHOD: BULK	SHEET 3 OF 3
	DRILLING	
	START TIME 13:30	FINISH TIME 15:20
	DATE 7/25/95	DATE 7/25/95
	NORTHING: _____ EASTING: _____ ELEVATION _____	

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD
ANGLE VERTICAL _____ BEARING NA	
SAMPLE HAMMER _____	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS	SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
				WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

31			(30.0-35.0 ft.) Pale yellowish brown (10YR 6/2), compact to dense, CLAYEY SAND, 60% fine to medium subrounded sand, 30% silty clay, 10% subangular coarse sand, damp, (SC)	
32				
33				
34				
35				
36			(35.0-40.0 ft.) Pale yellowish brown (10YR 6/2), compact, CLAYEY SAND, 50% fine to medium subrounded sand, 50% silty clay, <5% subangular coarse sand, damp, (SC)	
37				
38				
39				
40				
41			(40.0-44.0 ft.) Pale yellowish brown (10YR 6/2), compact to dense, CLAYEY SAND, 60% fine to medium and 10% coarse subrounded to subangular sand sized basalt and quartz, 30% silty clay, damp, (SC)	
42				
43				
44				
			@ 44.0 Ft. BASALT BEDROCK	(-44.0) 44.0
			End of Borehole at 44.2 ft.	(-44.2) 44.2

DRILLING CONTR \_\_\_\_\_ LAYNE WESTERN EXPLORATION  
 LOGGED BY \_\_\_\_\_ M.D. ALLOWAY  
 DATE \_\_\_\_\_ SEPTEMBER 1995 \_\_\_\_\_ CHK'D BY \_\_\_\_\_ I. BUSH  
 JOB NO.: 943-27691170  
 FILENAME: BA-2

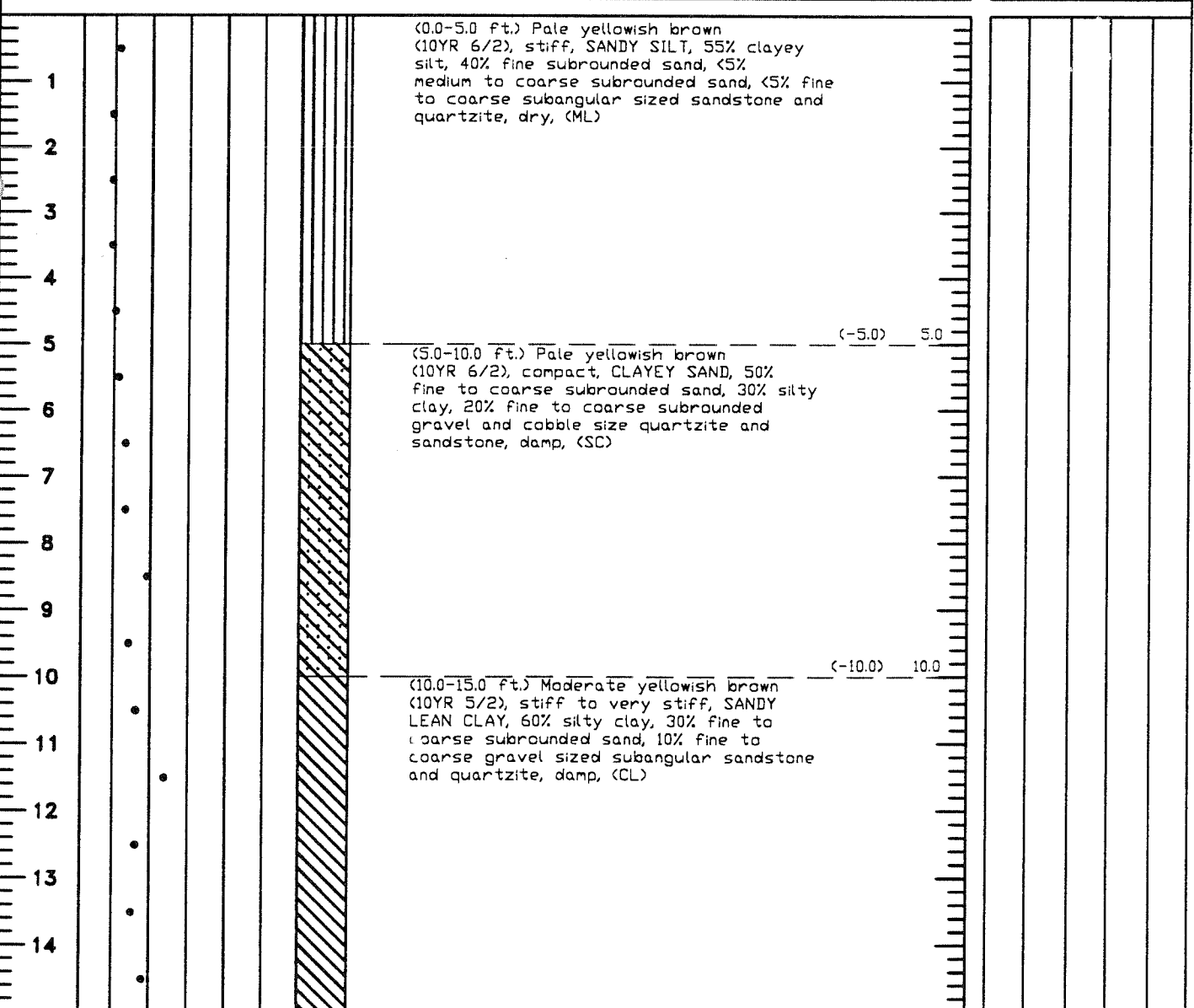
# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. BA-3	
	SAMPLING METHOD: BULK				SHEET 1 OF 3	
					DRILLING	
					START TIME	FINISH TIME
					15:20	8:40
					DATE	DATE
				7/25/95	7/26/95	

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	60			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS



JOB NO.: 243-27691170  
 FILENAME: BA-3  
 LOGGED BY: M.D. ALLOVAY  
 DATE: SEPTEMBER 1995  
 CHECK'D BY: L. BUSH  
 DRILLING CONTR: LAYNE WESTERN EXPLORATION

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  <p style="text-align: center;">MINER FLAT DAM SANDSTONE RIDGE</p>	DRILLING METHOD: BECKER HAMMER				BORING NO. BA-3	
	SAMPLING METHOD: BULK				SHEET 2 OF 3	
					DRILLING	
					START TIME 15:20	FINISH TIME 8:40
					DATE 7/25/95	DATE 7/26/95
	NORTHING: EASTING:				ELEVATION	
DRILL RIG AP1000 BECKER HAMMER			SURFACE CONDITIONS ROAD			
ANGLE VERTICAL BEARING NA						
SAMPLE HAMMER						

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	60			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

16							(15.0-20.0 ft.) Moderate yellowish brown (10YR 5/2), stiff to very stiff, SANDY LEAN CLAY, 70% silty clay, 20% fine to medium subrounded to subangular sand with basalt, 10% fine to coarse, subangular gravel sized sandstone, (CL)														
17							(20.0-25.0 ft.) Light brown (5YR 5/6), stiff to very stiff, LEAN CLAY with SAND, 80% silty clay, 10% fine to medium subrounded sand, 10% coarse sand sized subangular sandstone and basalt, (CL)														
18							(25.0-30.0 ft.) Light brown (5YR 6/2), stiff to very stiff, SANDY LEAN CLAY with GRAVEL, 65% silty clay, 20% fine to coarse subrounded sand, 15% fine to coarse gravel sized subrounded sandstone and angular quartzite, (CL)														
19																					
20																					
21																					
22																					
23																					
24																					
25																					
26																					
27																					
28																					
29																					

JOB NO.: 943-27691170  
 FILENAME: BA-3  
 LOGGED BY: M.P. ALLOWAY  
 DATE: SEPTEMBER 1995  
 CHK'D BY: L. BUSH  
 DRILLING CONTR: LAYNE WESTERN EXPLORATION

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. BA-3	
					SHEET 3 OF 3	
	SAMPLING METHOD: BULK					
					DRILLING	
					START TIME 15:20	FINISH TIME 8:40
					DATE 7/25/95	DATE 7/26/95

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

31				
32	•		(30.0-35.0 ft.) Light brown (SYR 6/2), stiff, LEAN CLAY with SAND, 50% silty clay, 50% fine to medium subrounded sand, <5% fine to coarse gravel and cobble sized subangular sandstone, damp, (CL)	
33	•			
34	•			
35	•			
36	•		(35.0-40.0 ft.) Light brown (SYR 6/2), very stiff, LEAN CLAY with SAND, 60% silty clay, 35% fine to medium subrounded sand, <5% fine to coarse gravel sized subangular sandstone, <5% coarse sand, damp, (CL)	
37	•			
38	•			
39	•			
40	•			
41	•		(40.0-41.3 ft.) Light brown (SYR 6/2), hard, LEAN SILTY CLAY with SAND, 80% silty clay, 15% fine to medium subrounded sand, 5% coarse subangular sand, damp, (CL)	(-41.3) 41.3
42	•		@ 41.3 ft., BEDROCK, basalt End of Borehole at 41.7 ft.	(-41.7) 41.7
43	•			
44	•			

DRILLING CONTR LAYNE WESTERN EXPLORATION  
  
 LOGGED BY M.D. ALLUWAY  
 DATE SEPTEMBER 1995  
 CHECK'D BY I. BUSH  
  
 JOB NO.: 243-27691170  
 FILENAME: BA-3



# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. BA-4	
	SAMPLING METHOD: BULK				SHEET 1 OF 3	
	NORTHING: EASTING:				DRILLING START TIME: 9:00 FINISH TIME: 10:05	
	ELEVATION				DATE: 7/26/95	
	DRILL RIG: AP1000 BECKER HAMMER				SURFACE CONDITIONS: ROAD	

DRILL RIG: AP1000 BECKER HAMMER	SURFACE CONDITIONS: ROAD
ANGLE: VERTICAL      BEARING: NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

1							(0.0-5.0 ft.) Light brown (SYR 5/6), soft to firm, LEAN CLAY with SAND, 70% silty clay, 20% fine to medium subrounded sand, 10% fine to coarse subrounded gravel sized sandstone and quartzite, dry, (CL)							
2														
3														
4														
5							(5.0-10.0 ft.) Light brown (SYR 5/6), firm to stiff, SANDY LEAN CLAY with GRAVEL, 60% silty clay, 20% fine to medium subrounded sand, 20% fine to coarse subrounded gravel sized sandstone and quartzite, (CL)							
6														
7														
8														
9														
10														
11							(10.0-15.0 ft.) Moderate brown (SYR 4/4), firm to stiff, LEAN CLAY, 90% silty clay, 10% fine to medium subrounded sand, (CL)							
12														
13														
14														

JOB NO.: 943-27691170  
 FILENAME: BA-4  
 LOGGED BY: M.D. ALLOWAY  
 DATE: SEPTEMBER 1995  
 CHECK'D BY: L. BUSH  
 DRILLING CONTR: LAYNIR/VEINER/REGISTRATION

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. BA-4	
	SAMPLING METHOD: BULK				SHEET 2 OF 3	
					DRILLING	
					START TIME	FINISH TIME
					DATE	DATE
					9:00	10:05
				7/26/95	7/26/95	

NORTHING: EASTING:	ELEVATION	SURFACE CONDITIONS ROAD
DRILL RIG AP1000 BECKER HAMMER		ANGLE VERTICAL BEARING NA
SAMPLE HAMMER		

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS					
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS	
16	•					/	(15.0-20.0 Ft.) Moderate brown (SYR 4/4), stiff to very stiff, LEAN CLAY with SAND, 80% silty clay, 10% fine to medium subrounded sand, 10% fine to coarse gravel and cobble sized subangular sandstone, (CL)						
17	•					/		(20.0-25.0 Ft.) Moderate brown (SYR 4/4), stiff, LEAN CLAY with SAND, 80% silty clay, 10% fine to medium subrounded sand, 10% fine to coarse subangular gravel and cobble sized subangular sandstone, (CL)					
18	•					/			(25.0-30.0 Ft.) Moderate brown (SYR 4/4), stiff, LEAN CLAY with SAND, 85% silty clay, 10% fine to medium subrounded sand, 5% fine to coarse subangular gravel, (CL)				
19	•					/							
20	•					/							
21	•					/							
22	•					/							
23	•					/							
24	•					/							
25	•					/							
26	•					/							
27	•					/							
28	•					/							
29	•					/							

DRILLING CONTR. LAYNE/WEINER/REGISTRATION  
 LOGGED BY M.D. ALLOVAY  
 DATE SEPTEMBER 1995  
 JOB NO.: 943-27691.170  
 FILENAME: BA-4  
 CHK'D BY L. BUSH

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. BA-4	
	SAMPLING METHOD: BULK				SHEET 3 OF 3	
					DRILLING	
					START TIME 9:00	FINISH TIME 10:05
					DATE 7/26/95	DATE 7/26/95
	NORTHING: EASTING:				ELEVATION	

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	60			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS
31						•	(30.0-35.0 ft.) Moderate brown (5YR 4/4), stiff to very stiff, LEAN CLAY with SAND, 65% silty clay, 30% fine to medium subrounded sand, <5% fine to coarse subangular gravel, (CL)					
32					•							
33					•							
34					•							
35					•			(35.0-40.4 ft.) Moderate brown to pale yellowish brown (5YR 4/4 to 10YR 6/2), very stiff to hard, SANDY LEAN CLAY with GRAVEL, 70% silty clay, 25% fine to medium subrounded sand, <5% fine to coarse gravel sized subangular sandstone, <5% coarse sand size angular basalt, (CL)				
36					•							
37					•							
38					•							
39					•							
40					•		(-40.4) 40.4					
41					•	■	@ 40.4 ft, BEDROCK, basalt					
42					•		(-41.4) 41.4					
43					•		End of Borehole at 41.4 ft.					
44					•							

JOB NO.: 243-27691170  
 FILENAME: BA-4  
 LOGGED BY: M.D. ALLOVAY  
 DATE: SEPTEMBER 1995  
 CHECK'D BY: L. BUSH  
 DRILLING CONTR: LAYNE/WEINER/REX/VEEDERATION

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. BA-5	
					SHEET 1 OF 3	
	SAMPLING METHOD: BULK					
					DRILLING	
					START TIME 10:05	FINISH TIME 14:20
					DATE 7/26/95	DATE 7/26/95

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

1						(0.0-5.0 ft.) Moderate brown (SYR 4/4), firm to stiff, LEAN CLAY with SAND, 75% silty clay, 15% fine to medium subrounded sand, 10% fine to coarse gravel sized subangular sandstone, <5% cobble sized sandstone, (CL)					
2						3					
4						5					
6						7					
8						9					
10						11					
12						13					
14						15					

JOB NO.: 243-27691170  
 FILENAME: BA-5  
 LOGGED BY: M.D. ALLOVAY  
 DATE: SEPTEMBER 1995  
 CHECK'D BY: L. BUSH  
 DRILLING CONTR: LAYNE WESTERN EXPLORATION

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**SAMPLING METHOD:** BULK

**BORING NO.**  
BA-5

**SHEET**  
2 OF 3

**DRILLING**

START TIME	FINISH TIME
10:05	14:20
DATE	DATE
7/26/95	7/26/95

**NORTHING:** \_\_\_\_\_  
**EASTING:** \_\_\_\_\_ **ELEVATION** \_\_\_\_\_

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS** ROAD

**ANGLE** VERTICAL **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS									
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS					
16						●	(15.0-20.0 Ft.) Moderate yellowish brown (10YR 5/2), stiff to very stiff, LEAN CLAY, 90% silty clay, 10% fine to coarse subrounded sand and subangular basalt, (CL)										
17						●											
18						●											
19						●											
20						●	(20.0-25.0 Ft.) Moderate yellowish brown (10YR 5/2), stiff, LEAN CLAY with SAND, 80% silty clay, 20% fine to coarse subrounded sand and subangular basalt, (CL)										
21						●											
22						●											
23						●											
24						●											
25						●	(25.0-30.0 Ft.) Moderate yellowish brown (10YR 5/2), stiff to very stiff, LEAN CLAY with SAND, 80% silty clay, 20% fine to coarse subrounded sand and sand size subangular basalt, (CL)										
26						●											
27						●											
28						●											
29						●											

DRILLING CONTR LAYNE, WESTERN EXPLORATION

LOGGED BY M.D. ALLOWAY  
DATE SEPTEMBER 1995  
CHK'D BY L. BUSH

JOB NO.: 943-27691170  
FILENAME: BA-5

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**NORTHING:**  
**EASTING:** **ELEVATION**

**DRILLING METHOD:** BECKER HAMMER

**SAMPLING METHOD:** BULK

<b>DRILLING</b>	
<b>START TIME</b>	<b>FINISH TIME</b>
10:05	14:20
<b>DATE</b>	<b>DATE</b>
7/26/95	7/26/95

**BORING NO.**  
BA-5

**SHEET**  
3 OF 3

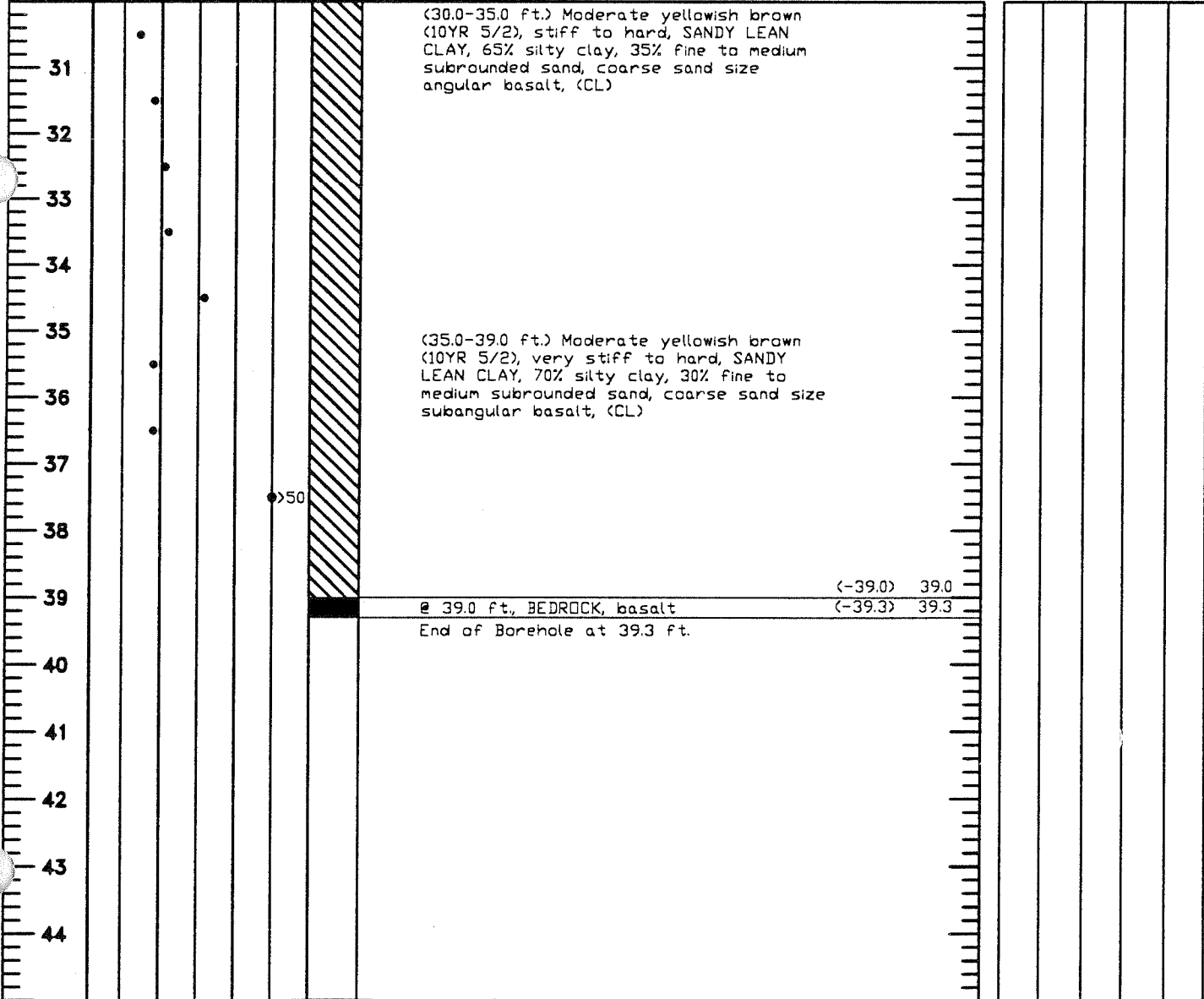
**DRILL RIG** AP1000 BECKER HAMMER

**ANGLE** VERTICAL      **BEARING** NA

**SAMPLE HAMMER**

**SURFACE CONDITIONS** ROAD

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS



DRILLING CONTR LAYNE WESTERN EXPLORATION

LOGGED BY M.D. ALLOWAY

JOB NO.: 943-27691170

DATE SEPTEMBER 1995

FILENAME: BA-5

CHK'D BY L. BUSH

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**  
BA-6

**SAMPLING METHOD:** BULK

**SHEET**  
1 OF 3

**DRILLING**

<b>START TIME</b>	<b>FINISH TIME</b>
14:20	17:30

**NORTHING:**  
**EASTING:** **ELEVATION**

<b>DATE</b>	<b>DATE</b>
7/26/95	7/26/95

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS** ROAD

**ANGLE** VERTICAL **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS				SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40			50	WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200
1						(0.0-5.0 ft.) Moderate brown (SYR 4/4), firm to stiff, LEAN CLAY with SAND, 80% silty clay, 15% fine to medium subrounded sand and subangular basalt, 5% fine to coarse gravel sized subrounded sandstone, dry, (CL)					
2											
3											
4											
5						(5.0-10.0 ft.) Moderate brown (SYR 4/4), very stiff to hard, LEAN CLAY with SAND, 80% silty clay, 15% fine to medium subrounded sand and subangular basalt, 5% coarse subrounded sand and subangular basalt, <5% fine gravel sized subrounded to angular sandstone, dry, (CL)					
6											
7											
8											
9											
10					>50	(10.0-15.0 ft.) Moderate brown to moderate yellowish brown (SYR 4/4 to 10YR 5/2), very dense, CLAYEY SAND, 80% subrounded sand, 20% silty clay, dry, (SC)					
11					>50						
12					>50	@ 10.0-11.2 ft., sandstone boulder					
13					>50						
14					>50						

DRILLING CONTR LAYNE WESTERN EXPLORATION

LOGGED BY M.D. ALLODWAY  
DATE SEPTEMBER 1995  
CHK'D BY L. BUSH

JOB NO.: 943-27691170  
FILENAME: BA-6

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**  
BA-6

**SAMPLING METHOD:** BULK

**SHEET**  
2 OF 3

**NORTHING:** \_\_\_\_\_  
**EASTING:** \_\_\_\_\_ **ELEVATION** \_\_\_\_\_

**DRILLING**

<b>START TIME</b>	<b>FINISH TIME</b>
14:20	17:30
<b>DATE</b>	<b>DATE</b>
7/26/95	7/26/95

**DRILL RIG:** AP1000 BECKER HAMMER

**SURFACE CONDITIONS:** ROAD

**ANGLE:** VERTICAL **BEARING:** NA

**SAMPLE HAMMER:**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS							
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS			
16						•>50	(15.0-20.0 ft.) Moderate brown (5YR 4/4), hard, SANDY LEAN CLAY, 70% silty clay, 30% fine to medium subrounded sand, damp, (CL)								
17						•>50	@ 16.0-16.8 ft., sandstone boulder								
18						•									
19						•									
20						•	(20.0-25.0 ft.) Pale yellowish brown (10YR 6/2), compact to dense, POORLY GRADED SAND, fine to medium subrounded sand, dry, (SP)	(-20.0) 20.0							
21						•									
22						•									
23						•									
24						•	@ 23.5-25.0 ft., fine grained sandstone boulder								
25						•	(25.0-30.0 ft.) Moderate brown (5YR 4/4), stiff to very stiff, LEAN CLAY with SAND, 55% silty clay, 45% fine to medium subrounded sand, <5% fine to coarse sand size angular basalt, damp, (CL)	(-25.0) 25.0							
26						•									
27						•									
28						•									
29						•									

DRILLING CONTR. LAYNE WESTERN EXPLORATION  
 LOGGED BY M.D. ALLOWAY  
 DATE SEPTEMBER 1995  
 FILENAME: 943-27691170  
 JOB NO.: BA-6  
 CHK'D BY L. BUSH



# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**  
BA-6

**SAMPLING METHOD:** BULK

**SHEET**  
3 OF 3

**NORTHING:** \_\_\_\_\_  
**EASTING:** \_\_\_\_\_  
**ELEVATION** \_\_\_\_\_

**DRILLING**

<b>START TIME</b>	<b>FINISH TIME</b>
14:20	17:30
<b>DATE</b>	<b>DATE</b>
7/26/95	7/26/95

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS** ROAD

**ANGLE** VERTICAL **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS
31						•	(30.0-35.0 ft.) Moderate brown (5YR 4/4), very stiff, SANDY LEAN CLAY, 55% silty clay, 45% fine to medium subrounded sand, <5% fine to coarse gravel and cobble size sandstone, (CL)					
32						•						
33						•	(35.0-36.2 ft.) Moderate brown (5YR 4/4), hard, SANDY LEAN CLAY with GRAVEL, 55% silty clay, 30% fine to medium subrounded sand, 15% fine to coarse gravel and cobble size sandstone, (CL)					
34						•						
35						•	@ 36.2 ft., BEDROCK, basalt					
36						•						
37							End of Borehole at 36.4 ft.					
38												
39												
40												
41												
42												
43												
44												

DRILLING CONTR LAYNE WESTERN EXPLORATION

LOGGED BY M.D. ALLOWAY

JOB NO.: 943-27691170

DATE SEPTEMBER 1995

FILENAME: BA-6

CHK'D BY L. BUSH

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

BA-7

**SAMPLING METHOD:** BULK

**SHEET**

1 OF 3

**DRILLING**

**START**      **FINISH**

**TIME**      **TIME**

8:00      8:30

**DATE**      **DATE**

7/27/95      7/27/95

**NORTHING:**  
**EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS** ROAD

**ANGLE** VERTICAL      **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS					
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS	
1							(0.0-5.0 ft.) Moderate brown (5Y 4/4), firm to hard, SANDY LEAN CLAY with GRAVEL, 65% silty clay, 20% fine to coarse subrounded sand, 15% fine to coarse gravel and cobble sized subrounded sandstone and quartzite, damp, (CL)						
2													
3													
4													
5							(-5.0) 5.0	(5.0-10.0 ft.) Light brown (5YR 6/4), dense to very dense, CLAYEY SAND with GRAVEL, 50% fine to medium subrounded sand, 30% fine to coarse gravel and cobble sized subrounded to subangular sandstone and subangular quartzite, 20% silty clay, damp, (SC)					
6													
7													
8							>50						
9													
10							(-10.0) 10.0	(10.0-15.0 ft.) Light brown (5YR 6/4), dense to very dense, POORLY GRADED SAND with GRAVEL, 70% fine to medium subrounded sand, 30% gravel sized subrounded and subangular sandstone, <5% silty clay, damp, (SP)					
11													
12							>50						
13													
14													

DRILLING CONTR LAYNE WESTERN EXPLORATION

LOGGED BY M.D. ALLOWAY  
DATE SEPTEMBER 1995  
CHK'D BY L. BUSH

JOB NO.: 943-27691170  
FILENAME: BA-7

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

BA-7

**SAMPLING METHOD:** BULK

**SHEET**

2 OF 3

**DRILLING**

**START FINISH**

**TIME TIME**  
8:00 8:30

**DATE DATE**

7/27/95 7/27/95

**NORTHING:  
EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS** ROAD

**ANGLE** VERTICAL **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS
16						•	(15.0-20.0 ft.) Light brown (SYR 6/4), compact to dense, POORLY GRADED SAND with GRAVEL, 70% fine to medium subrounded sand, 30% gravel sized subrounded and subangular sandstone, <5% silty clay, damp, (SP)  @ 17.0-17.5 ft, 10% silty clay					
17						•						
18						•						
19						•						
20						•	<-20.0> 20.0					
21						•	(20.0-25.0 ft.) Light brown (SYR 6/4), dense to very dense, POORLY GRADED SAND with GRAVEL, 70% fine to medium subrounded sand, 30% gravel sized subrounded and subangular sandstone, <5% silty clay, damp, (SP)					
22						•						
23						•						
24						•						
25						•>50	(25.0-28.0 ft.) Light brown (SYR 6/4), compact to dense, POORLY GRADED SAND with GRAVEL, 70% fine to medium subrounded sand, 30% gravel sized subrounded and subangular sandstone, <5% silty clay, damp, (SP)					
26						•						
27						•						
28						•	<-28.0> 28.0					
29						•	(28.0-30.5 ft.) Light brown (SYR 6/4), very stiff to hard, LEAN CLAY with SAND, 80% silty clay, 20% fine to medium subrounded sand, damp, (CL)					
29						•						

LOGGED BY \_\_\_\_\_ M.D. ALLOWAY \_\_\_\_\_  
 DATE \_\_\_\_\_ SEPTEMBER 1995 \_\_\_\_\_ CHK'D BY \_\_\_\_\_ I. BUSH \_\_\_\_\_  
 JOB NO.: 243-27691170 \_\_\_\_\_  
 FILENAME: BA-7 \_\_\_\_\_

DRILLING CONTR LAYNE, WESTERN EXPLORATION

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. BA-7	
	SAMPLING METHOD: BULK				SHEET 3 OF 3	
					DRILLING	
	START TIME 8:00		FINISH TIME 8:30			
	DATE 7/27/95		DATE 7/27/95			
	NORTHING: EASTING:		ELEVATION			

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS						
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS		
31						●	(28.0-30.5 ft.) Light brown (SYR 6/4), very stiff to hard, LEAN CLAY with SAND, 80% silty clay, 20% fine to medium subrounded sand, damp, (CL) (-30.5) 30.5 @ 30.5 ft, BEDROCK, basalt (-31.0) 31.0 End of Soil Borehole at 31.0 ft.							
32														
33														
34														
35														
36														
37														
38														
39														
40														
41														
42														
43														
44														

LOGGED BY \_\_\_\_\_ M.D. ALLOWAY  
 DATE SEPTEMBER 1995 \_\_\_\_\_  
 FILENAME: 943-27691170  
 BA-7  
 DRILLING CONTR LAYNE WESTERN EXPLORATION  
 CHK'D BY L. BUSH

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  <p style="text-align: center;">MINER FLAT DAM SANDSTONE RIDGE</p>	DRILLING METHOD: BECKER HAMMER	BORING NO. BA-8
	SAMPLING METHOD: BULK	SHEET 1 OF 1
	DRILLING	
	START TIME 8:00	FINISH TIME 9:00
	DATE 8/1/95	DATE 8/1/95
	NORTHING: EASTING:	ELEVATION
DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD	
ANGLE VERTICAL BEARING NA		
SAMPLE HAMMER		

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS	SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
				WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS
1	•		(0.0-5.0 ft.) Moderate yellowish brown (10YR 5/2), loose to compact, SILTY SAND with GRAVEL, 60% fine to medium subrounded sand, 20% silt, 20% fine to coarse subangular to subrounded gravel sized sandstone, damp, (SM)					
2	•							
3	•							
4	•							
5	•		(5.0-10.0 ft.) Moderate yellowish brown (10YR 5/2), compact to dense, SILTY SAND with GRAVEL, 60% fine to medium subrounded sand, 20% silt, 20% fine to coarse subangular to subrounded gravel sized sandstone, damp, (SM)					
6	•							
7	•							
8	•							
9	•							
10	•							
11	•		(10.0-13.2 ft.) Moderate yellowish brown (10YR 5/2), dense, SILTY SAND with GRAVEL, 60% fine to medium subrounded sand, 20% silt, 20% fine to coarse subangular to subrounded gravel sized sandstone, damp, (SM)					
12	•							
13	•							
13	•		⊕ 13.2 ft., BEDROCK, sandstone	(-13.2)	13.2			
14	•		End of Borehole at 14.0 ft.	(-14.0)	14.0			

DRILLING CONTR. LAYNE WESTERN EXPLORATION  
  
 LOGGED BY M.D. ALLOVAY  
 DATE SEPTEMBER 1995  
 JOB NO.: 943-27691170  
 FILENAME: BA-8  
 CHK'D BY L. BUSH

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

BA-9

**SAMPLING METHOD:** BULK

**SHEET**

1 OF 1

**DRILLING**

**START FINISH**

**TIME TIME**

9:00 9:35

**DATE DATE**

8/1/95 8/1/95

**NORTHING:  
EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS** ROAD

**ANGLE** VERTICAL **BEARING** NA

**SAMPLE HAMMER**

**DEPTH IN FEET  
(ELEVATION)**

**SPT (N<sub>60</sub>) EQUIVALENT  
BLOWCOUNT FROM  
BECKER HAMMER  
BLOW COUNTS**

10 30 30 40 80

**SYMBOL**

**DESCRIPTION OF MATERIAL**

**TEST RESULTS**

<b>WATER CONTENT %</b>	<b>LIQUID LIMIT %</b>	<b>PLASTICITY INDEX %</b>	<b>LESS THAN NO. 200</b>	<b>OTHER TESTS</b>

(0.0-5.0 ft.) Pale yellowish brown  
(10YR 6/2), loose to dense, SILTY SAND,  
60% fine to medium subrounded sand, 20%  
silt, 20% fine to coarse subrounded  
gravel sized sandstone, damp, (SM)

(5.0-10.0 ft.) Pale yellowish brown  
(10YR 6/2), dense to very dense, SILTY  
SAND with GRAVEL, 60% fine to medium  
subrounded sand, 20% silt, 20% fine to coarse  
subrounded gravel sized sandstone, <5%  
clay, damp, (SM)

(10.0-13.6 ft.) Pale yellowish brown  
(10YR 6/2), compact to dense, SILTY SAND,  
60% fine to medium subrounded sand,  
30% clayey silt, 10% fine to coarse  
subrounded gravel sized sandstone,  
damp, (SM)

@ 13.6 ft., BEDROCK, basalt

End of Borehole at 14.0 ft.

(-13.6) 13.6

(-14.0) 14.0

**DRILLING CONTR** LAYNE WESTERN EXPLORATION

**LOGGED BY** M.D. ALLDWAY

**DATE** SEPTEMBER 1995 **CHK'D BY** L. BUSH

**JOB NO.:** 943-27691170

**FILENAME:** BA-9

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

BC-1

**SAMPLING METHOD:** BULK

**SHEET**

1 OF 1

**DRILLING**

**START**      **FINISH**

**TIME**      **TIME**  
9:35      9:50

**DATE**      **DATE**

8/1/95      8/1/95

**NORTHING:**  
**EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS** ROAD

**ANGLE** VERTICAL      **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	60			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

1				
2			(0.0-4.5 ft.) Pale yellowish brown (10YR 6/2), loose to dense, SILTY SAND, 60% fine subrounded sand, 30% silt, 10% fine to coarse gravel sized subrounded sandstone, dry, (SM)	
3				
4				
5			⊖ 4.5 ft, BEDROCK, basalt	(-4.5) 4.5
6			End of Borehole at 5.3 ft.	(-5.3) 5.3
7				
8				
9				
10				
11				
12				
13				
14				

JOB NO.: 943-27691170  
 FILENAME: BC-1  
 LOGGED BY: M.D. ALLOWAY  
 DATE: SEPTEMBER 1995  
 CHECK'D BY: L. BUSH  
 DRILLING CONTR: LAYNE WESTERN EXPLORATION

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

BC-2

**SAMPLING METHOD:** BULK

**SHEET**

1 OF 1

**DRILLING**

**START**      **FINISH**

**TIME**      **TIME**

9:50      10:20

**DATE**      **DATE**

8/1/95      8/1/95

**NORTHING:**  
**EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS** ROAD

**ANGLE** VERTICAL      **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS					
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS	
1						[Diagonal Hatching]	(0.0-5.0 ft.) Moderate yellowish brown (10YR 5/2), loose to dense, CLAYEY SAND, 80% fine to coarse subrounded sand, 20% silty clay, damp, (SC)						
2													
3													
4													
5													
6							(5.0-6.7 ft.) Moderate yellowish brown (10YR 5/2), compact, CLAYEY SAND, 80% fine to coarse subrounded sand, 20% silty clay, damp, (SC)						
7						[Solid Black]	@ 6.7 ft, BEDROCK, basalt	(-6.7) 6.7					
							End of Soil Borehole at 7.0 ft.	(-7.0) 7.0					
8													
9													
10													
11													
12													
13													
14													

DRILLING CONTR \_\_\_\_\_ LAYNE WESTERN EXPLORATION  
 LOGGED BY \_\_\_\_\_ M.D. ALLOVAY  
 DATE \_\_\_\_\_ SEPTEMBER 1995 \_\_\_\_\_ CHK'D BY \_\_\_\_\_ L. BUSH  
 JOB NO.: \_\_\_\_\_ 943-27691170 \_\_\_\_\_  
 FILENAME: \_\_\_\_\_ BC-2 \_\_\_\_\_



# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

BC-3

**SAMPLING METHOD:** BULK

**SHEET**

1 OF 1

**DRILLING**

<b>START TIME</b>	<b>FINISH TIME</b>
10:40	10:45
<b>DATE</b>	<b>DATE</b>
8/1/95	8/1/95

**NORTHING:**  
**EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS** ROAD

**ANGLE** VERTICAL **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS								
	10	20	30	40	60			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS				
1						•	(0.0-2.0 ft.) Moderate yellowish brown (10YR 5/2), loose, SILTY SAND, 80% fine to medium sand, <5% coarse subrounded sand, 20% silt, dry, (SM)									
2						•	(-2.0) 2.0	@ 2.0 ft., BEDROCK, basalt								
3						•	(-2.5) 2.5	End of Borehole at 2.5 ft.								
4																
5																
6																
7																
8																
9																
10																
11																
12																
13																
14																

**JOB NO.:** 943-27691170  
**FILENAME:** BC-3

**LOGGED BY** M.D. ALLOVAY  
**DATE** SEPTEMBER 1995 **CHK'D BY** L. BUSH

**DRILLING CONTR** LAYNE WESTERN EXPLORATION

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

BC-4

**SAMPLING METHOD:** BULK

**SHEET**

1 OF 1

**DRILLING**

<b>START TIME</b>	<b>FINISH TIME</b>
10:45	11:00
<b>DATE</b>	<b>DATE</b>
8/1/95	8/1/95

**NORTHING:**  
**EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS** ROAD

**ANGLE** VERTICAL **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS							
	10	20	30	40	60			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS			
1							(0.0-2.5 Ft.) Moderate yellowish brown (10YR 5/2), loose to compact, SILTY SAND, 80% fine to medium subrounded sand, 20% silt, dry, (SM)								
2															
3						•>50	@ 2.5 ft. BEDROCK, basalt	(-2.5) 2.5							
3							End of Borehole at 2.8 ft.	(-2.8) 2.8							
4															
5															
6															
7															
8															
9															
10															
11															
12															
13															
14															

DRILLING CONTR LAYNE WESTERN EXPLORATION

LOGGED BY M.D. ALLOWAY  
DATE SEPTEMBER 1995  
CHK'D BY L. BUSH

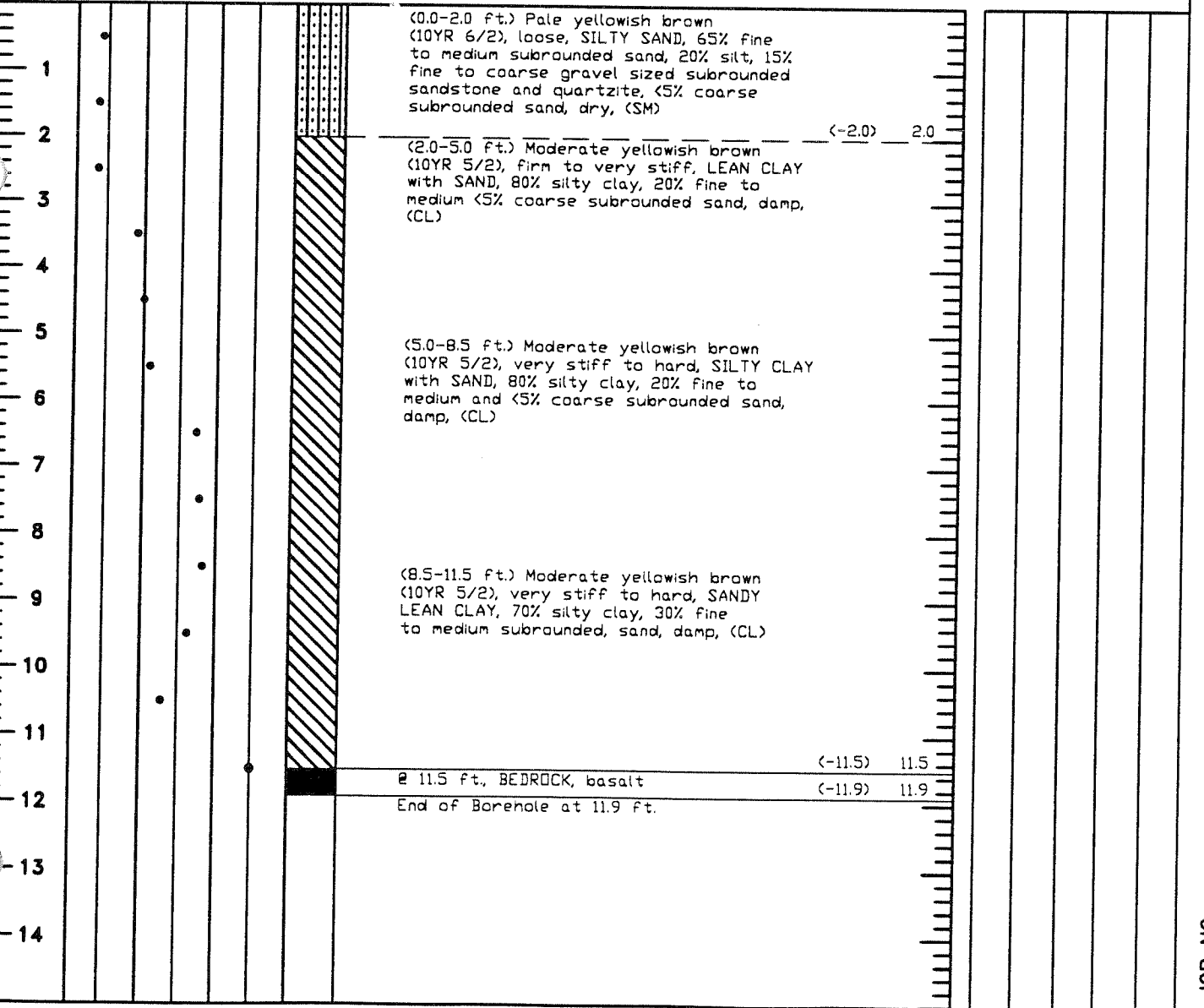
JOB NO.: 243-27621170  
FILENAME: BC-4

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  <p style="text-align: center;">MINER FLAT DAM SANDSTONE RIDGE</p>	DRILLING METHOD: BECKER HAMMER	BORING NO. BC-5
	SAMPLING METHOD: BULK	SHEET 1 OF 1
	DRILLING	
	START TIME 11:00	FINISH TIME 11:40
	DATE 8/1/95	DATE 8/1/95
	NORTHING: EASTING: ELEVATION	
DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS ROAD	
ANGLE VERTICAL BEARING NA		
SAMPLE HAMMER		

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS



DRILLING CONTR LAYNE WESTERN EXPLORATION  
  
 LOGGED BY M.D. ALLOWAY  
 DATE SEPTEMBER 1995 CHK'D BY L. BUSH  
  
 JOB NO.: 943-27691170  
 FILENAME: BC-5

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. MF-B20	
					SHEET 1 OF 1	
	SAMPLING METHOD: BULK					
					DRILLING	
					START TIME 9:50	FINISH TIME 10:30
					DATE 12/9/95	DATE 12/9/95

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	60			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

1							(0.0-2.5 ft.) Dark yellowish orange (10YR 6/6), very loose to loose, SAND, fine to medium grain, <5% fine subangular gravel, dry (SP)					
2												
3						>50	(-2.5) 2.5					
4						>50	(-4.0) 4.0					
5							End of Borehole at 4.0 ft.					
6												
7												
8												
9												
10												
11												
12												
13												
14												

DRILLING CONTR \_\_\_\_\_ NORTH AMERICAN DRILLING  
 LOGGED BY \_\_\_\_\_ L. BUSH/C. ROBINSON  
 DATE \_\_\_\_\_ FEBRUARY 1996 \_\_\_\_\_ CHK'D BY \_\_\_\_\_ L. BUSH  
 JOB NO.: 943-27691170  
 FILENAME: MFB20

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE  NORTHING: EASTING:	DRILLING METHOD: BECKER HAMMER	BORING NO. MF-B21	
	SAMPLING METHOD: BULK	SHEET 1 OF 1	
	DRILLING		
		START TIME 10:35	FINISH TIME 11:10
		DATE 12/9/95	DATE 12/9/95

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS	SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
				WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

1			(0.0-2.5 ft.) Light brown (5YR 5/6) to dark yellowish orange (10YR 6/6), very loose to compact, SAND, fine to medium grain, <5% fine subangular gravel, dry (SP)						
2				(-2.5) 2.5					
3			(2.5-5.0 ft.) BEDROCK, Sandstone						
4									
5			End of Borehole at 5.0 ft.	(-5.0) 5.0					
6									
7									
8									
9									
10									
11									
12									
13									
14									

JOB NO.: 243-27621170  
 FILENAME: MF B21  
 LOGGED BY: L. BUSH/C. ROBINSON  
 DATE: FEBRUARY 1996  
 CHK'D BY: L. BUSH  
 DRILLING CONTR: NORTH AMERICAN DRILLING

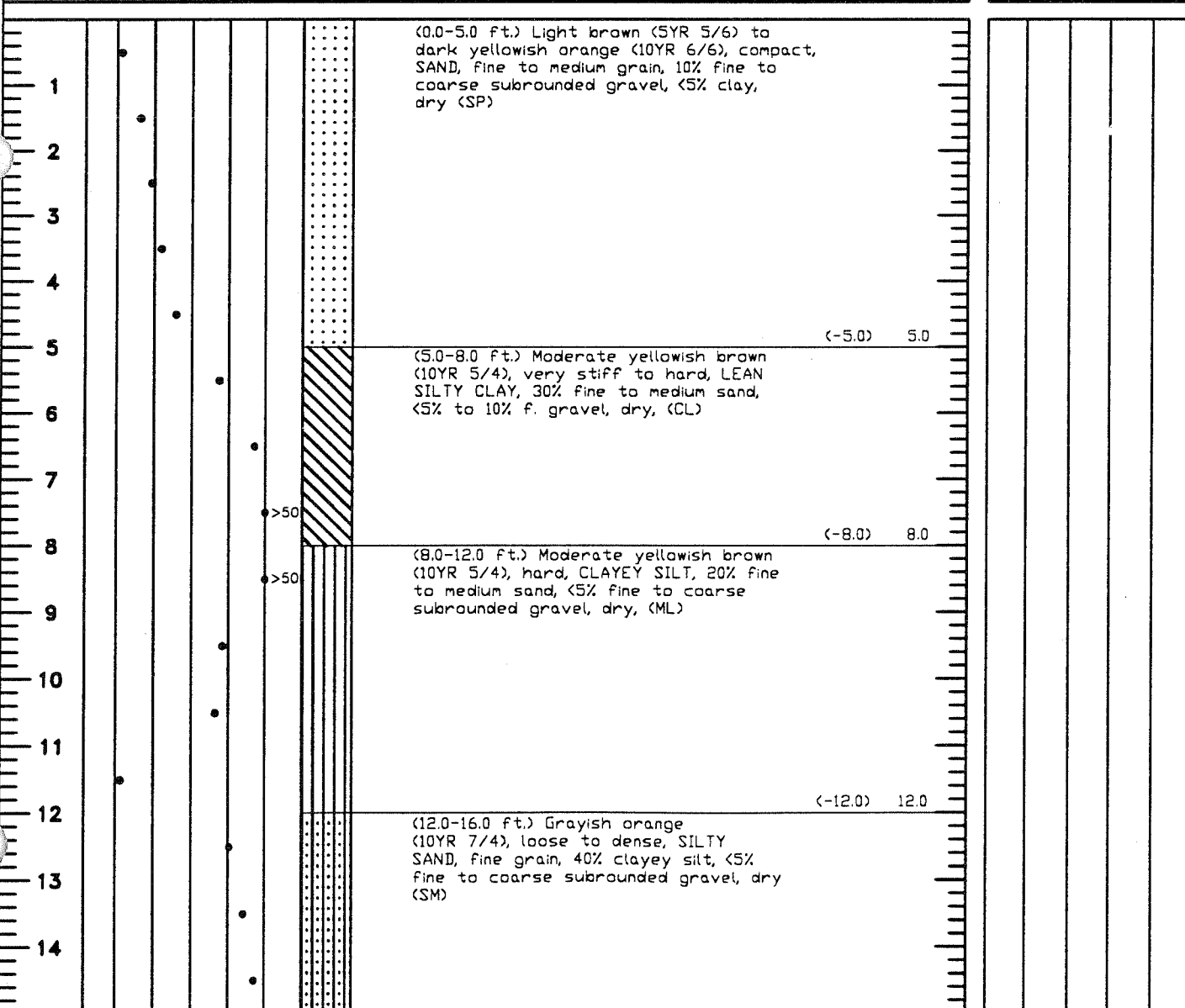
# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER	BORING NO. MF-B23		
	SAMPLING METHOD: BULK	SHEET 1 OF 4		
	DRILLING			
			START TIME 13:10	FINISH TIME 17:00
			DATE 12/9/95	DATE 12/9/95
	NORTHING: EASTING:	ELEVATION		

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS



DRILLING CONTR \_\_\_\_\_ NORTH AMERICAN DRILLING  
 LOGGED BY \_\_\_\_\_ L. BUSH/C. ROBINSON  
 DATE \_\_\_\_\_ FEBRUARY 1996 \_\_\_\_\_ CHK'D BY \_\_\_\_\_ L. BUSH  
 JOB NO.: \_\_\_\_\_ 243-27691170 \_\_\_\_\_  
 FILENAME: \_\_\_\_\_ MFB23 \_\_\_\_\_

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. MF-B23	
					SHEET 2 OF 4	
	SAMPLING METHOD: BULK					
					DRILLING	
					START TIME 13:10	FINISH TIME 17:00
					DATE 12/9/95	DATE 12/9/95

NORTHING: EASTING:	ELEVATION	SURFACE CONDITIONS
DRILL RIG AP1000 BECKER HAMMER		
ANGLE VERTICAL BEARING NA		
SAMPLE HAMMER		

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

16							(12.0-16.0 ft.) Grayish orange (10YR 7/4), loose to dense, SILTY SAND, fine grain, 40% clayey silt, <5% fine to coarse subrounded gravel, dry (SM)					
17							(16.0-19.0 ft.) Grayish orange (10YR 7/4), very stiff to hard, CLAYEY SILT, 30% fine sand, <5% fine subrounded gravel, dry (ML)					
18												
19							(19.0-26.0 ft.) Grayish orange (10YR 7/4), dense to very dense, SILTY SAND, fine grained, 30% clayey silt, <5% fine to coarse subrounded gravel, dry (SM)					
20												
21												
22												
23												
24												
25												
26							(26.0-47.0 ft.) Light brown (5YR 6/4), compact to dense, SAND, fine to medium grain, <5% to 10% fine to coarse subangular gravel, dry (SP)					
27												
28												
29												

JOB NO.: 943-27691170  
 FILENAME: MFB23  
 LOGGED BY: L. BUSH/C. ROBINSON  
 DATE: FEBRUARY 1996  
 CHK'D BY: L. BUSH  
 DRILLING CONTR: NORTH AMERICAN DRILLING

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

MF-B23

**SHEET**

3 OF 4

**SAMPLING METHOD:** BULK

**DRILLING**

**START**

TIME

13:10

**FINISH**

TIME

17:00

**DATE**

12/9/95

**DATE**

12/9/95

**NORTHING:**  
**EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS**

**ANGLE** VERTICAL **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	60			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS
31						•	(26.0-47.0 ft.) Light brown (5YR 6/4), compact to dense, SAND, fine to medium grain, <5% to 10% fine to coarse subangular gravel, dry (SP)					
32						•						
33						•						
34						•						
35						•						
36						•						
37						•						
38						•						
39						•						
40						•						
41						•						
42						•						
43						•						
44						•						

DRILLING CONTR NORTH AMERICAN DRILLING

LOGGED BY L. BUSH/C. ROBINSON  
DATE FEBRUARY 1996 CHK'D BY L. BUSH

JOB NO.: 943-27621170  
FILENAME: MFB23



# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. MF-B23	
					SHEET 4 OF 4	
	SAMPLING METHOD: BULK					
					DRILLING	
					START TIME 13:10	FINISH TIME 17:00
					DATE 12/9/95	DATE 12/9/95

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS				SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS							
	10	20	30	40			60	WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS		
46						(26.0-47.0 ft.) Light brown (5YR 6/4), compact to dense, SAND, fine to medium grain, <5% to 10% fine to coarse subangular gravel, dry (SP)								
47						(-47.0) 47.0								
48						(47.0-49.0 ft.) Moderate yellowish brown (10YR 5/4), compact to dense, SAND, fine to medium grain, <5% to 20% clay, 10% fine gravel, (SP-SC)								
49					>50	(-49.0) 49.0								
50						(49.0-51.0 ft.) BEDROCK, Sandstone								
51						(-51.0) 51.0								
52						End of Borehole at 51.0 ft.								
53														
54														
55														
56														
57														
58														
59														

JOB NO.: 943-27691170  
 FILENAME: MFB23  
 LOGGED BY: L. BUSH/C. ROBINSON  
 DATE: FEBRUARY 1996  
 CHK'D BY: L. BUSH  
 DRILLING CONTR: NORTH AMERICAN DRILLING

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER			BORING NO. MF-B24	
				SHEET 1 OF 4	
	SAMPLING METHOD: BULK				
				DRILLING	
				START TIME 14:15	FINISH TIME 17:40
				DATE 12/11/95	DATE 12/11/95

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS				SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40			50	WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200

1						(0.0-10.0 ft.) Moderate yellowish brown (10YR 5/4) to moderate brown (5YR 4/4), firm to hard, LEAN SILTY CLAY, 10% fine to medium sand, 10% fine subrounded gravel, <5% roots, dry, (CL)					
2											
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
14											

(10.0-14.0 ft.) Moderate yellowish brown (10YR 5/4), hard, sandy LEAN SILTY CLAY, 25% fine to medium sand, 5% fine subrounded to subangular gravel, <5% carbon, + sandstone cobble at 10', damp, (CL)

(14.0-57.7 ft.) Moderate yellowish brown (10YR 5/4), compact to very dense, interbedded SILTY SAND and SAND, <5% to 20% fine subrounded to subangular gravel, <5% subrounded to subangular sandstone cobbles to 4.5", 6' silty clay seams at 34' and 55', dry, (SM and SP)

(-14.0) 14.0

>50

JOB NO.: 943-27691170  
 FILENAME: MFB24  
 LOGGED BY: L. BUSH/C. ROBINSON  
 DATE: FEBRUARY 1996  
 CHK'D BY: L. BUSH  
 DRILLING CONTR: NORTH AMERICAN DRILLING

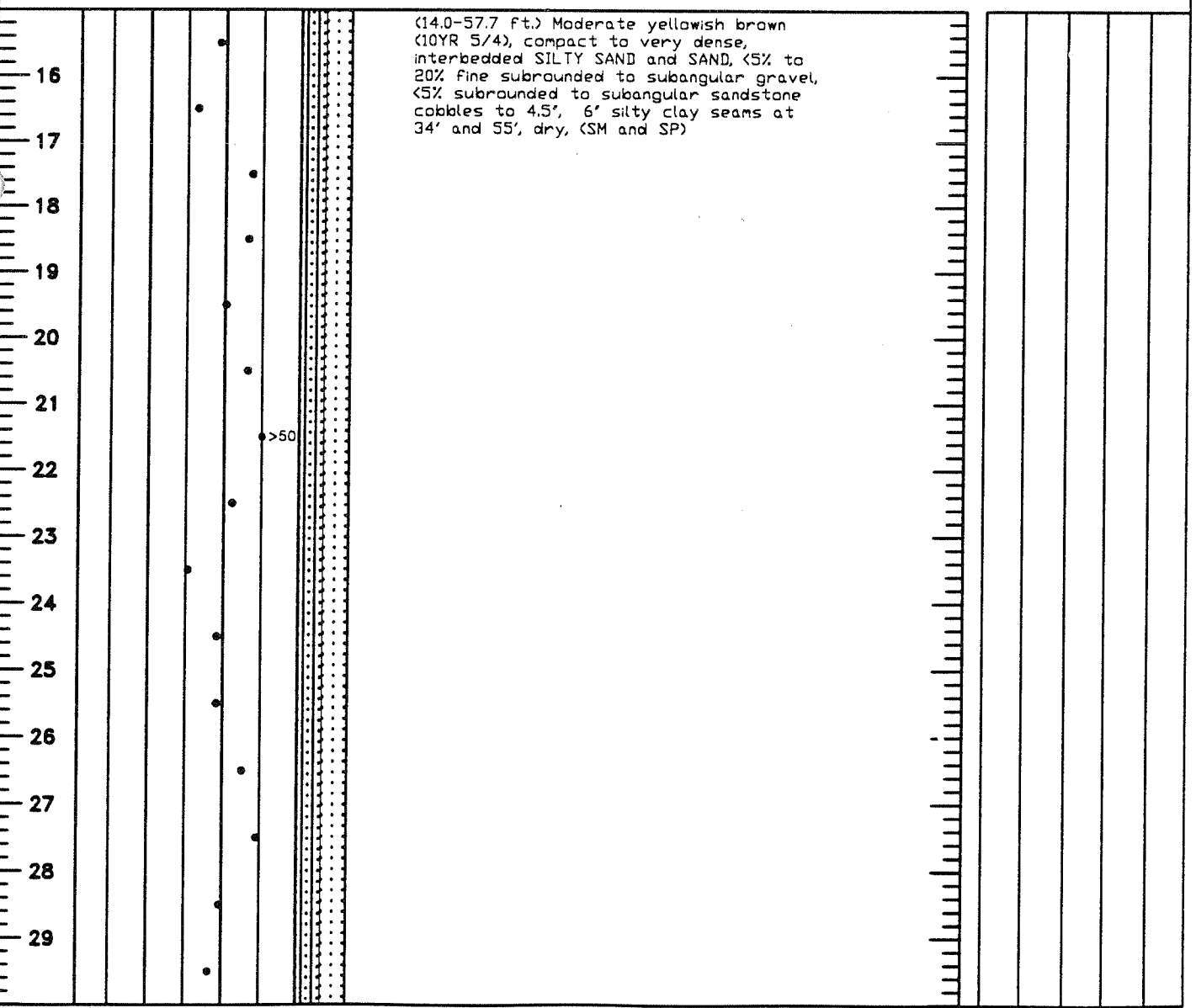
# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER	BORING NO. MF-B24
	SAMPLING METHOD: BULK	SHEET 2 OF 4
	DRILLING	
	START TIME: 14:15      FINISH TIME: 17:40	
	DATE: 12/11/95      DATE: 12/11/95	
	NORTHING: _____      EASTING: _____      ELEVATION: _____	

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL      BEARING NA	SAMPLE HAMMER

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS



DRILLING CONTR \_\_\_\_\_ NORTH AMERICAN DRILLING  
 LOGGED BY \_\_\_\_\_ L. BUSH/C. ROBINSON  
 DATE \_\_\_\_\_ FEBRUARY 1996 \_\_\_\_\_ CHK'D BY \_\_\_\_\_ L. BUSH  
 JOB NO.: \_\_\_\_\_ 943-27691170  
 FILENAME: \_\_\_\_\_ MFB24

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. MF-B24	
					SHEET 3 OF 4	
	SAMPLING METHOD: BULK					
					DRILLING	
					START TIME 14:15	FINISH TIME 17:40
					DATE 12/11/95	DATE 12/11/95

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS
31						•	(14.0-57.7 ft.) Moderate yellowish brown (10YR 5/4), compact to very dense, interbedded SILTY SAND and SAND, <5% to 20% fine subrounded to subangular gravel, <5% subrounded to subangular sandstone cobbles to 4.5", 6" silty clay seams at 34' and 55', dry, (SM and SP)					
32						•						
33						•						
34						•						
35						•						
36						•						
37						•						
38						•						
39						•						
40						•						
41						•						
42						•						
43						•						
44						•						

DRILLING CONTR \_\_\_\_\_ NORTH AMERICAN DRILLING  
 LOGGED BY \_\_\_\_\_ L. BUSH/C. ROBINSON  
 DATE \_\_\_\_\_ FEBRUARY 1996 \_\_\_\_\_ CHK'D BY \_\_\_\_\_ L. BUSH  
 JOB NO.: \_\_\_\_\_ 943-27691170 \_\_\_\_\_  
 FILENAME: \_\_\_\_\_ MFB24 \_\_\_\_\_

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. MF-B24	
					SHEET 4 OF 4	
	SAMPLING METHOD: BULK					
					DRILLING	
					START TIME 14:15	FINISH TIME 17:40
					DATE 12/11/95	DATE 12/11/95

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS
46						•	(14.0-57.7 ft.) Moderate yellowish brown (10YR 5/4), compact to very dense, interbedded SILTY SAND and SAND, <5% to 20% fine subrounded to subangular gravel, <5% subrounded to subangular sandstone cobbles to 4.5", 6" silty clay seams at 34' and 55', dry, (SM and SP)					
47						•						
48						•						
49						•						
50						•						
51						•						
52						•						
53						•						
54						•						
55						•						
56						•						
57						•	(-57.7) 57.7					
58						•	(57.7-59.0 ft.) BEDROCK, Basalt					
59						•	(-59.0) 59.0					
							End of Borehole at 59.0 ft.					

JOB NO.: 243-27691170  
 FILENAME: MFB24  
 LOGGED BY: L. BUSH/C. ROBINSON  
 DATE: FEBRUARY 1996  
 CHK'D BY: L. BUSH  
 DRILLING CONTR: NORTH AMERICAN DRILLING

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

MF-B25

**SAMPLING METHOD:** BULK

**SHEET**

1 OF 4

**DRILLING**

**START**      **FINISH**

**TIME**      **TIME**  
9:15      13:30

**DATE**      **DATE**

12/12/95      12/12/95

**NORTHING:**  
**EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS**

**ANGLE** VERTICAL      **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS
1						•	(0.0-7.0 ft.) Moderate brown (SYR 4/4), soft to very stiff, LEAN SILTY CLAY, 20% to 30% fine to medium sand, 10% fine subangular gravel, <5% roots, damp to dry, (CL)					
2						•						
3						•						
4						•						
5						•						
6						•						
7						•		(-7.0)    7.0				
8						•	(7.0-9.0 ft.) Dark yellowish brown (10YR 6/6), compact, SAND, 10% fine to coarse subrounded gravel, 10% silty clay, dry (SP-SC)					
9						•		(-9.0)    9.0				
10						•	(9.0-13.0 ft.) Moderate brown (SYR 4/4), very stiff to hard, LEAN SILTY CLAY, 10% fine to coarse sand, <5% fine to coarse subrounded to rounded gravel, quartzite cobble at 11', damp (CL)					
11						•						
12						•						
13						•		(-13.0)    13.0				
14						•	(13.0-21.0 ft.) Moderate brown (SYR 4/4), compact to dense, SILTY SAND, 10% fine to coarse subrounded to rounded gravel, 10% silty clay, dry (SM)					
14						•						

**JOB NO.:** 943-27691170      **LOGGED BY** L. BUSH/C. ROBINSON  
**FILENAME:** MFB25      **DATE** FEBRUARY 1996      **CHK'D BY** L. BUSH  
**DRILLING CONTR** NORTH AMERICAN DRILLING

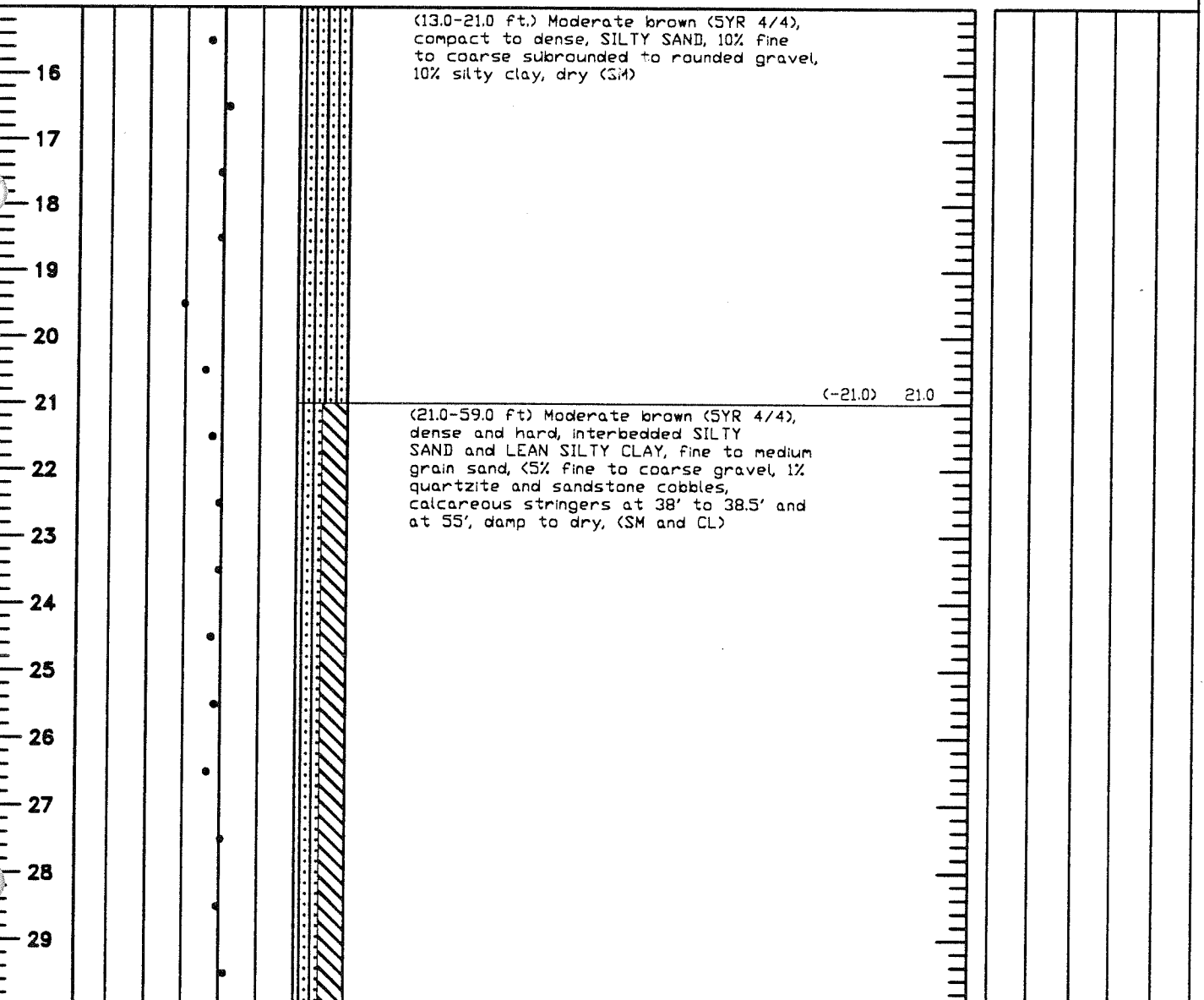
# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. MF-B25	
	SAMPLING METHOD: BULK				SHEET 2 OF 4	
					DRILLING	
					START TIME	FINISH TIME
					9:15	13:30
					DATE	DATE
12/12/95					12/12/95	

NORTHING: _____	EASTING: _____	ELEVATION _____
DRILL RIG: AP1000 BECKER HAMMER	SURFACE CONDITIONS _____	
ANGLE: VERTICAL	BEARING: NA	
SAMPLE HAMMER _____		

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS



NORTH AMERICAN DRILLING  
 DRILLING CONTR  
 LOGGED BY L. BUSH/C. ROBINSON  
 DATE FEBRUARY 1996  
 FILENAME: 243-27691170 MFB25  
 CHK'D BY L. BUSH

# SOIL BOREHOLE LOG

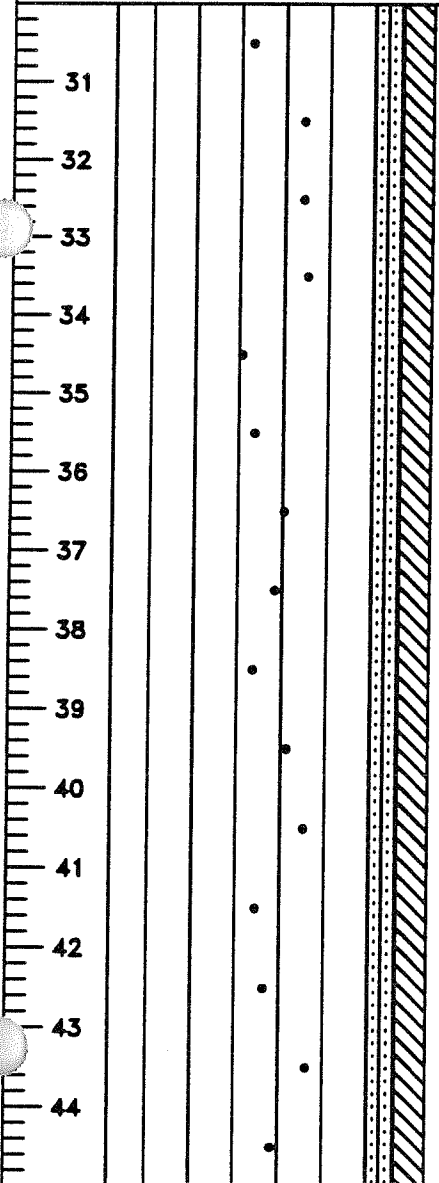


SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE  NORTHING: EASTING: _____ ELEVATION _____	DRILLING METHOD: BECKER HAMMER	BORING NO. MF-B25
	SHEET 3 OF 4	
	SAMPLING METHOD: BULK	
	DRILLING	
	START TIME 9:15	FINISH TIME 13:30

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

(21.0-59.0 ft) Moderate brown (5YR 4/4), dense and hard, interbedded SILTY SAND and LEAN SILTY CLAY, fine to medium grain sand, <5% fine to coarse gravel, 1% quartzite and sandstone cobbles, calcareous stringers at 38' to 38.5' and at 55', damp to dry, (SM and CL)



DRILLING CONTR \_\_\_\_\_ NORTH AMERICAN DRILLING  
 LOGGED BY \_\_\_\_\_ L. BUSH/C. ROBINSON  
 DATE \_\_\_\_\_ FEBRUARY 1996 \_\_\_\_\_ CHK'D BY \_\_\_\_\_ L. BUSH  
 JOB NO.: \_\_\_\_\_ 943-27691170  
 FILENAME: \_\_\_\_\_ MFB25



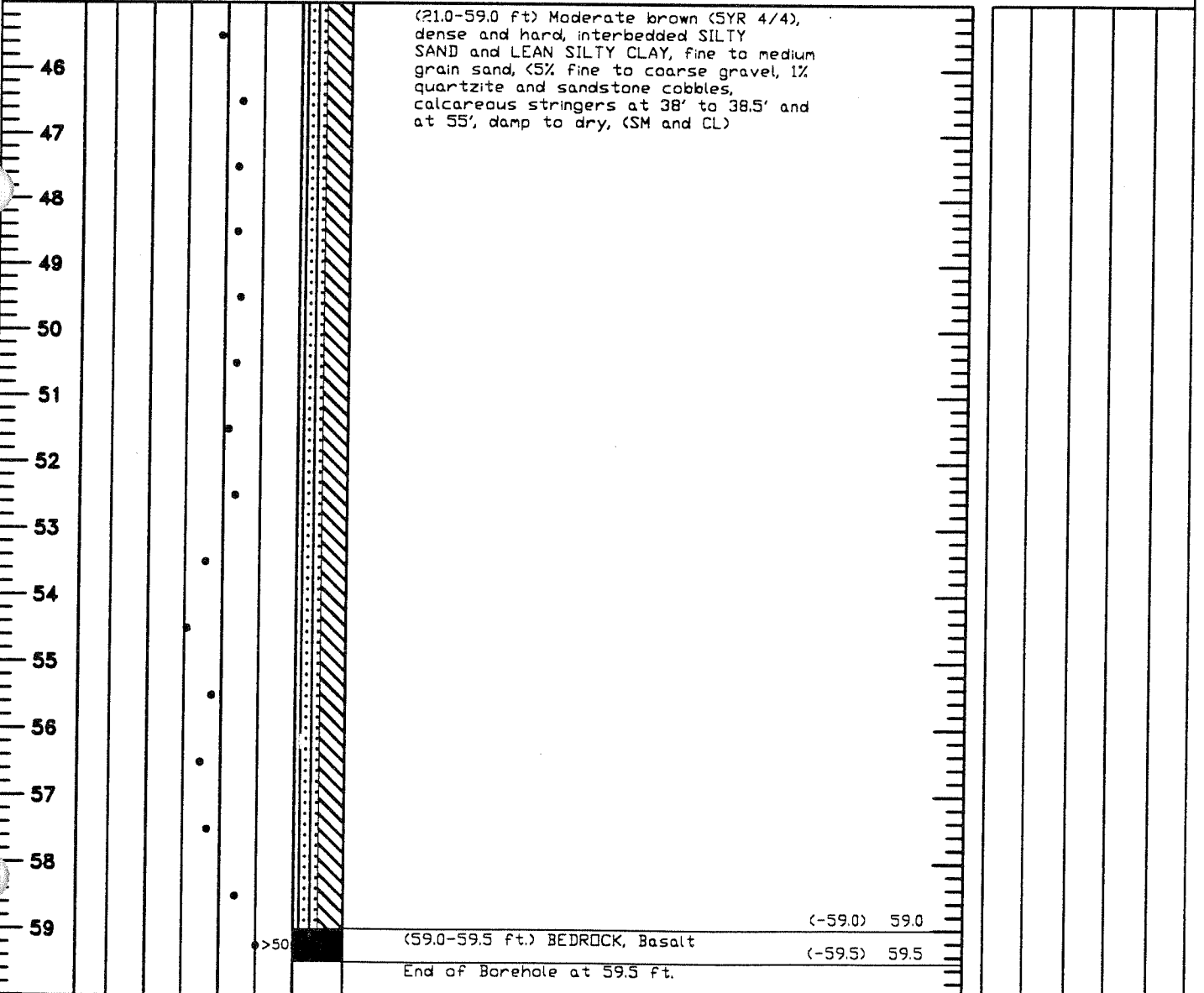
# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE  NORTHING: EASTING:  ELEVATION	DRILLING METHOD: BECKER HAMMER	BORING NO. MF-B25
	SAMPLING METHOD: BULK	SHEET 4 OF 4
	DRILLING	
	START TIME 9:15	FINISH TIME 13:30
	DATE 12/12/95	DATE 12/12/95
	DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL BEARING NA	SAMPLE HAMMER

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS



JOB NO.: 943-27691170  
 FILENAME: MFB25  
 LOGGED BY: L. BUSH/C. ROBINSON  
 DATE: FEBRUARY 1996  
 CHK'D BY: L. BUSH

DRILLING CONTR NORTH AMERICAN DRILLING

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

MF-B26

**SAMPLING METHOD:** BULK

**SHEET**

1 OF 2

**DRILLING**

**START**      **FINISH**

**TIME**      **TIME**

15:05      16:23

**DATE**      **DATE**

12/12/95      12/12/95

**NORTHING:**

**EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS**

**ANGLE** VERTICAL      **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS					
	10	20	30	40	60			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS	
1						•	(0.0-3.0 ft.) Grayish orange (10YR 7/4), soft to firm, SILT, <5% fine sand, dry (ML)						
2						•							
3						•	(-3.0) 3.0	(3.0-5.0 ft.) Pale yellowish brown (10YR 6/2) to light brown (5YR 6/4), loose, SAND, fine to medium grain, <5% coarse sand, 20% fine to coarse subangular gravel to 3', dry (SP)					
4						•							
5						•	(-5.0) 5.0	(5.0-15.0 ft.) Light brown, (5YR 6/4), firm to hard, LEAN SILTY CLAY, and SILTY SAND, fine grained, <5% fine gravel, dry (CL)					
6						•							
7						•							
8						•							
9						•							
10						•							
11						•							
12						•							
13						•							
14						•							

JOB NO.: 943-27691170      FILENAME: MFB26  
 LOGGED BY: L. BUSH/C. ROBINSON      DATE: FEBRUARY 1996  
 CHECK'D BY: L. BUSH  
 DRILLING CONTR: NORTH AMERICAN DRILLING

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**  
  
MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**SAMPLING METHOD:** BULK

**BORING NO.**  
MF-B26

**SHEET**  
2 OF 2

**DRILLING**

<b>START TIME</b>	<b>FINISH TIME</b>
15:05	16:23
<b>DATE</b>	<b>DATE</b>
12/12/95	12/12/95

**NORTHING:** \_\_\_\_\_  
**EASTING:** \_\_\_\_\_  
**ELEVATION** \_\_\_\_\_

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS**

**ANGLE** VERTICAL **BEARING** NA

**SAMPLE HAMMER**

**TEST RESULTS**

<b>WATER CONTENT %</b>	<b>LIQUID LIMIT %</b>	<b>PLASTICITY INDEX %</b>	<b>LESS THAN NO. 200</b>	<b>OTHER TESTS</b>

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS
16						•	(15.0-17.0 ft.) Pale yellowish brown (10YR 6/2), dense to very dense, SAND, fine grained, 10% silty clay, dry, (SP)					
17						•>50	(17.0-18.0 ft) BEDROCK, Sandstone					
18						•>50	End of Borehole at 18.0 ft.					
19												
20												
21												
22												
23												
24												
25												
26												
27												
28												
29												

DRILLING CONTR \_\_\_\_\_ NORTH AMERICAN DRILLING

**JOB NO.:** 943-27691170  
**FILENAME:** MFB26

**LOGGED BY:** L. BUSH/C. ROBINSON  
**DATE:** FEBRUARY 1996

**CHK'D BY:** L. BUSH

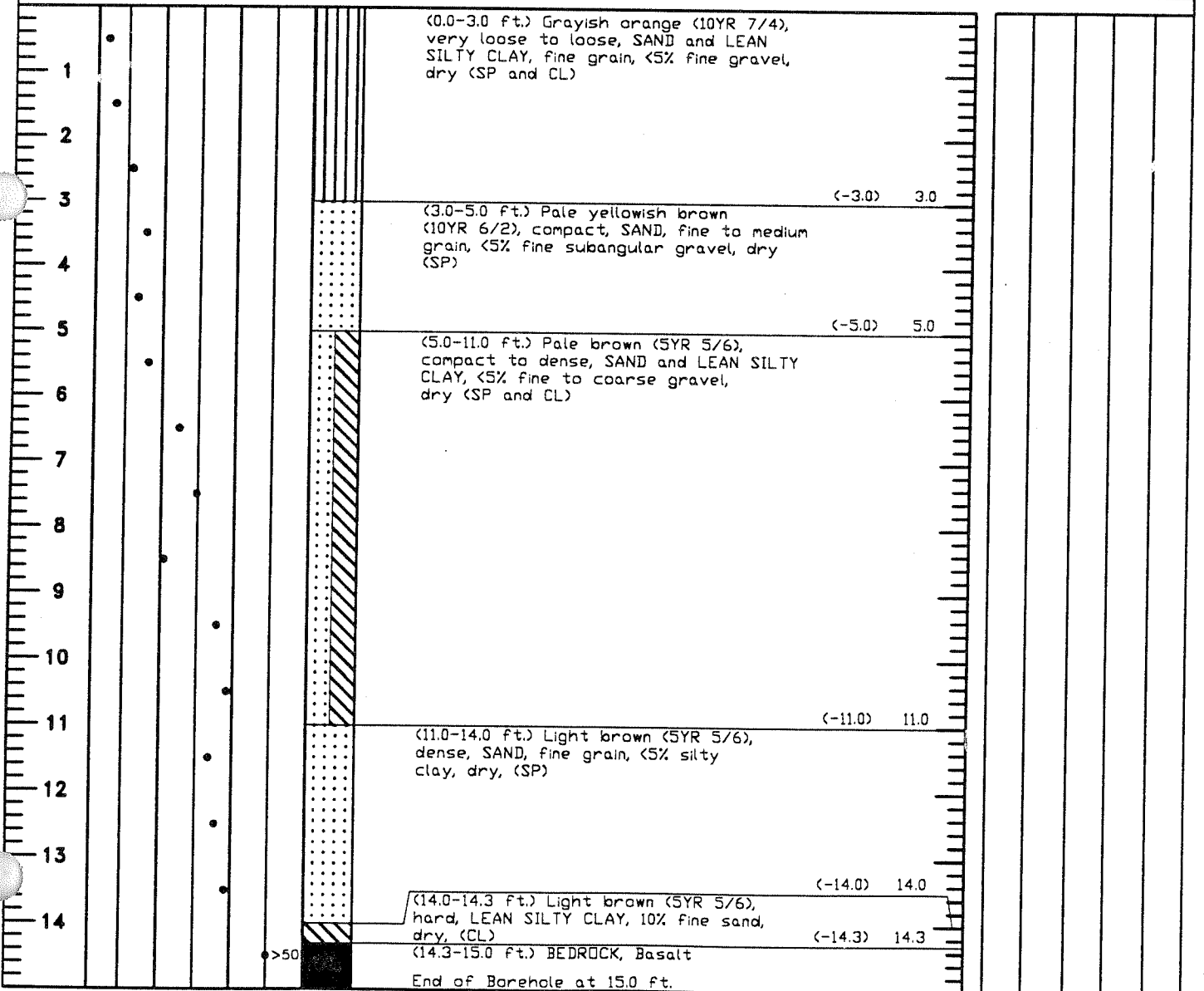
# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER	BORING NO. MF-B27
	SAMPLING METHOD: BULK	SHEET 1 OF 1
	DRILLING	
	START TIME 16:35	FINISH TIME 17:16
	DATE 12/12/95	DATE 12/12/95

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS



DRILLING CONTR NORTH AMERICAN DRILLING

JOB NO.: 943-27691170  
 FILENAME: MFB27  
 LOGGED BY: L. BUSH/C. ROBINSON  
 DATE: FEBRUARY 1996  
 CHK'D BY: L. BUSH

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**  
MF-B28

**SAMPLING METHOD:** BULK

**SHEET**  
1 OF 1

**DRILLING**

<b>START TIME</b>	<b>FINISH TIME</b>
7:55	10:15
<b>DATE</b>	<b>DATE</b>
12/13/95	12/13/95

**NORTHING:**  
**EASTING:** **ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS**

**ANGLE** VERTICAL **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS					
	10	20	30	40	60			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS	
1						•••••	(0.0-3.0 ft.) Dark yellowish orange (10YR 6/6) to pale yellowish brown (10YR 6/2), loose to compact, SAND, fine grain, 10% fine angular gravel, dry (SP)						
2						•••••							
3						•••••	(-3.0) 3.0	(3.0-4.5 ft.) Moderate brown (5YR 4/4), very stiff, LEAN SILTY CLAY, 20% fine to medium sand, <5% fine gravel, dry, (CL)					
4						•••••	(-4.5) 4.5	(4.5-5.0 ft.) Light brown (5YR 5/6), dense, SAND, fine to medium grain, 10% to 20% fine to coarse subangular to subrounded gravel, <5% silty clay, dry (SP)					
5						•••••	(-5.0) 5.0	(5.0-10.2 ft.) BEDROCK, Sandstone					
6						•>50							
7						•>50							
8						•>50							
9						•>50							
10						•>50	(-10.2) 10.2	End of Borehole at 10.2 ft.					
11													
12													
13													
14													

DRILLING CONTR \_\_\_\_\_ NORTH AMERICAN DRILLING

LOGGED BY \_\_\_\_\_ L. BUSH/C. ROBINSON  
DATE \_\_\_\_\_ FEBRUARY 1996 \_\_\_\_\_ CHK'D BY \_\_\_\_\_ L. BUSH

JOB NO.: 243-27691170  
FILENAME: MFB28

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER				BORING NO. MF-B29	
					SHEET 1 OF 1	
	SAMPLING METHOD: BULK				DRILLING	
					START TIME 10:25	FINISH TIME 10:45
					DATE 12/13/95	DATE 12/13/95

NORTHING: \_\_\_\_\_  
 EASTING: \_\_\_\_\_  
 ELEVATION \_\_\_\_\_

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS
1						•	(0.0-6.0 ft.) Moderate yellowish brown (10YR 5/4) to light brown (5YR 5/6), loose to dense, SAND, fine to medium grain, 10% fine to coarse subangular gravel, <5% clay, dry (SP)					
2						•						
3						•						
4						•						
5						•						
6						•		(-6.0) 6.0				
7						•	(6.0-9.0 ft.) BEDROCK, Sandstone					
8						•						
9						•		(-9.0) 9.0				
10							End of Borehole at 9.0 ft.					
11												
12												
13												
14												

DRILLING CONTR \_\_\_\_\_ NORTH AMERICAN DRILLING  
 LOGGED BY \_\_\_\_\_ L. BUSH/C. ROBINSON  
 DATE \_\_\_\_\_ FEBRUARY 1996  
 CHK'D BY \_\_\_\_\_ L. BUSH  
 JOB NO.: \_\_\_\_\_ 243-27691170  
 FILENAME: \_\_\_\_\_ MFB29

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE  NORTHING: EASTING: _____ ELEVATION _____	DRILLING METHOD: BECKER HAMMER	BORING NO. MF-B30	
	SHEET 1 OF 1		
	SAMPLING METHOD: BULK	DRILLING	
		START TIME 11:00	FINISH TIME 11:30
		DATE 12/13/95	DATE 12/13/95

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS									
	10	20	30	40	60			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS					
1						•	(0.0-3.5 ft.) Moderate yellowish brown (10YR 5/4), loose to compact, SAND, fine to medium grain, <5% fine angular gravel, dry (SP)										
2						•											
3						•											
4						•	(-3.5) 3.5										
5						•	(3.5-9.0 ft.) BEDROCK, Sandstone										
6						•											
7						•											
8						•											
9						•	(-9.0) 9.0										
10							End of Borehole at 9.0 ft.										
11																	
12																	
13																	
14																	

DRILLING CONTR \_\_\_\_\_ NORTH AMERICAN DRILLING  
 LOGGED BY \_\_\_\_\_ L. BUSH/C. ROBINSON  
 DATE \_\_\_\_\_ FEBRUARY 1996 \_\_\_\_\_ CHK'D BY \_\_\_\_\_ L. BUSH  
 JOB NO.: \_\_\_\_\_ 943-27691170  
 FILENAME: \_\_\_\_\_ MFB30

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

MF-B31

**SAMPLING METHOD:** BULK

**SHEET**

1 OF 2

**DRILLING**

**START**      **FINISH**

**TIME**      **TIME**  
12:30      13:55

**DATE**      **DATE**

12/13/95      12/13/95

**NORTHING:**

**EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS**

**ANGLE** VERTICAL      **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS							
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS			
1						•	(0.0-3.0 ft.) Pale yellowish brown (10YR 6/2), very loose to loose, SILTY SAND, fine to medium grain, <5% fine to coarse subrounded gravel, dry (SM)								
2						•									
3						•	(-3.0) 3.0								
4						•	(3.0-12.0 ft.) Moderate yellowish brown (10YR 6/2) to moderate brown (5YR 4/4), firm to hard, LEAN SILTY CLAY, 5% to 40% fine sand, 10% fine subrounded gravel, dry (CL)								
5						•									
6						•									
7						•									
8						•									
9						•									
10						•									
11						•									
12						•	(-12.0) 12.0								
13						•	(12.0-18.0 ft.) Moderate yellowish brown (10YR 6/2) to moderate brown (5YR 4/4), dense, CLAYEY SAND, fine grain, 40% silty clay, <5% fine gravel, dry (SC)								
14						•									

DRILLING CONTR NORTH AMERICAN DRILLING

LOGGED BY L. BUSH/C. ROBINSON

DATE FEBRUARY 1996 CHK'D BY L. BUSH

JOB NO.: 943-27691170

FILENAME: MFB31



# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

MF-B31

**SHEET**

2 OF 2

**SAMPLING METHOD:** BULK

**DRILLING**

**START**      **FINISH**

**TIME**      **TIME**

12:30      13:55

**DATE**      **DATE**

12/13/95      12/13/95

**NORTHING:**  
**EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS**

**ANGLE** VERTICAL      **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS						
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS		
16						•	(12.0-18.0 ft.) Moderate yellowish brown (10YR 6/2) to moderate brown (5YR 4/4), dense, CLAYEY SAND, fine grain, 40% silty clay, <5% fine gravel, dry (SC)							
17						•								
18						•	(18.0-18.5 ft.) BEDROCK, Basalt	(-18.0)	18.0					
18.5						•	End of Borehole at 18.5 ft.	(-18.5)	18.5					
19						•								
20						•								
21						•								
22						•								
23						•								
24						•								
25						•								
26						•								
27						•								
28						•								
29						•								

DRILLING CONTR \_\_\_\_\_ NORTH AMERICAN DRILLING

LOGGED BY \_\_\_\_\_ L. BUSH/C. ROBINSON  
DATE \_\_\_\_\_ FEBRUARY 1996      CHK'D BY \_\_\_\_\_ L. BUSH

JOB NO.: 943-27691170  
FILENAME: MFB31

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

MF-B32

**SAMPLING METHOD:** BULK

**SHEET**

1 OF 4

**DRILLING**

**START FINISH**

**TIME TIME**  
14:20 16:00

**DATE DATE**

12/13/95 12/13/95

**NORTHING:  
EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS**

**ANGLE** VERTICAL **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS
1						•••••	(0.0-3.0 ft.) Moderate yellowish brown (10YR 5/4), very loose to loose, SAND, fine to medium grain, dry, (SP)					
2						•••••						
3						•••••	(-3.0) 3.0					
4						▨▨▨▨▨	(3.0-3.5 ft.) Moderate brown (5YR 4/4), very stiff, LEAN SANDY SILTY CLAY, fine to medium grain, dry (CL) (-3.5) 3.5					
5						•••••	(3.5-6.0 ft.) Light brown (5YR 5/6), loose to compact, CLAYEY SAND, fine to medium grain, <5% fine to coarse subangular gravel to 4%, damp (SC)					
6						•••••	(-6.0) 6.0					
7						•••••	(6.0-29.0 ft.) Moderate brown (5YR 4/4), compact to dense, SAND, fine to medium grain, <5% clay, <5% to 20% fine subangular gravel, <5% sandstone cobbles to 4", dry (SP)					
8						•••••						
9						•••••						
10						•••••						
11						•••••						
12						•••••						
13						•••••						
14						•••••						

JOB NO.: 943-27691170  
 FILENAME: MFB32  
 LOGGED BY: L. BUSH/C. ROBINSON  
 DATE: FEBRUARY 1996  
 CHK'D BY: L. BUSH  
 DRILLING CONTR: NORTH AMERICAN DRILLING

# SOIL BOREHOLE LOG



**SITE NAME AND LOCATION**

MINER FLAT DAM  
SANDSTONE RIDGE

**DRILLING METHOD:** BECKER HAMMER

**BORING NO.**

MF-B32

**SAMPLING METHOD:** BULK

**SHEET**

2 OF 4

**DRILLING**

**START FINISH**

**TIME TIME**  
14:20 16:00

**DATE DATE**

12/13/95 12/13/95

**NORTHING:  
EASTING:**

**ELEVATION**

**DRILL RIG** AP1000 BECKER HAMMER

**SURFACE CONDITIONS**

**ANGLE** VERTICAL **BEARING** NA

**SAMPLE HAMMER**

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS
16						•	(6.0-29.0 ft.) Moderate brown (5YR 4/4), compact to dense, SAND, fine to medium grain, <5% clay, <5% to 20% fine subangular gravel, <5% sandstone cobbles to 4", dry (SP)					
17						•						
18						•						
19						•						
20						•						
21						•						
22						•						
23						•						
24						•						
25						•						
26						•						
27						•						
28						•						
29						•	(-29.0) 29.0  (29.0-47.5 ft.) Moderate reddish orange (10R 6/6) to moderate orange pink (10R 7/4), compact to very dense, interbedded CLAYEY SAND and SAND, fine to medium grain, <5% to 20% fine to coarse gravel, dry, (SC and SP)					

DRILLING CONTR \_\_\_\_\_ NORTH AMERICAN DRILLING  
 LOGGED BY \_\_\_\_\_ L. BUSH/C. ROBINSON  
 DATE \_\_\_\_\_ FEBRUARY 1996 \_\_\_\_\_ CHK'D BY \_\_\_\_\_ L. BUSH  
 JOB NO.: \_\_\_\_\_ 943-27691170  
 FILENAME: \_\_\_\_\_ MFB32

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION

MINER FLAT DAM  
SANDSTONE RIDGE

DRILLING METHOD: BECKER HAMMER

BORING NO.

MF-B32

SAMPLING METHOD: BULK

SHEET

3 OF 4

DRILLING

START TIME

14:20

FINISH TIME

16:00

DATE

12/13/95

DATE

12/13/95

NORTHING:  
EASTING:

ELEVATION

DRILL RIG AP1000 BECKER HAMMER

SURFACE CONDITIONS

ANGLE VERTICAL BEARING NA

SAMPLE HAMMER

DEPTH IN FEET  
(ELEVATION)

SPT (N<sub>60</sub>) EQUIVALENT  
BLOWCOUNT FROM  
BECKER HAMMER  
BLOW COUNTS

10 20 30 40 50

SYMBOL

DESCRIPTION OF MATERIAL

TEST RESULTS

WATER  
CONTENT %

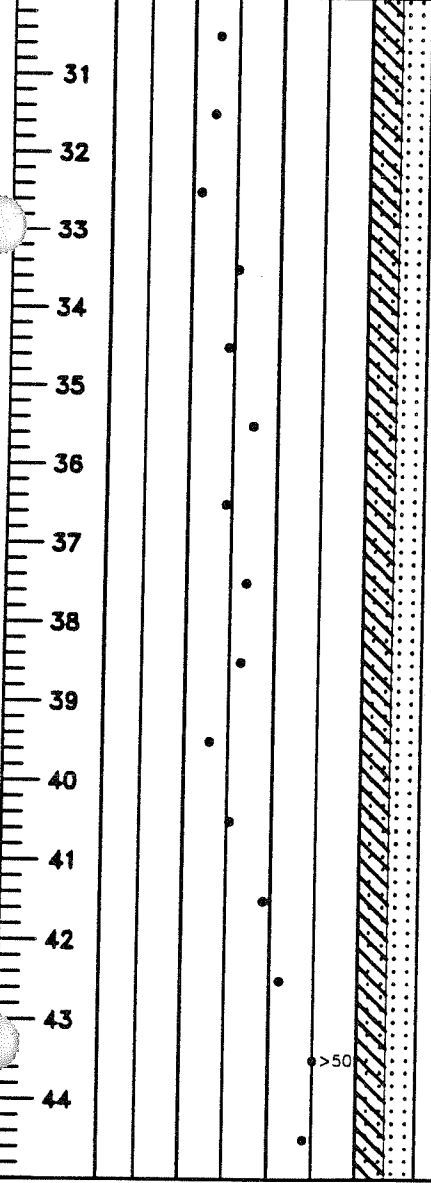
LIQUID LIMIT %

PLASTICITY  
INDEX %

LESS THAN  
NO. 200

OTHER  
TESTS

(29.0-47.5 ft.) Moderate reddish orange  
(10R 6/6) to moderate orange pink  
(10R 7/4), compact to very dense,  
interbedded CLAYEY SAND and SAND,  
fine to medium grain, <5% to 20%  
fine to coarse gravel, dry, (SC and SP)



DRILLING CONTR \_\_\_\_\_ NORTH AMERICAN DRILLING

LOGGED BY \_\_\_\_\_ L. BUSH/C. ROBINSON  
DATE \_\_\_\_\_ FEBRUARY 1996 CHK'D BY \_\_\_\_\_ L. BUSH

JOB NO.: 943-27691170  
FILENAME: MFB32

# SOIL BOREHOLE LOG



SITE NAME AND LOCATION  MINER FLAT DAM SANDSTONE RIDGE	DRILLING METHOD: BECKER HAMMER	BORING NO. MF-B32
	SAMPLING METHOD: BULK	SHEET 4 OF 4
	DRILLING	
	START TIME 14:20	FINISH TIME 16:00
	DATE 12/13/95	DATE 12/13/95

DRILL RIG AP1000 BECKER HAMMER	SURFACE CONDITIONS
ANGLE VERTICAL BEARING NA	
SAMPLE HAMMER	

DEPTH IN FEET (ELEVATION)	SPT (N <sub>60</sub> ) EQUIVALENT BLOWCOUNT FROM BECKER HAMMER BLOW COUNTS					SYMBOL	DESCRIPTION OF MATERIAL	TEST RESULTS				
	10	20	30	40	50			WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	LESS THAN NO. 200	OTHER TESTS

46					
47			(29.0-47.5 ft.) Moderate reddish orange (10R 6/6) to moderate orange pink (10R 7/4), interbedded CLAYEY SAND and SAND, fine to medium grain, <5% to 20% fine to coarse gravel, dry, (SC and SP)		
48		>50	(47.5-49.0 ft.) BEDROCK, Sandstone	(-47.5)	47.5
49		>50	End of Borehole at 49.0 ft.	(-49.0)	49.0
50					
51					
52					
53					
54					
55					
56					
57					
58					
59					

DRILLING CONTR \_\_\_\_\_ NORTH AMERICAN DRILLING  
 LOGGED BY \_\_\_\_\_ L. BUSH/C. ROBINSON  
 DATE \_\_\_\_\_ FEBRUARY 1996 \_\_\_\_\_ CHK'D BY \_\_\_\_\_ L. BUSH  
 JOB NO.: \_\_\_\_\_ 943-27691170 \_\_\_\_\_  
 FILENAME: \_\_\_\_\_ MFB32 \_\_\_\_\_

**APPENDIX B**

**LABORATORY TESTS RESULTS**

TABLE 1  
SUMMARY OF SOIL DATA

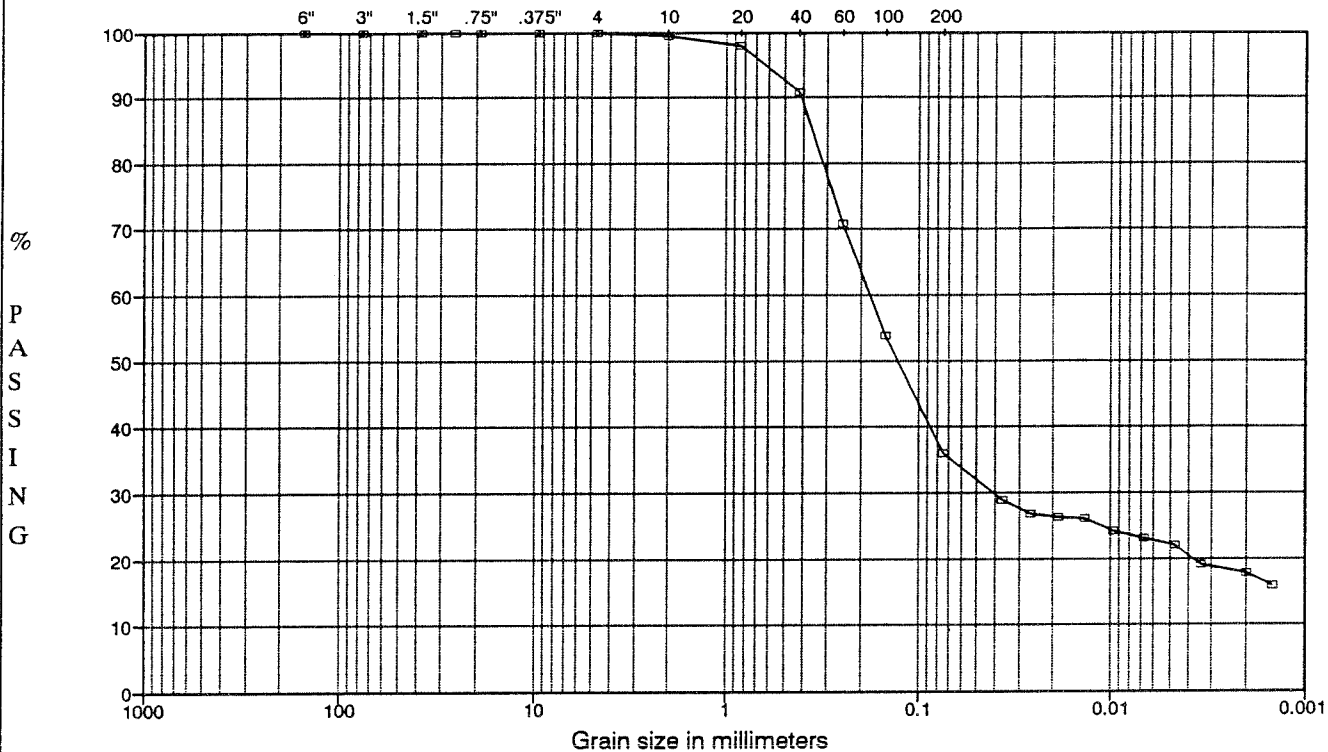
Project Name MMCSSA/MINER FLAT/AZ ARIZONA  
Location  
Project Number 943-27691.170

BORING NO.	SAMPLE NO.	SAMPLE DEPTH (ft)	USCS SOIL CLASSIFICATION	NATURAL MOISTURE %	ATTERBERG LIMITS			GRAIN SIZE DISTRIBUTION		SPECIFIC GRAVITY	MOISTURE RELATIONSHIP		ADDITIONAL TESTS COMMENTS (SEE NOTES)
					L.L.	P.L.	P.I.	% FINER NO. 4 SIEVE	% FINER NO. 200 SIEVE		STANDARD PROCTOR PC(DRY)	MOISTURE(%)	
BA-1	--	35-40'	--	--	--	--	--	100	36	2.57	127.0	9.5	--
BA-2	--	35-40'	SC	--	27	12	15	100	50	2.57	124.5	11.0	--
BA-2	--	40-45'	--	--	30	13	17	--	--	--	--	--	--
BA-3	--	30-35'	CL	--	29	14	15	100	51	2.60	--	--	--
BA-3	--	35-40'	CL	--	34	16	18	100	62	2.64	--	--	--
BA-4	--	30-35'	CL	--	40	17	23	100	67	2.58	115.0	15.0	--
BA-4	--	35-40'	CL	--	35	19	16	100	71	2.65	--	--	--
BA-5	--	30-35'	CL	--	34	17	17	100	65	2.63	113.0	15.5	--
BA-5	--	35-39'	CL	--	38	18	20	100	70	2.63	--	--	--
BA-6	--	25-30'	CL	--	37	15	22	100	57	2.66	--	--	--
BA-6	--	30-35'	CL	--	34	16	18	100	54	2.64	--	--	--
--	--	--	--	--	--	--	--	--	--	--	--	--	--
--	--	--	--	--	--	--	--	--	--	--	--	--	--
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NOTES:  
LL = LIQUID LIMIT  
PL = PLASTIC LIMIT  
PI = PLASTIC INDEX  
SL = SHRINKAGE LIMIT

T = TRIAXIAL TEST  
U = UNCONFINED COMPRESSION TEST  
C = CONSOLIDATION TEST  
P = PROCTOR TEST  
DS = DIRECT SHEAR TEST  
Perm = PERMEABILITY

**PARTICLE SIZE DISTRIBUTION ASTM D421 AND D422  
US STANDARD SIEVE OPENING SIZES**



USCS

COBBLES	Coarse	Fine	Cor	Med	Fine	FINES (Silt and Clay)
	GRAVEL		SAND			

SAMPLE ID	W%	LL	PL	PI	Gs	DESCRIPTION
BOREHOLE SAMPLE NO. DEPTH	BA-1 -- 35-40'	--	--	--	2.57	Coarse to fine SAND and FINES

Sample Type:                      Date Tested: 10/19/95                      USCS: ---

TECH:                      RCS  
DATE:                      10/23/95  
CHECKED:                      ASF  
REVIEWED:

MMCSSA/MINER FLAT/AZ  
943-27691.170

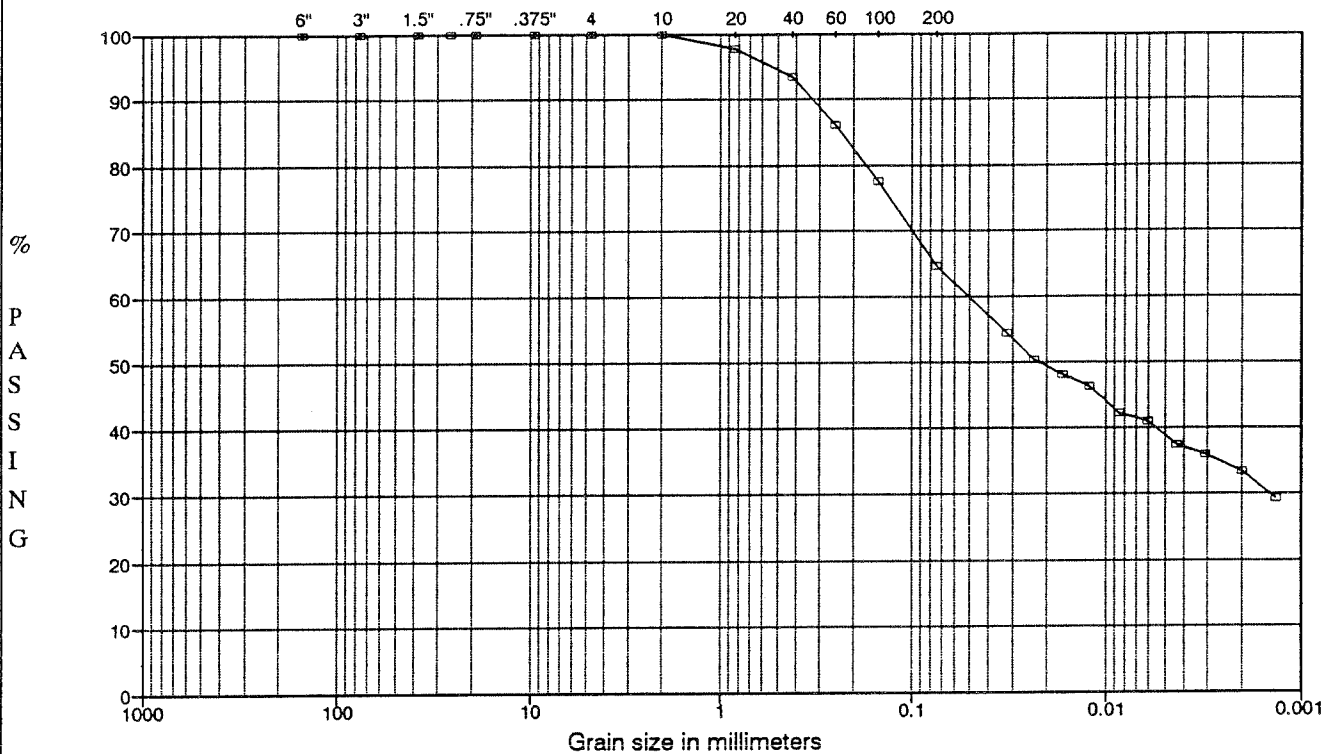
FILENAME: 2769140

GOLDER ASSOCIATES INC.  
LAKEWOOD, COLORADO



**PARTICLE SIZE DISTRIBUTION ASTM D421 AND D422**

**US STANDARD SIEVE OPENING SIZES**



USCS

<b>COBBLES</b>	Coarse	Fine	Cor	Med	Fine	<b>FINES (Silt and Clay)</b>
	<b>GRAVEL</b>		<b>SAND</b>			

SAMPLE ID	W%	LL	PL	PI	Gs	DESCRIPTION
BOREHOLE SAMPLE NO. DEPTH	BA-5 -- 30-35'	34	17	17	2.63	Silty CLAY and medium to fine SAND

Sample Type:                      Date Tested: 10/18/95                      USCS: CL

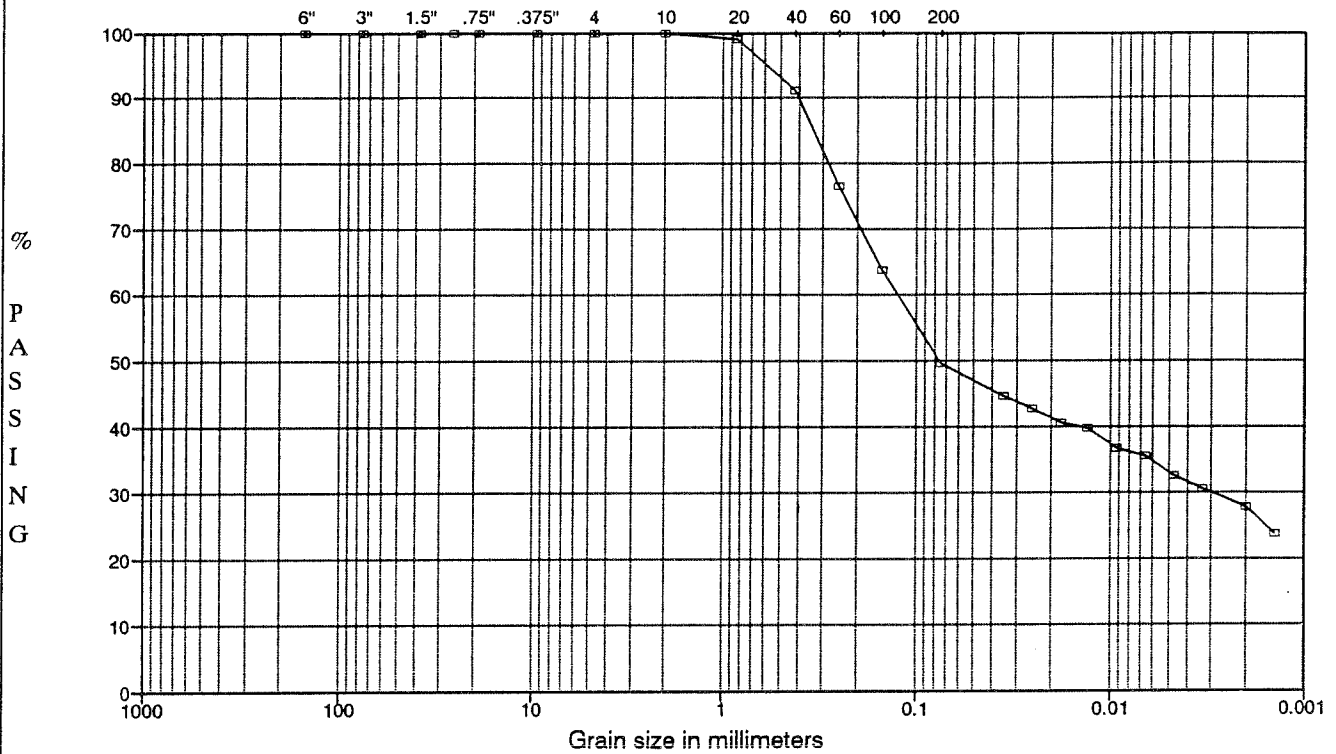
TECH:                      RCS  
DATE:                      10/23/95  
CHECKED:                  ASF  
REVIEWED:

MMCSSA/MINER FLAT/AZ  
943-27691.170

FILENAME: 276935

GOLDER ASSOCIATES INC.  
LAKEWOOD, COLORADO

**PARTICLE SIZE DISTRIBUTION ASTM D421 AND D422  
US STANDARD SIEVE OPENING SIZES**



USCS

<b>COBBLES</b>	Coarse	Fine	Cor	Med	Fine	<b>FINES (Silt and Clay)</b>
	<b>GRAVEL</b>		<b>SAND</b>			

SAMPLE ID	W%	LL	PL	PI	Gs	DESCRIPTION
BOREHOLE BA-2	--	27	12	15	2.57	Medium to fine SAND and silty CLAY
SAMPLE NO.						
DEPTH 35-40'						
Sample Type:		Date Tested: 10/19/95			USCS: SC	

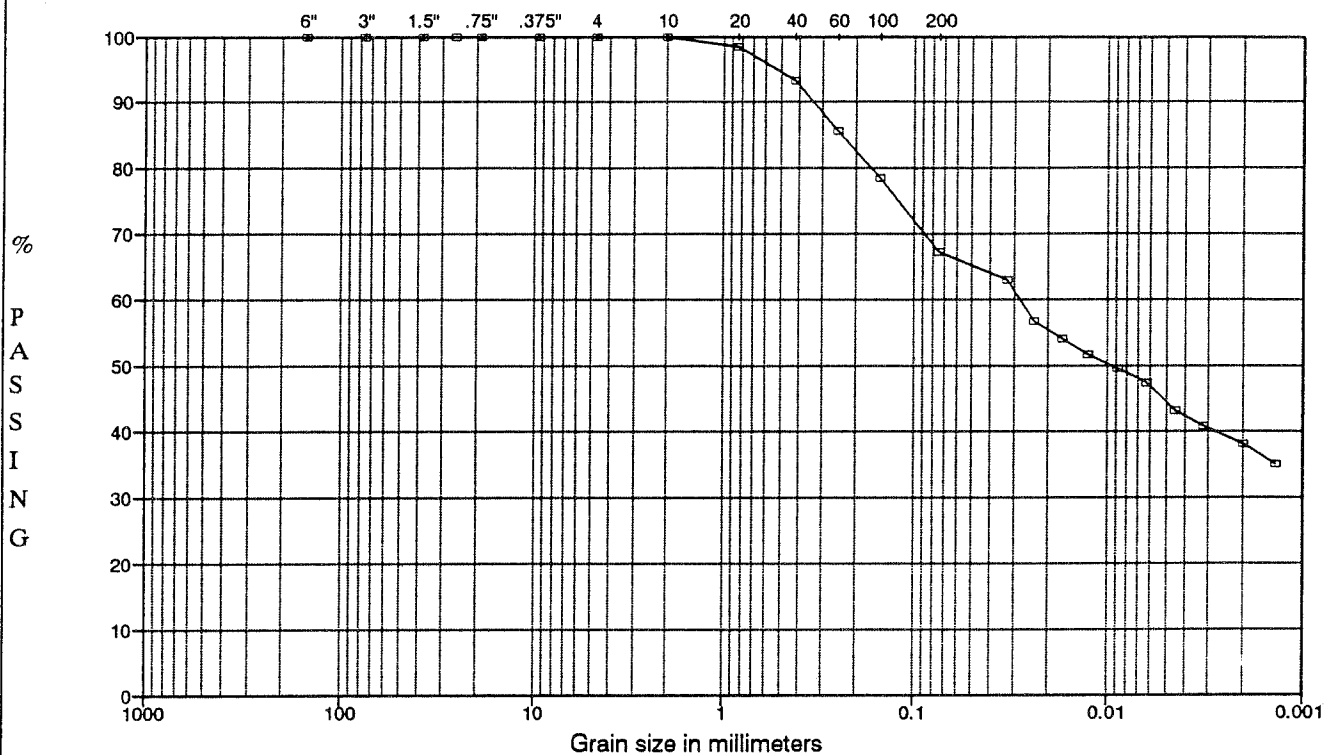
TECH: BRB  
 DATE: 10/23/95  
 CHECKED: ASF  
 REVIEWED:

MMCSSA/MINER FLAT/AZ  
 943-27691.170

GOLDER ASSOCIATES INC.  
 LAKEWOOD, COLORADO

FILENAME: 2769240

**PARTICLE SIZE DISTRIBUTION ASTM D421 AND D422  
US STANDARD SIEVE OPENING SIZES**



USCS

<b>COBBLES</b>	Coarse	Fine	Cor	Med	Fine	<b>FINES (Silt and Clay)</b>
	<b>GRAVEL</b>		<b>SAND</b>			

SAMPLE ID	W%	LL	PL	PI	Gs	DESCRIPTION
BOREHOLE SAMPLE NO. DEPTH	BA-4 -- 30-35'	40	17	23	2.58	Silty CLAY and medium to fine SAND
Sample Type:		Date Tested: 10/19/95			USCS: CL	

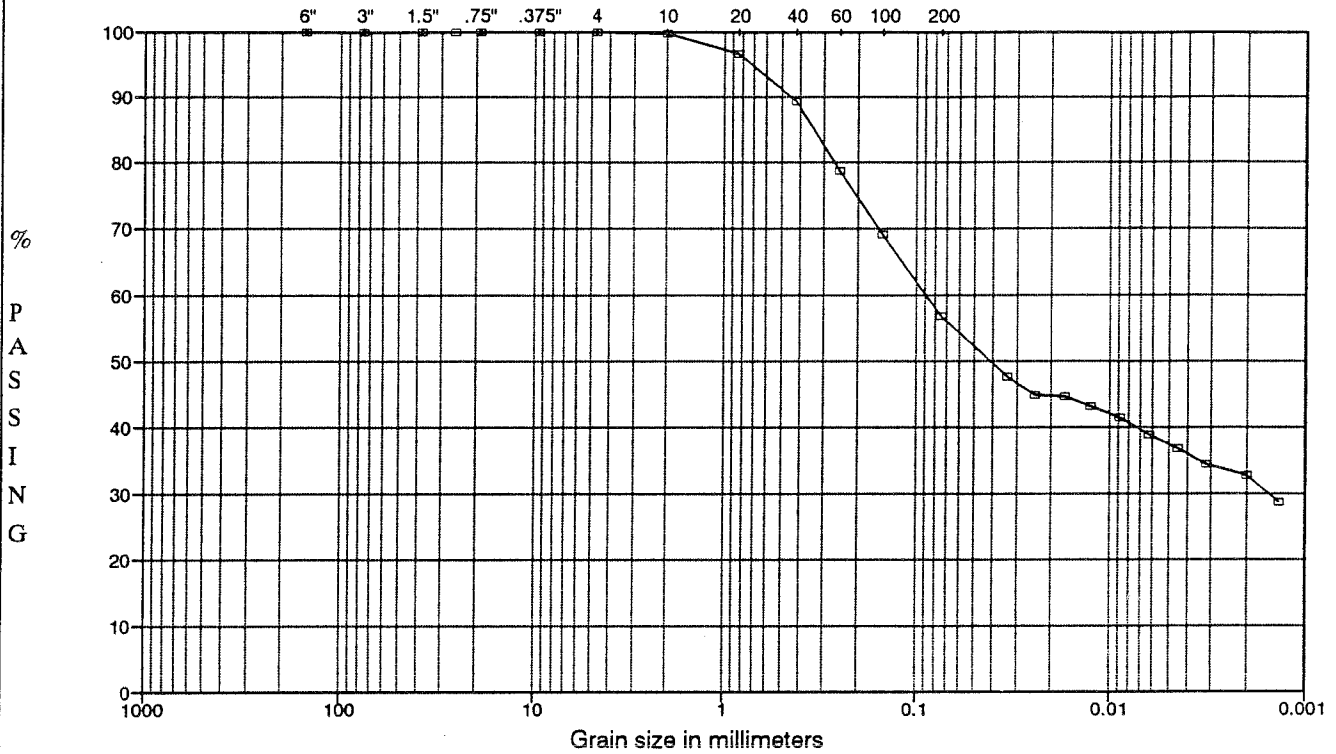
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DATE: 10/23/95  
CHECKED: ASF  
REVIEWED:

MMCSSA/MINER FLAT/AZ  
943-27691.170

FILENAME: 2769135

GOLDER ASSOCIATES INC.  
LAKEWOOD, COLORADO

**PARTICLE SIZE DISTRIBUTION ASTM D421 AND D422  
US STANDARD SIEVE OPENING SIZES**



USCS

<b>COBBLES</b>	Coarse	Fine	Cor	Med	Fine	<b>FINES (Silt and Clay)</b>
	<b>GRAVEL</b>		<b>SAND</b>			

SAMPLE ID	W%	LL	PL	PI	Gs	DESCRIPTION
BOREHOLE SAMPLE NO. DEPTH	BA-6 -- 25-30'	37	15	22	2.66	Silty CLAY and medium to fine SAND
Sample Type:		Date Tested: 10/18/95			USCS: CL	

TECH: RCS  
DATE: 10/23/95  
CHECKED: ASF  
REVIEWED:

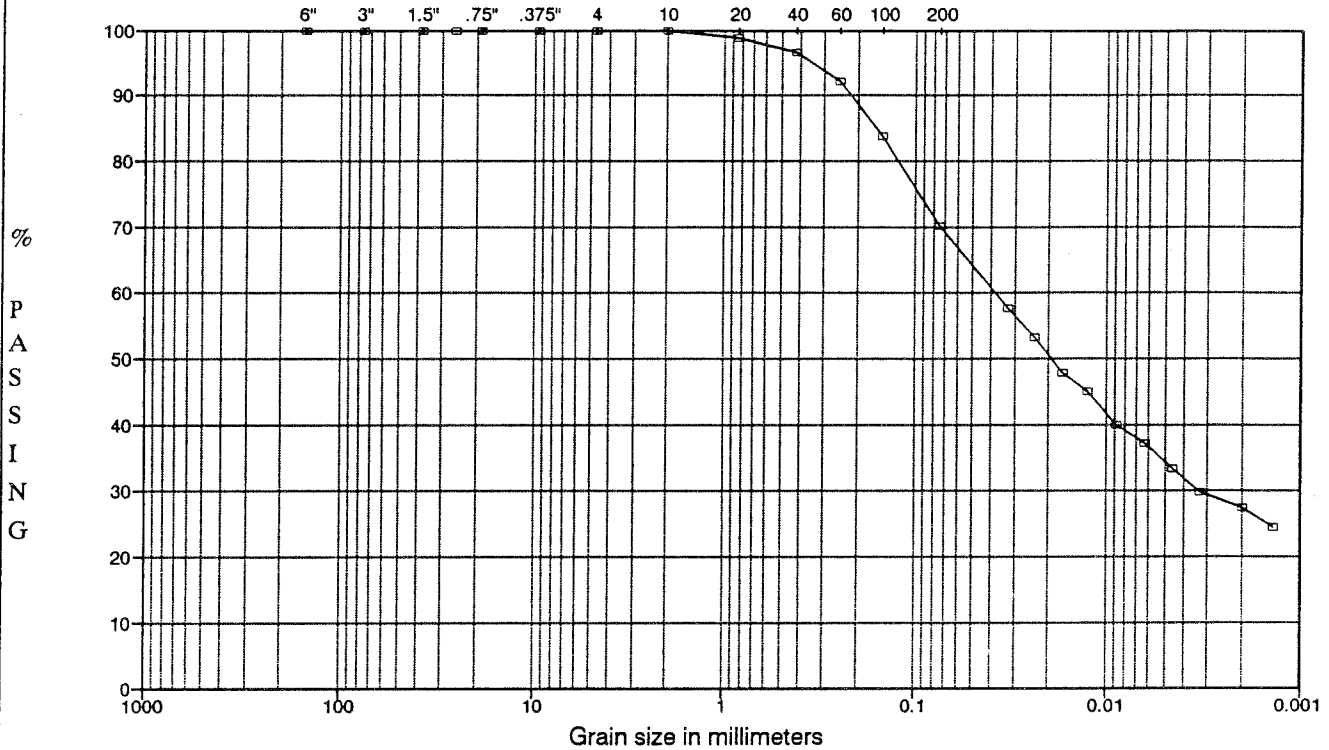
MMCSSA/MINER FLAT/AZ  
943-27691.170

FILENAME: 276930

GOLDER ASSOCIATES INC.  
LAKEWOOD, COLORADO

**PARTICLE SIZE DISTRIBUTION ASTM D421 AND D422**

US STANDARD SIEVE OPENING SIZES



USCS

<b>COBBLES</b>	Coarse	Fine	Cor	Med	Fine	<b>FINES (Silt and Clay)</b>
	<b>GRAVEL</b>		<b>SAND</b>			

SAMPLE ID	W%	LL	PL	PI	Gs	DESCRIPTION
BOREHOLE	BA-5	--	38	18	20	Silty CLAY with some medium to fine sand
SAMPLE NO.						
DEPTH	35-39'					
Sample Type:		Date Tested: 10/18/95			USCS: CL	

TECH: RCS  
 DATE: 10/23/95  
 CHECKED: ASF  
 REVIEWED:

MMCSSA/MINER FLAT/AZ

943-27691.170

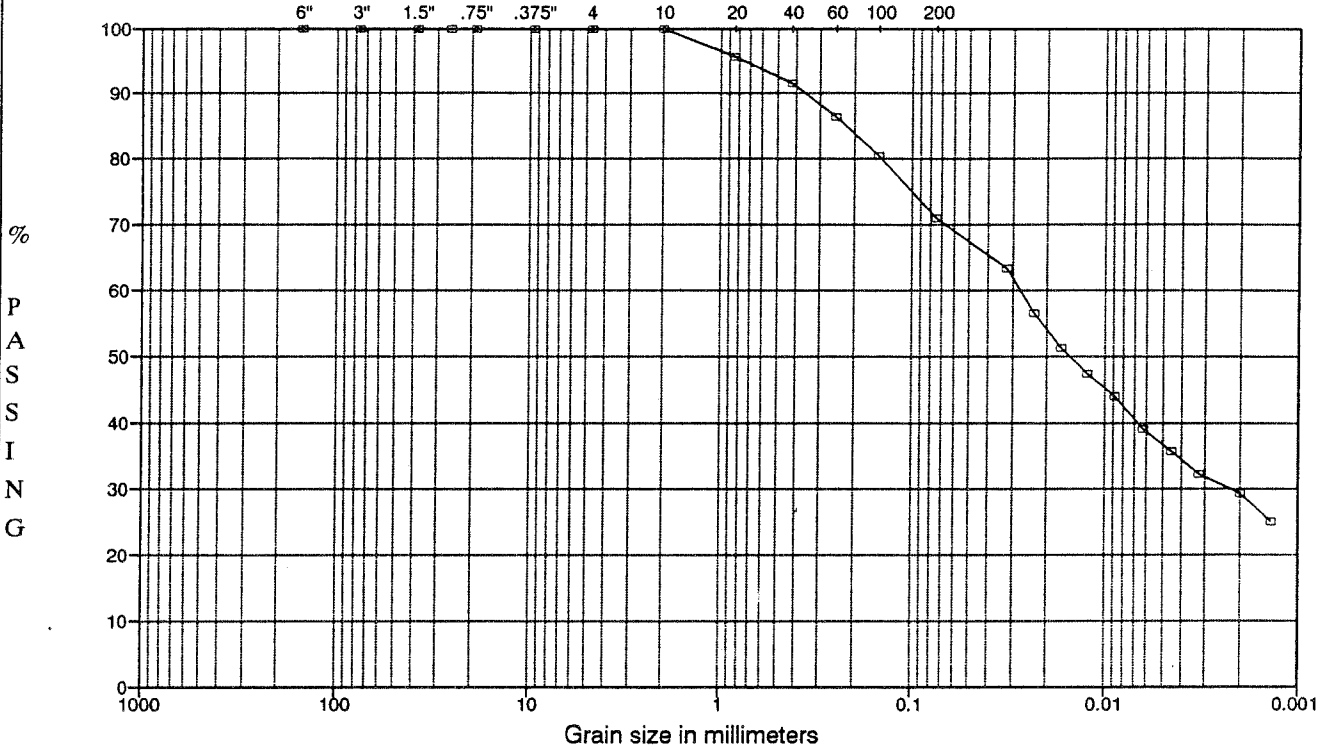
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GOLDER ASSOCIATES INC.  
 LAKEWOOD, COLORADO



**PARTICLE SIZE DISTRIBUTION ASTM D421 AND D422**

**US STANDARD SIEVE OPENING SIZES**



USCS

<b>COBBLES</b>	Coarse	Fine	Cor	Med	Fine	<b>FINES (Silt and Clay)</b>
	<b>GRAVEL</b>		<b>SAND</b>			

SAMPLE ID	W%	LL	PL	PI	Gs	DESCRIPTION
BOREHOLE SAMPLE NO. DEPTH	BA-4 -- 35-40'	35	19	16	2.65	Silty CLAY with some medium to fine sand
Sample Type:		Date Tested: 10/19/95			USCS: CL	

TECH: BRB  
 DATE: 10/23/95  
 CHECKED: ASF  
 REVIEWED:

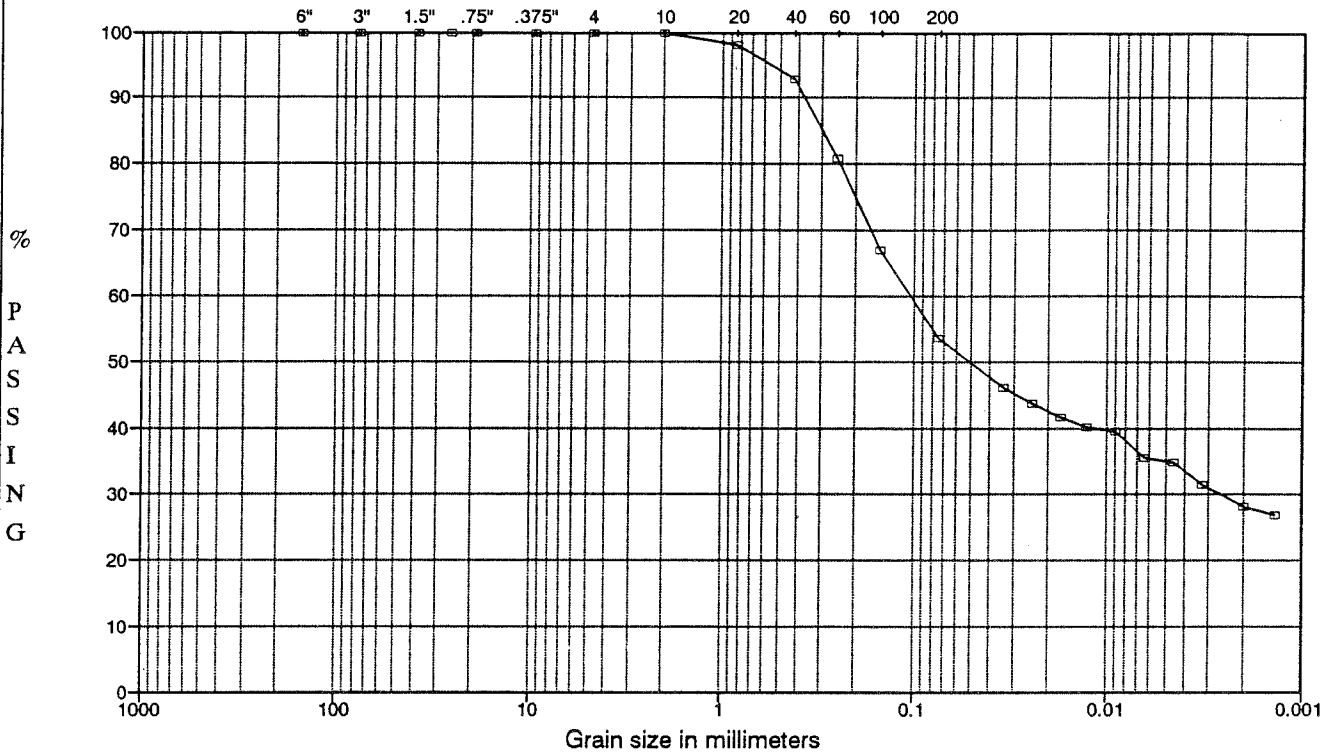
MMCSSA/MINER FLAT/AZ  
 943-27691.170

FILENAME: 27691.170

GOLDER ASSOCIATES INC.  
 LAKEWOOD, COLORADO

# PARTICLE SIZE DISTRIBUTION ASTM D421 AND D422

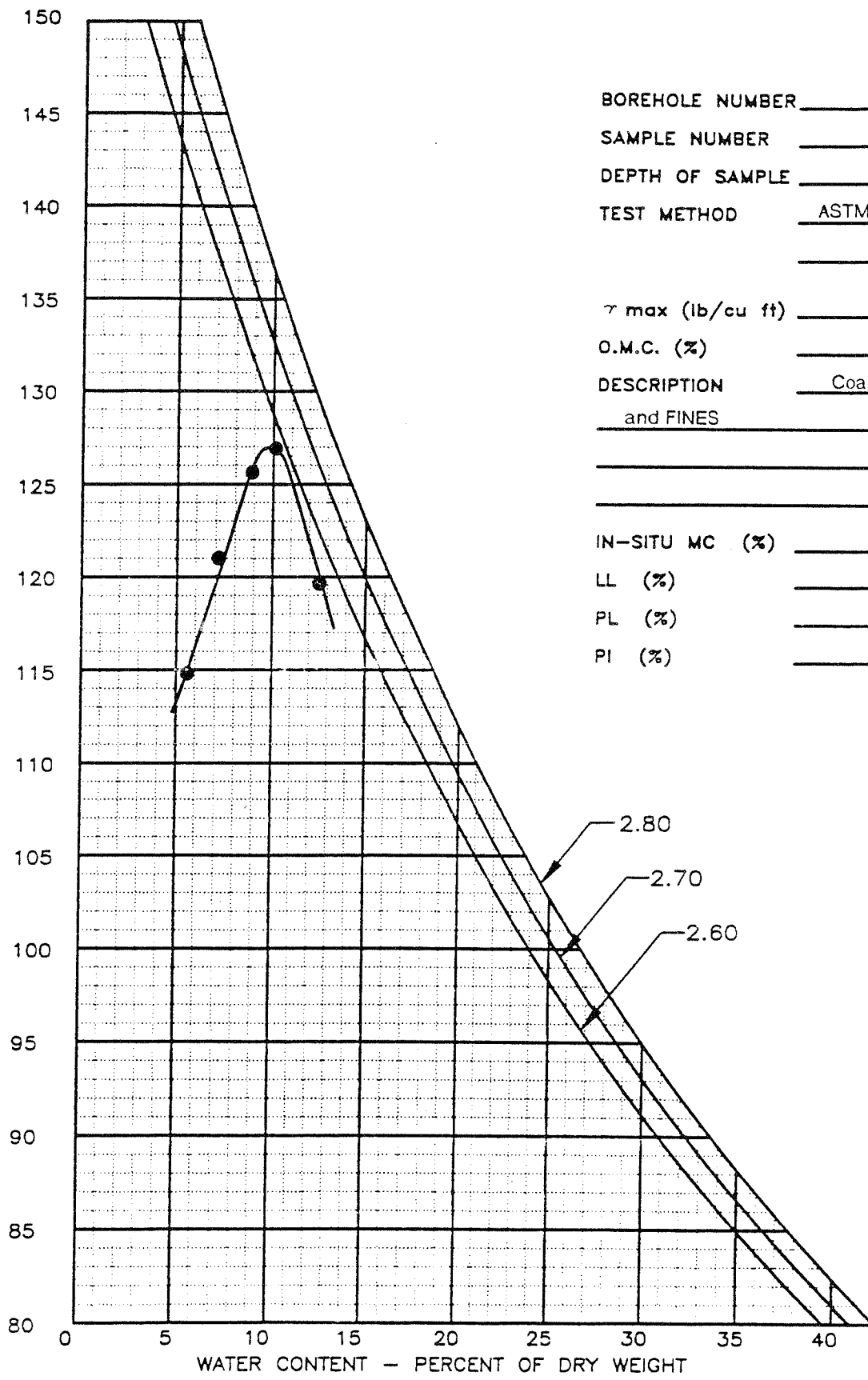
## US STANDARD SIEVE OPENING SIZES







DRY UNIT WEIGHT - POUNDS PER CUBIC FOOT



BOREHOLE NUMBER BA-1  
 SAMPLE NUMBER \_\_\_\_\_  
 DEPTH OF SAMPLE 35-40'  
 TEST METHOD ASTM D698 METHOD A  
 \_\_\_\_\_  
 \_\_\_\_\_  
 $\gamma$  max (lb/cu ft) 127.0  
 O.M.C. (%) 9.5  
 DESCRIPTION Coarse to fine SAND  
and FINES  
 \_\_\_\_\_  
 \_\_\_\_\_  
 IN-SITU MC (%) \_\_\_\_\_  
 LL (%) \_\_\_\_\_  
 PL (%) \_\_\_\_\_  
 PI (%) \_\_\_\_\_

PROCTOR CURVE  
 SAMPLE #BA-1

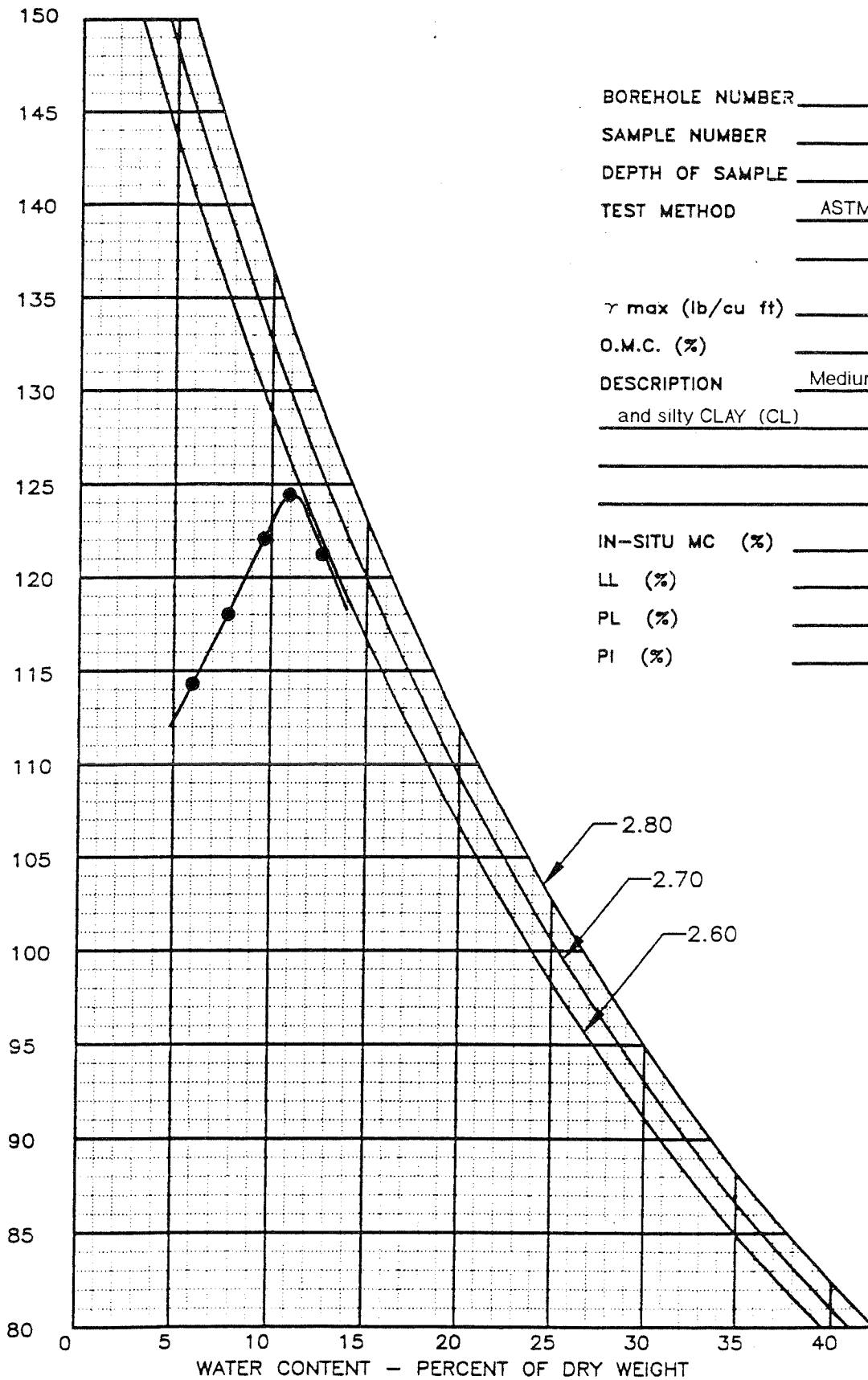


Denver, Colorado

CLIENT/PROJECT  
 MMCSSA/MINER FLAT/AZ

TITLE	PROCTOR CURVE SAMPLE #BA-1		
DRAWN	ASF	DATE	OCTOBER 1995
CHECKED	KMR	SCALE	AS SHOWN
REVIEWED		FILE NO.	WATER.DWG
		JOB NO.	943-27691
		DWG NO./REV. NO.	
		FIGURE NO.	

DRY UNIT WEIGHT - POUNDS PER CUBIC FOOT



BOREHOLE NUMBER BA-2  
 SAMPLE NUMBER \_\_\_\_\_  
 DEPTH OF SAMPLE 35-40'  
 TEST METHOD ASTM D698 METHOD A

$\gamma$  max (lb/cu ft) 124.5  
 O.M.C. (%) 11.0  
 DESCRIPTION Medium to fine SAND  
and silty CLAY (CL)

IN-SITU MC (%) \_\_\_\_\_  
 LL (%) 27  
 PL (%) 12  
 PI (%) 15

2.80  
 2.70  
 2.60



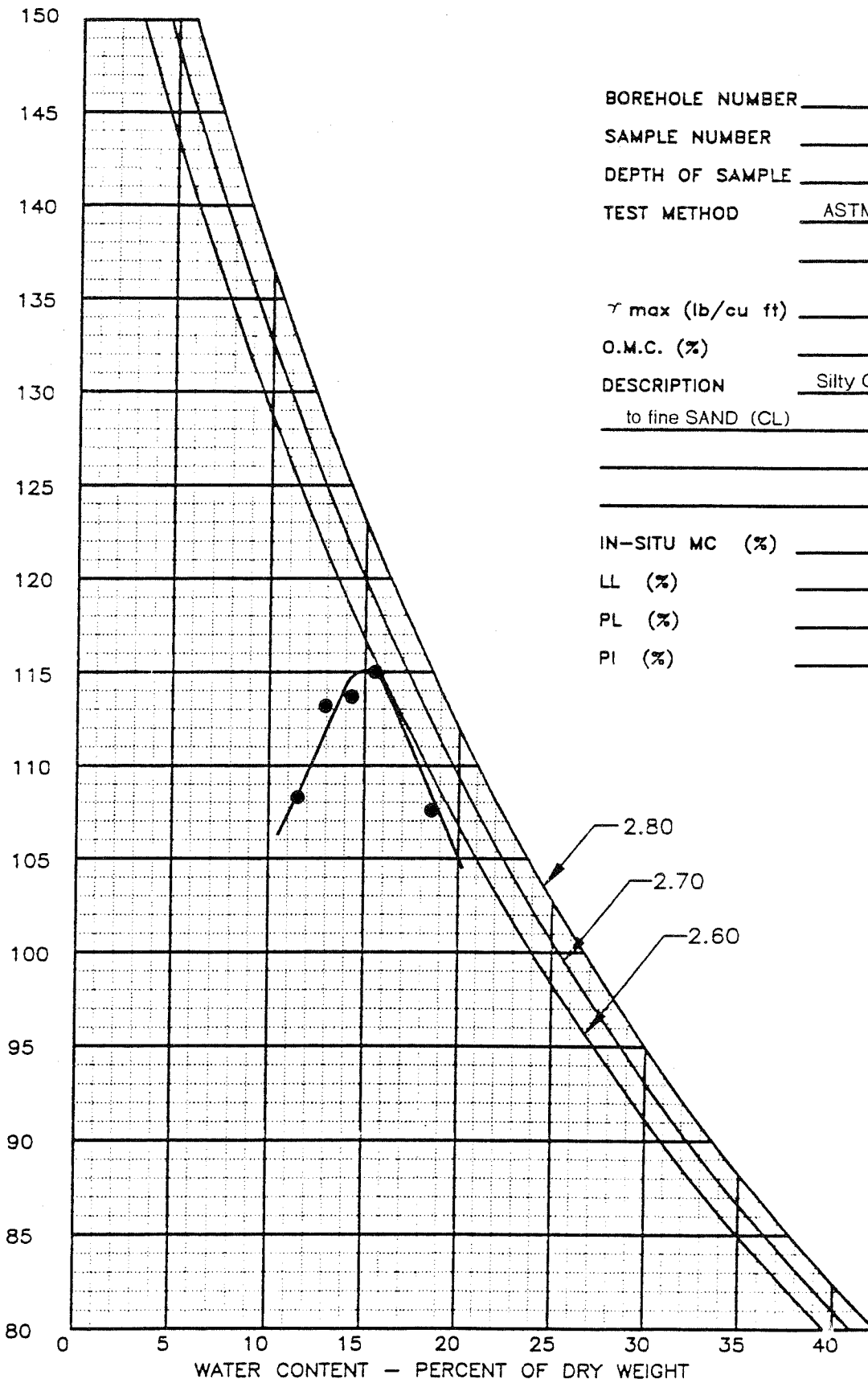
Denver, Colorado

TITLE  
**PROCTOR CURVE**  
**SAMPLE #BA-2**

CLIENT/PROJECT  
**MMCSSA/MINER FLAT/AZ**

DRAWN	ASF	DATE	OCTOBER 1995	JOB NO.	943-27691
CHECKED	KMR	SCALE	AS SHOWN	OWG NO./REV. NO.	
REVIEWED		FILE NO.	WATER.DWG	FIGURE NO.	

DRY UNIT WEIGHT - POUNDS PER CUBIC FOOT



BOREHOLE NUMBER BA-4  
 SAMPLE NUMBER \_\_\_\_\_  
 DEPTH OF SAMPLE 30-35'  
 TEST METHOD ASTM D698 METHOD A  
 \_\_\_\_\_  
 \_\_\_\_\_  
 $\tau$  max (lb/cu ft) 115.0  
 O.M.C. (%) 15.0  
 DESCRIPTION Silty CLAY and medium  
to fine SAND (CL)  
 \_\_\_\_\_  
 \_\_\_\_\_  
 IN-SITU MC (%) \_\_\_\_\_  
 LL (%) 40  
 PL (%) 17  
 PI (%) 23



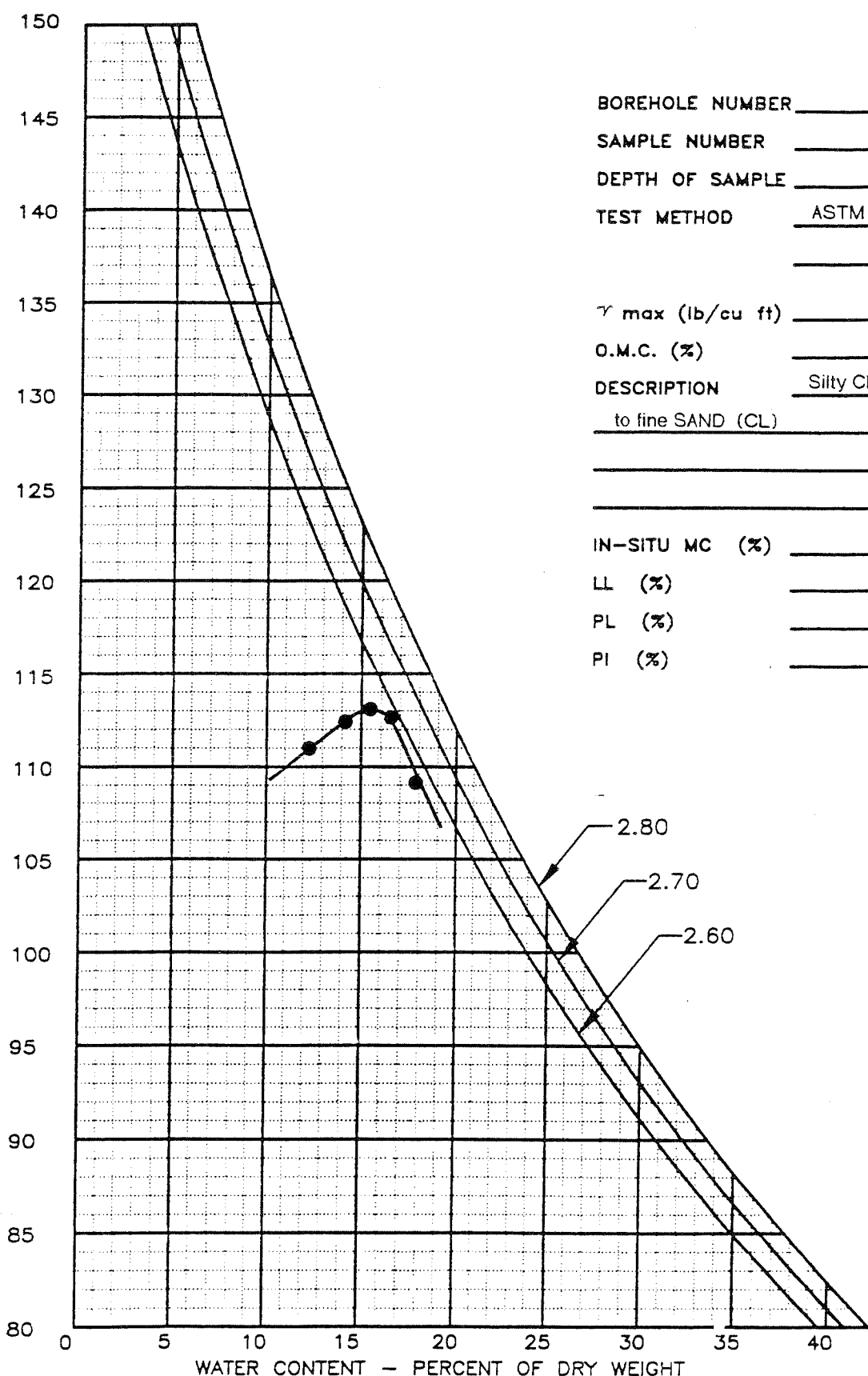
Denver, Colorado

TITLE  
**PROCTOR CURVE**  
**SAMPLE #BA-4**

CLIENT/PROJECT  
**MMCSSA/MINER FLAT/AZ**

DRAWN	ASF	DATE	OCTOBER 1995	JOB NO.	943-27691
CHECKED	KMR	SCALE	AS SHOWN	DWG NO./REV. NO.	
REVIEWED		FILE NO.	WATER.DWG	FIGURE NO.	

DRY UNIT WEIGHT - POUNDS PER CUBIC FOOT



BOREHOLE NUMBER BA-5  
 SAMPLE NUMBER \_\_\_\_\_  
 DEPTH OF SAMPLE 30-35'  
 TEST METHOD ASTM D698 METHOD A  
 \_\_\_\_\_  
 \_\_\_\_\_  
 $\gamma$  max (lb/cu ft) 113.0  
 O.M.C. (%) 15.5  
 DESCRIPTION Silty CLAY and medium  
to fine SAND (CL)  
 \_\_\_\_\_  
 \_\_\_\_\_  
 IN-SITU MC (%) \_\_\_\_\_  
 LL (%) 34  
 PL (%) 17  
 PI (%) 17



Denver, Colorado

TITLE  
**PROCTOR CURVE**  
**SAMPLE #BA-5**

CLIENT/PROJECT  
**MMCSSA/MINER FLAT/AZ**

DRAWN	ASF	DATE	OCTOBER 1995	JOB NO.	943-27691
CHECKED	KMR	SCALE	AS SHOWN	DWG NO./REV. NO.	
REVIEWED		FILE NO.	WATER.DWG	FIGURE NO.	

TABLE 2  
 SUMMARY OF FLEXIBLE WALL PERMEABILITY TEST RESULTS

SAMPLE NUMBER	BA-5 30-35							
Sample Length (cm)	9.60							
Sample Diameter (cm)	7.26							
Sample Dry Density (pcf)	108.2							
Maximum Dry Density (pcf)	--							
Compaction (%)	--							
Initial Moisture Content (%)	9.2							
Optimum Moisture Content (%)	--							
Effective Stress (psi)	5							
Back Pressure (psi)	95							
Gradient	13							
Average Permeability (cm/sec)	2.7X10 <sup>-7</sup>							

**APPENDIX C**

**DOWNHOLE SURVEYS**

**BOREHOLE: MF-218A**  
**COLLAR COORDINATES**

**NORTH** 1,085,010.0 \*  
**EAST** 576,800.0 \*  
**ELEVATION** 6,007.0 \* (Estimated)

**Miner Flat Dam**

**Main Access Road**

**BOREHOLE - AS DRILLED**

Surveyed By: DA

Date Surveyed: August 29, 1995

Time Surveyed: 10:03 am

Raw Data From Owl Technical Assoc.

HOLE DEPTH (ft.)	Model 780 INCLINATION (from Vert.)	DIRECTION North=360	TRUE DEPTH (ft.)	DISTANCE NORTH (ft.)	DISTANCE EAST (ft.)	CLOSURE (ft.)	NORTHING (ft.)	EASTING (ft.)	ELEVATION (ft.)
0.0	0.0	0	0.00	0.00	0.00	0.00	1,085,010.0	576,800.0	6,007.0
10.0	2.4	11	10.00	0.21	0.04	0.21	1,085,010.2	576,800.0	5,997.0
20.0	1.6	0	19.99	0.55	0.08	0.56	1,085,010.6	576,800.1	5,987.0
30.0	3.0	18	29.98	0.94	0.16	0.95	1,085,010.9	576,800.2	5,977.0
40.0	2.8	2	39.97	1.43	0.25	1.45	1,085,011.4	576,800.3	5,967.0
50.0	2.8	0	49.96	1.92	0.26	1.94	1,085,011.9	576,800.3	5,957.0
60.0	2.7	356	59.94	2.40	0.24	2.41	1,085,012.4	576,800.2	5,947.1
70.0	2.4	351	69.93	2.84	0.19	2.85	1,085,012.8	576,800.2	5,937.1
80.0	2.0	358	79.93	3.22	0.15	3.23	1,085,013.2	576,800.2	5,927.1
90.0	2.7	10	89.92	3.63	0.19	3.63	1,085,013.6	576,800.2	5,917.1
100.0	2.5	351	99.91	4.08	0.20	4.08	1,085,014.1	576,800.2	5,907.1
110.0	2.7	15	109.90	4.52	0.22	4.52	1,085,014.5	576,800.2	5,897.1
120.0	2.8	354	119.89	4.99	0.26	5.00	1,085,015.0	576,800.3	5,887.1
130.0	2.4	343	129.88	5.43	0.17	5.44	1,085,015.4	576,800.2	5,877.1
140.0	1.8	347	139.87	5.79	0.07	5.79	1,085,015.8	576,800.1	5,867.1
150.0	2.8	355	149.86	6.17	0.02	6.17	1,085,016.2	576,800.0	5,857.1
160.0	1.8	11	159.85	6.55	0.03	6.55	1,085,016.5	576,800.0	5,847.1
170.0	1.8	351	169.85	6.85	0.04	6.85	1,085,016.9	576,800.0	5,837.2
180.0	2.0	10	179.84	7.18	0.04	7.18	1,085,017.2	576,800.0	5,827.2
190.0	1.7	349	189.84	7.50	0.04	7.50	1,085,017.5	576,800.0	5,817.2
200.0	2.0	2	199.83	7.82	0.02	7.82	1,085,017.8	576,800.0	5,807.2
210.0	1.6	343	209.83	8.13	-0.01	8.13	1,085,018.1	576,800.0	5,797.2
220.0	1.5	347	219.82	8.39	-0.08	8.39	1,085,018.4	576,799.9	5,787.2
230.0	1.8	323	229.82	8.64	-0.21	8.64	1,085,018.6	576,799.8	5,777.2
240.0	2.0	322	239.81	8.90	-0.41	8.91	1,085,018.9	576,799.6	5,767.2
250.0	2.0	317	249.81	9.17	-0.64	9.19	1,085,019.2	576,799.4	5,757.2
260.0	2.7	346	259.80	9.52	-0.81	9.56	1,085,019.5	576,799.2	5,747.2
270.0	2.7	353	269.79	9.99	-0.90	10.03	1,085,020.0	576,799.1	5,737.2
280.0	2.5	353	279.78	10.44	-0.95	10.48	1,085,020.4	576,799.0	5,727.2
290.0	2.5	350	289.77	10.87	-1.02	10.92	1,085,020.9	576,799.0	5,717.2
300.0	2.7	345	299.76	11.31	-1.12	11.37	1,085,021.3	576,798.9	5,707.2
310.0	2.7	350	309.75	11.77	-1.22	11.83	1,085,021.8	576,798.8	5,697.3
320.0	1.6	329	319.74	12.12	-1.33	12.20	1,085,022.1	576,798.7	5,687.3
330.0	1.7	327	329.74	12.37	-1.48	12.45	1,085,022.4	576,798.5	5,677.3
340.0	1.7	320	339.73	12.60	-1.66	12.71	1,085,022.6	576,798.3	5,667.3
350.0	2.2	334	349.73	12.89	-1.84	13.02	1,085,022.9	576,798.2	5,657.3
359.9	2.6	336	359.62	13.27	-2.01	13.42	1,085,023.3	576,798.0	5,647.4

**Error Analysis**  
**Bottom Coordinates**

Missed the target of the designed hole (0.0°,0.0°) by:

13.3 ft North  
2.0 ft East  
0.3 ft Higher  
-----  
13.42 feet Total  
3.73% Tolerance

Northing 1,085,023.3 +/- 0.1  
Easting 576,798.0 +/- 0.3  
Elevation 5,647.4 +/- 0.0

**EXPLANATION**

**HOLE DEPTH:** Length along the borehole axis from the collar to the point of the survey, NOT vertical depth.

**INCLINATION:** Equivalent to "Dip" with the exception that 0° INCLINATION = VERTICAL and 90° INCLINATION = HORIZONTAL. INCLINATION greater than 90° implies UP.

**DIRECTION:** Equivalent to "Bearing" of the borehole at the point of survey. Measured on a magnetic azimuth (0°-360°) system.

**DISTANCE NORTH, DISTANCE EAST --** Distances from the collar calculated on a TRUE NORTH-SOUTH grid.

**CLOSURE:** The horizontal distance from the collar of the borehole to the survey point.

**NORTHING, EASTING, ELEV.:** Coordinates calculated from the collar coordinates. Collar coordinates furnished by Geodetic Surveys, Inc.. (\* Estimated coordinates have not been surveyed.)



**BOREHOLE: MF250**  
**COLLAR COORDINATES**

NORTH 1,085,815.0 \*  
EAST 576,875.0 \*  
ELEVATION 8,081.0 \* (Estimated)

**Miner Flat Dam**

**Sandstone Ridge**

**BOREHOLE - AS DRILLED**

Surveyed By: lb/cr

Date Surveyed: June 19, 1995

Time Surveyed: 12:45 pm

Raw Data From Owl Technical Assoc.

HOLE DEPTH (ft.)	Model 780 INCLINATION (from Vert.) * North=360 °	DIRECTION	TRUE DEPTH (ft.)	DISTANCE NORTH (ft.)	DISTANCE EAST (ft.)	CLOSURE (ft.)	NORTHING (ft.)	EASTING (ft.)	ELEVATION (ft.)
0.0	0.0 °	0 °	0.00	0.00	0.00	0.00	1,085,815.0	576,875.0	8,081.0
5.0	0.5 °	108 °	5.00	-0.01	0.02	0.02	1,085,815.0	576,875.0	8,078.0
10.0	1.3 °	316 °	10.00	0.03	0.00	0.03	1,085,815.0	576,875.0	8,071.0
20.0	0.5 °	106 °	20.00	0.10	-0.03	0.10	1,085,815.1	576,875.0	8,061.0
30.0	1.7 °	83 °	30.00	0.10	0.15	0.19	1,085,815.1	576,875.2	8,051.0
40.0	1.2 °	40 °	39.99	0.20	0.37	0.42	1,085,815.2	576,875.4	8,041.0
50.0	1.0 °	45 °	49.99	0.34	0.50	0.60	1,085,815.3	576,875.5	8,031.0
60.0	1.1 °	35 °	59.99	0.48	0.61	0.78	1,085,815.5	576,875.6	8,021.0
70.0	1.0 °	37 °	69.99	0.63	0.72	0.96	1,085,815.6	576,875.7	8,011.0
80.0	0.9 °	29 °	79.99	0.77	0.81	1.12	1,085,815.8	576,875.8	8,001.0
90.0	0.4 °	85 °	89.98	0.84	0.89	1.22	1,085,815.8	576,875.9	5,991.0
100.0	1.7 °	99 °	99.98	0.82	1.07	1.35	1,085,815.8	576,876.1	5,981.0
110.0	1.2 °	47 °	109.98	0.87	1.29	1.56	1,085,815.9	576,876.3	5,971.0
120.0	1.8 °	79 °	119.98	0.97	1.52	1.80	1,085,816.0	576,876.5	5,961.0
130.0	0.9 °	35 °	129.97	1.07	1.72	2.02	1,085,816.1	576,876.7	5,951.0
140.0	1.0 °	41 °	139.97	1.20	1.82	2.18	1,085,816.2	576,876.8	5,941.0
150.0	1.9 °	101 °	149.97	1.23	2.04	2.38	1,085,816.2	576,877.0	5,931.0
160.0	0.5 °	37 °	159.96	1.23	2.23	2.55	1,085,816.2	576,877.2	5,921.0
170.0	0.9 °	129 °	169.96	1.22	2.32	2.62	1,085,816.2	576,877.3	5,911.0
180.0	1.7 °	109 °	179.96	1.12	2.52	2.76	1,085,816.1	576,877.5	5,901.0
190.0	1.8 °	72 °	189.96	1.12	2.81	3.02	1,085,816.1	576,877.8	5,891.0
200.0	1.8 °	90 °	199.95	1.17	3.12	3.33	1,085,816.2	576,878.1	5,881.0
210.0	1.5 °	43 °	209.95	1.26	3.37	3.59	1,085,816.3	576,878.4	5,871.1
220.0	1.7 °	49 °	219.94	1.45	3.57	3.85	1,085,816.5	576,878.6	5,861.1
230.0	2.4 °	61 °	229.94	1.65	3.87	4.20	1,085,816.7	576,878.9	5,851.1
240.0	1.4 °	44 °	239.93	1.84	4.13	4.52	1,085,816.8	576,879.1	5,841.1
250.0	1.1 °	43 °	249.93	2.00	4.28	4.73	1,085,817.0	576,879.3	5,831.1
260.0	1.3 °	44 °	259.93	2.15	4.43	4.92	1,085,817.2	576,879.4	5,821.1
270.0	1.6 °	47 °	269.92	2.33	4.61	5.16	1,085,817.3	576,879.6	5,811.1
280.0	1.1 °	40 °	279.92	2.50	4.77	5.39	1,085,817.5	576,879.8	5,801.1
290.0	1.1 °	44 °	289.92	2.64	4.90	5.57	1,085,817.6	576,879.9	5,791.1
300.0	1.2 °	43 °	299.92	2.78	5.04	5.76	1,085,817.8	576,880.0	5,781.1
310.0	1.7 °	54 °	309.91	2.95	5.23	6.00	1,085,817.9	576,880.2	5,771.1
316.8	2.7 °	65 °	316.51	3.07	5.45	6.26	1,085,818.1	576,880.5	5,764.5
320.0	1.9 °	100 °	319.90	3.10	5.58	6.38	1,085,818.1	576,880.6	5,761.1
330.0	2.1 °	58 °	329.90	3.16	5.90	6.69	1,085,818.2	576,880.9	5,751.1
331.3	2.3 °	61 °	331.20	3.19	5.94	6.74	1,085,818.2	576,880.9	5,749.8

**Error Analysis**

**Bottom Coordinates**

Missed the target of the designed hole (0.0°,0.0°) by:

3.2 ft North  
5.9 ft East  
0.1 ft Higher  
-----  
6.74 feet Total  
2.04% Tolerance

Northing 1,085,818.2 +/- 0.3  
Easting 576,880.9 +/- 0.1  
Elevation 5,749.8 +/- 0.0

**EXPLANATION**

**HOLE DEPTH:** Length along the borehole axis from the collar to the point of the survey, NOT vertical depth.

**INCLINATION:** Equivalent to "Dip" with the exception that 0° INCLINATION = VERTICAL and 90° INCLINATION = HORIZONTAL. INCLINATION greater than 90° implies UP.

**DIRECTION:** Equivalent to "Bearing" of the borehole at the point of survey. Measured on a magnetic azimuth (0° -360°) system.

**DISTANCE NORTH, DISTANCE EAST –** Distances from the collar calculated on a TRUE NORTH-SOUTH grid.

**CLOSURE:** The horizontal distance from the collar of the borehole to the survey point.

**NORTHING, EASTING, ELEV.:** Coordinates calculated from the collar coordinates. Collar coordinates furnished by Geodetic Surveys, Inc.. (\* Estimated coordinates have not been surveyed.)

**BOREHOLE: MF-250A  
COLLAR COORDINATES**

NORTH 1,085,615.0 \*  
EAST 576,840.0 \*  
ELEVATION 6,081.0 \* (Estimated)

**Miner Flat Dam****DAM END OF SANDSTONE RIDGE**

Surveyed By: CHR

Date Surveyed: September 1, 1995

**BOREHOLE - AS DRILLED**

Time Surveyed: 2:23 pm

**Raw Data From Owl Technical Assoc.**

HOLE DEPTH (ft.)	Model 780 INCLINATION (from Vert.) * North=360 *	DIRECTION * North=360 *	TRUE DEPTH (ft.)	DISTANCE NORTH (ft.)	DISTANCE EAST (ft.)	CLOSURE (ft.)	NORTHING (ft.)	EASTING (ft.)	ELEVATION (ft.)
0.0	0.0 °	0 °	0.00	0.00	0.00	0.00	1,085,615.0	576,840.0	6,081.0
5.0	0.9 °	55 °	5.00	0.02	0.03	0.04	1,085,615.0	576,840.0	6,076.0
6.0	0.8 °	290 °	6.00	0.03	0.03	0.04	1,085,615.0	576,840.0	6,075.0
7.5	0.8 °	94 °	7.50	0.03	0.03	0.05	1,085,615.0	576,840.0	6,073.5
10.0	1.0 °	93 °	10.00	0.03	0.07	0.08	1,085,615.0	576,840.1	6,071.0
15.0	1.0 °	359 °	15.00	0.07	0.11	0.13	1,085,615.1	576,840.1	6,066.0
20.0	1.0 °	117 °	20.00	0.10	0.15	0.18	1,085,615.1	576,840.2	6,061.0
25.0	1.6 °	84 °	25.00	0.08	0.26	0.27	1,085,615.1	576,840.3	6,056.0
30.0	1.6 °	78 °	29.99	0.10	0.40	0.41	1,085,615.1	576,840.4	6,051.0
35.0	1.4 °	58 °	34.99	0.15	0.52	0.54	1,085,615.2	576,840.5	6,046.0
40.0	1.4 °	61 °	39.99	0.21	0.62	0.66	1,085,615.2	576,840.6	6,041.0
45.0	1.3 °	58 °	44.99	0.27	0.73	0.78	1,085,615.3	576,840.7	6,036.0
50.0	1.3 °	49 °	49.99	0.34	0.82	0.88	1,085,615.3	576,840.8	6,031.0
55.0	1.5 °	63 °	54.99	0.41	0.92	1.00	1,085,615.4	576,840.9	6,026.0
60.0	1.5 °	83 °	59.99	0.44	1.04	1.13	1,085,615.4	576,841.0	6,021.0
65.0	1.5 °	81 °	64.98	0.46	1.17	1.26	1,085,615.5	576,841.2	6,016.0
70.0	1.6 °	81 °	69.98	0.48	1.30	1.39	1,085,615.5	576,841.3	6,011.0
75.0	1.6 °	96 °	74.98	0.49	1.44	1.52	1,085,615.5	576,841.4	6,006.0
80.0	1.6 °	103 °	79.98	0.46	1.58	1.65	1,085,615.5	576,841.6	6,001.0
85.0	1.4 °	112 °	84.98	0.43	1.70	1.76	1,085,615.4	576,841.7	5,996.0
86.0	1.4 °	114 °	85.98	0.42	1.73	1.78	1,085,615.4	576,841.7	5,995.0

**Error Analysis****Bottom Coordinates**

Missed the target of the designed hole (0.0°, 0.0°) by:

0.4 ft. North

1.7 ft. East

0.0 ft. Higher

1.78 feet Total

2.07% Tolerance

Northing 1,085,615.4 +/- 0.1

Easting 576,841.7 +/- 0.0

Elevation 5,995.0 +/- 0.0

**EXPLANATION**

HOLE DEPTH: Length along the borehole axis from the collar to the point of the survey, NOT vertical depth.

INCLINATION: Equivalent to "Dip" with the exception that 0° INCLINATION = VERTICAL and 90° INCLINATION = HORIZONTAL. INCLINATION greater than 90° implies UP.

DIRECTION: Equivalent to "Bearing" of the borehole at the point of survey. Measured on a magnetic azimuth (0°-360°) system.

DISTANCE NORTH, DISTANCE EAST -- Distances from the collar calculated on a TRUE NORTH-SOUTH grid.

CLOSURE: The horizontal distance from the collar of the borehole to the survey point.

NORTHING, EASTING, ELEV.: Coordinates calculated from the collar coordinates. Collar coordinates furnished by Geodetic Surveys, Inc.  
(\* Estimated coordinates have not been surveyed.)

**BOREHOLE: MF-251  
COLLAR COORDINATES**

NORTH 1,085,390.0 \*  
EAST 576,875.0 \*  
ELEVATION 6,089.0 \* (Estimated)

**Minor Flat Dam**

**Secondary Access Road**

**BOREHOLE - AS DRILLED**

Surveyed By: da

Date Surveyed: August 21, 1995

Time Surveyed: 5:03 pm,

**Raw Data From Owl Technical Assoc.**

HOLE DEPTH (ft.)	Model 780 INCLINATION (from Vert.)	DIRECTION * North=360 *	TRUE DEPTH (ft.)	DISTANCE NORTH (ft.)	DISTANCE EAST (ft.)	CLOSURE (ft.)	NORTHING (ft.)	EASTING (ft.)	ELEVATION (ft.)
0.0	0.0 *	0 *	0.00	0.00	0.00	0.00	1,085,390.0	576,875.0	6,089.0
10.0	1.8 *	110 *	10.00	-0.05	0.15	0.16	1,085,389.9	576,875.1	6,079.0
20.0	2.0 *	189 *	19.99	-0.28	0.27	0.39	1,085,389.7	576,875.3	6,069.0
30.0	2.0 *	291 *	29.99	-0.39	0.08	0.40	1,085,389.6	576,875.1	6,059.0
40.0	2.1 *	309 *	39.98	-0.21	-0.23	0.31	1,085,389.8	576,874.8	6,049.0
50.0	0.9 *	210 *	49.98	-0.16	-0.41	0.44	1,085,389.8	576,874.6	6,039.0
60.0	1.6 *	352 *	59.97	-0.09	-0.47	0.48	1,085,389.9	576,874.5	6,029.0
70.0	2.1 *	288 *	69.97	0.10	-0.66	0.67	1,085,390.1	576,874.3	6,019.0
80.0	2.0 *	113 *	79.96	0.09	-0.68	0.68	1,085,390.1	576,874.3	6,009.0
90.0	2.1 *	97 *	89.95	-0.00	-0.33	0.33	1,085,390.0	576,874.7	5,999.0
100.0	2.1 *	14 *	99.95	0.15	-0.11	0.19	1,085,390.2	576,874.9	5,989.1
110.0	2.0 *	25 *	109.94	0.49	0.01	0.49	1,085,390.5	576,875.0	5,979.1
120.0	2.1 *	222 *	119.94	0.51	-0.04	0.51	1,085,390.5	576,875.0	5,969.1
130.0	2.2 *	304 *	129.93	0.48	-0.32	0.58	1,085,390.5	576,874.7	5,959.1
140.0	2.2 *	342 *	139.92	0.77	-0.54	0.94	1,085,390.8	576,874.5	5,949.1
150.0	2.0 *	67 *	149.91	1.02	-0.44	1.11	1,085,391.0	576,874.6	5,939.1
160.0	2.0 *	33 *	159.91	1.24	-0.18	1.25	1,085,391.2	576,874.8	5,929.1
170.0	1.9 *	98 *	169.90	1.36	0.08	1.36	1,085,391.4	576,875.1	5,919.1
180.0	1.9 *	6 *	179.90	1.50	0.26	1.53	1,085,391.5	576,875.3	5,909.1
190.0	1.4 *	252 *	189.89	1.63	0.16	1.64	1,085,391.6	576,875.2	5,899.1
200.0	1.6 *	176 *	199.89	1.45	0.05	1.45	1,085,391.5	576,875.1	5,889.1
210.0	1.1 *	79 *	209.89	1.33	0.16	1.34	1,085,391.3	576,875.2	5,879.1
220.0	1.9 *	87 *	219.88	1.36	0.42	1.42	1,085,391.4	576,875.4	5,869.1
230.0	1.4 *	76 *	229.88	1.40	0.70	1.56	1,085,391.4	576,875.7	5,859.1
240.0	0.9 *	74 *	239.88	1.45	0.90	1.70	1,085,391.4	576,875.9	5,849.1
250.0	0.9 *	69 *	249.88	1.50	1.05	1.83	1,085,391.5	576,876.0	5,839.1
260.0	0.9 *	68 *	259.87	1.56	1.19	1.96	1,085,391.6	576,876.2	5,829.1
270.0	1.4 *	81 *	269.87	1.60	1.39	2.12	1,085,391.6	576,876.4	5,819.1
280.0	1.0 *	71 *	279.87	1.65	1.59	2.29	1,085,391.7	576,876.6	5,809.1
290.0	1.0 *	76 *	289.87	1.70	1.76	2.44	1,085,391.7	576,876.8	5,799.1
300.0	1.0 *	166 *	299.87	1.64	1.86	2.48	1,085,391.6	576,876.9	5,789.1
310.0	0.9 *	165 *	309.86	1.48	1.90	2.41	1,085,391.5	576,876.9	5,779.1
320.0	0.5 *	176 *	319.86	1.36	1.93	2.36	1,085,391.4	576,876.9	5,769.1
330.0	0.3 *	124 *	329.86	1.30	1.95	2.34	1,085,391.3	576,877.0	5,759.1
331.8	0.4 *	142 *	331.66	1.29	1.96	2.35	1,085,391.3	576,877.0	5,757.3

**Error Analysis  
Bottom Coordinates**

Missed the target of the designed hole (0.0°,0.0°) by:  
1.3 ft. North  
2.0 ft. East  
0.1 ft. Higher  
-----  
2.35 feet Total  
0.71% Tolerance

Northing 1,085,391.3 +/- 0.3  
Easting 576,877.0 +/- 0.1  
Elevation 5,757.3 +/- 0.0

**EXPLANATION**

HOLE DEPTH: Length along the borehole axis from the collar to the point of the survey, NOT vertical depth.

INCLINATION: Equivalent to "Dip" with the exception that 0° INCLINATION = VERTICAL and 90° INCLINATION = HORIZONTAL. INCLINATION greater than 90° implies UP.

DIRECTION: Equivalent to "Bearing" of the borehole at the point of survey. Measured on a magnetic azimuth (0°-360°) system.

DISTANCE NORTH, DISTANCE EAST – Distances from the collar calculated on a TRUE NORTH-SOUTH grid.

CLOSURE: The horizontal distance from the collar of the borehole to the survey point.

NORTHING, EASTING, ELEV.: Coordinates calculated from the collar coordinates. Collar coordinates furnished by Geodetic Surveys, Inc.  
(\* Estimated coordinates have not been surveyed.)

**BOREHOLE: MF-252**  
**COLLAR COORDINATES**

NORTH 1,085,205.0 \*  
 EAST 576,845.0 \*  
 ELEVATION 6,082.0 \* (Estimated)

**Miner Flat Dam**

**Secondary Access Road**

**BOREHOLE - AS DRILLED**

Surveyed By: DA

Date Surveyed: August 22, 1995

Time Surveyed: 09:29 am

Raw Data From Owl Technical Assoc.

HOLE DEPTH (ft.)	Model 780 INCLINATION (from Vert.)	DIRECTION North=360°	TRUE DEPTH (ft.)	DISTANCE NORTH (ft.)	DISTANCE EAST (ft.)	CLOSURE (ft.)	NORTHING (ft.)	EASTING (ft.)	ELEVATION (ft.)
0.0	0.0°	0°	0.00	0.00	0.00	0.00	1,085,205.0	576,845.0	6,082.0
10.0	2.5°	8°	10.00	0.22	0.03	0.22	1,085,205.2	576,845.0	6,072.0
20.0	2.4°	323°	19.99	0.60	-0.07	0.60	1,085,205.6	576,844.9	6,062.0
30.0	1.8°	19°	29.98	0.91	-0.14	0.93	1,085,205.9	576,844.9	6,052.0
40.0	1.6°	22°	39.97	1.19	-0.04	1.19	1,085,206.2	576,845.0	6,042.0
50.0	1.6°	24°	49.97	1.45	0.07	1.45	1,085,206.4	576,845.1	6,032.0
60.0	1.7°	21°	59.97	1.72	0.18	1.73	1,085,206.7	576,845.2	6,022.0
70.0	1.5°	21°	69.96	1.98	0.28	2.00	1,085,207.0	576,845.3	6,012.0
80.0	1.6°	23°	79.96	2.23	0.38	2.26	1,085,207.2	576,845.4	6,002.0
90.0	2.2°	54°	89.95	2.47	0.59	2.54	1,085,207.5	576,845.6	5,992.0
100.0	2.1°	57°	99.95	2.68	0.90	2.83	1,085,207.7	576,845.9	5,982.1
110.0	1.8°	18°	109.94	2.93	1.10	3.13	1,085,207.9	576,846.1	5,972.1
120.0	2.2°	51°	119.93	3.20	1.30	3.46	1,085,208.2	576,846.3	5,962.1
130.0	2.5°	30°	129.93	3.51	1.56	3.84	1,085,208.5	576,846.6	5,952.1
140.0	2.5°	24°	139.92	3.90	1.76	4.28	1,085,208.9	576,846.8	5,942.1
150.0	1.5°	26°	149.91	4.22	1.90	4.63	1,085,209.2	576,846.9	5,932.1
160.0	1.6°	20°	159.91	4.46	2.01	4.90	1,085,209.5	576,847.0	5,922.1
170.0	1.3°	14°	169.90	4.71	2.08	5.15	1,085,209.7	576,847.1	5,912.1
180.0	2.2°	55°	179.90	4.93	2.27	5.42	1,085,209.9	576,847.3	5,902.1
190.0	2.4°	39°	189.89	5.20	2.56	5.79	1,085,210.2	576,847.6	5,892.1
200.0	2.2°	48°	199.88	5.49	2.83	6.18	1,085,210.5	576,847.8	5,882.1
210.0	2.4°	39°	209.87	5.78	3.11	6.56	1,085,210.8	576,848.1	5,872.1
220.0	2.3°	49°	219.87	6.07	3.39	6.96	1,085,211.1	576,848.4	5,862.1
230.0	1.3°	37°	229.86	6.30	3.61	7.26	1,085,211.3	576,848.6	5,852.1
240.0	1.5°	18°	239.86	6.51	3.72	7.50	1,085,211.5	576,848.7	5,842.1
250.0	1.4°	20°	249.85	6.75	3.80	7.75	1,085,211.8	576,848.8	5,832.1
260.0	1.9°	10°	259.85	7.03	3.87	8.03	1,085,212.0	576,848.9	5,822.2
270.0	1.7°	9°	269.84	7.34	3.92	8.32	1,085,212.3	576,848.9	5,812.2
280.0	1.7°	11°	279.84	7.63	3.98	8.60	1,085,212.6	576,849.0	5,802.2
290.0	1.3°	22°	289.84	7.88	4.05	8.86	1,085,212.9	576,849.0	5,792.2
300.0	2.2°	29°	299.83	8.16	4.18	9.16	1,085,213.2	576,849.2	5,782.2
310.0	1.1°	22°	309.83	8.41	4.31	9.45	1,085,213.4	576,849.3	5,772.2
320.0	1.5°	12°	319.82	8.63	4.37	9.67	1,085,213.6	576,849.4	5,762.2
330.0	2.5°	20°	329.82	8.96	4.48	10.02	1,085,214.0	576,849.5	5,752.2

**Error Analysis**

**Bottom Coordinates**

Missed the target of the designed hole (0.0°,0.0°) by:

9.0 ft North  
 4.5 ft East  
 0.2 ft Higher  
 -----  
 10.02 feet Total  
 3.04% Tolerance

Northing 1,085,214.0 +/- 0.2  
 Easting 576,849.5 +/- 0.2  
 Elevation 5,752.2 +/- 0.0

**EXPLANATION**

**HOLE DEPTH:** Length along the borehole axis from the collar to the point of the survey, NOT vertical depth.

**INCLINATION:** Equivalent to "Dip" with the exception that 0° INCLINATION = VERTICAL and 90° INCLINATION = HORIZONTAL. INCLINATION greater than 90° implies UP.

**DIRECTION:** Equivalent to "Bearing" of the borehole at the point of survey. Measured on a magnetic azimuth (0°-360°) system.

**DISTANCE NORTH, DISTANCE EAST –** Distances from the collar calculated on a TRUE NORTH-SOUTH grid.

**CLOSURE:** The horizontal distance from the collar of the borehole to the survey point.

**NORTHING, EASTING, ELEV.:** Coordinates calculated from the collar coordinates. Collar coordinates furnished by Geodetic Surveys, Inc. (\* Estimated coordinates have not been surveyed.)

**BOREHOLE: MF-253**  
**COLLAR COORDINATES**

NORTH 1,084,815.0 \*  
 EAST 576,940.0 \*  
 ELEVATION 6,079.0 \* (Estimated)

**MINER FLAT**

**ROAD E**

**BOREHOLE - AS DRILLED**

Surveyed By: DA

Date Surveyed: June 19, 1995

Time Surveyed: 12:45 pm

Raw Data From Owl Technical Assoc.

HOLE DEPTH (ft.)	Model 780 INCLINATION (from Vert.)	DIRECTION North=360°	TRUE DEPTH (ft.)	DISTANCE NORTH (ft.)	DISTANCE EAST (ft.)	CLOSURE (ft.)	NORTHING (ft.)	EASTING (ft.)	ELEVATION (ft.)
0.0	0.0°	0°	0.00	0.00	0.00	0.00	1,084,815.0	576,940.0	6,079.0
10.0	0.7°	31°	10.00	0.05	0.03	0.06	1,084,815.1	576,940.0	6,069.0
20.0	0.3°	107°	20.00	0.10	0.09	0.13	1,084,815.1	576,940.1	6,059.0
30.0	0.9°	158°	30.00	0.02	0.14	0.14	1,084,815.0	576,940.1	6,049.0
40.0	1.0°	217°	40.00	-0.13	0.12	0.17	1,084,814.9	576,940.1	6,039.0
50.0	1.0°	232°	50.00	-0.25	-0.00	0.25	1,084,814.8	576,940.0	6,029.0
60.0	1.0°	257°	59.99	-0.32	-0.18	0.36	1,084,814.7	576,939.8	6,019.0
70.0	1.1°	223°	69.99	-0.41	-0.31	0.51	1,084,814.6	576,939.7	6,009.0
80.0	0.9°	179°	79.99	-0.56	-0.37	0.67	1,084,814.4	576,939.6	5,999.0
90.0	0.9°	158°	89.99	-0.71	-0.34	0.79	1,084,814.3	576,939.7	5,989.0
100.0	0.8°	278°	99.99	-0.78	-0.38	0.86	1,084,814.2	576,939.6	5,979.0
110.0	1.1°	211°	109.99	-0.85	-0.50	0.98	1,084,814.2	576,939.5	5,969.0
120.0	0.6°	287°	119.99	-0.92	-0.80	1.09	1,084,814.1	576,939.4	5,959.0
130.0	1.3°	253°	129.98	-0.93	-0.76	1.20	1,084,814.1	576,939.2	5,949.0
140.0	1.3°	229°	139.98	-1.04	-0.95	1.41	1,084,814.0	576,939.1	5,939.0
150.0	1.3°	281°	149.98	-1.13	-1.15	1.81	1,084,813.9	576,938.9	5,929.0
160.0	0.5°	143°	159.98	-1.19	-1.23	1.71	1,084,813.8	576,938.8	5,919.0
170.0	0.7°	181°	169.98	-1.28	-1.19	1.74	1,084,813.7	576,938.8	5,909.0
180.0	1.0°	177°	179.98	-1.42	-1.16	1.84	1,084,813.6	576,938.8	5,899.0
190.0	0.7°	162°	189.97	-1.57	-1.14	1.94	1,084,813.4	576,938.9	5,889.0
200.0	1.0°	174°	199.97	-1.71	-1.11	2.04	1,084,813.3	576,938.9	5,879.0
210.0	0.9°	171°	209.97	-1.88	-1.09	2.17	1,084,813.1	576,938.9	5,869.0
220.0	0.7°	161°	219.97	-2.01	-1.06	2.27	1,084,813.0	576,938.9	5,859.0
230.0	1.1°	170°	229.97	-2.17	-1.02	2.39	1,084,812.8	576,939.0	5,849.0
240.0	1.0°	189°	239.97	-2.35	-1.02	2.56	1,084,812.7	576,939.0	5,839.0
250.0	0.9°	176°	249.97	-2.51	-1.03	2.71	1,084,812.5	576,939.0	5,829.0
260.0	0.2°	287°	259.97	-2.58	-1.04	2.78	1,084,812.4	576,939.0	5,819.0
270.0	1.4°	295°	269.96	-2.53	-1.16	2.78	1,084,812.5	576,938.8	5,809.0
280.0	1.2°	287°	279.96	-2.44	-1.38	2.81	1,084,812.6	576,938.6	5,799.0
290.0	1.7°	245°	289.96	-2.48	-1.81	2.95	1,084,812.5	576,938.4	5,789.0
300.0	1.4°	268°	299.96	-2.54	-1.87	3.16	1,084,812.5	576,938.1	5,779.0
310.0	1.5°	272°	309.95	-2.54	-2.12	3.31	1,084,812.5	576,937.9	5,769.0
320.0	1.4°	277°	319.95	-2.52	-2.37	3.46	1,084,812.5	576,937.6	5,759.1

**Error Analysis**  
**Bottom Coordinates**

Missed the target of the designed hole (0.0°,0.0°) by:

2.5 ft. North  
 1.9 ft. East  
 20.0 ft. Higher  
 20.29 feet Total  
 6.34% Tolerance

Northing 1,084,812.5 +/- 0.2  
 Easting 576,937.6 +/- 0.2  
 Elevation 5,759.1 +/- 0.0

**EXPLANATION**

**HOLE DEPTH:** Length along the borehole axis from the collar to the point of the survey, NOT vertical depth.

**INCLINATION:** Equivalent to "Dip" with the exception that 0° INCLINATION = VERTICAL and 90° INCLINATION = HORIZONTAL. INCLINATION greater than 90° implies UP.

**DIRECTION:** Equivalent to "Bearing" of the borehole at the point of survey. Measured on a magnetic azimuth (0°-360°) system.

**DISTANCE NORTH, DISTANCE EAST –** Distances from the collar calculated on a TRUE NORTH-SOUTH grid.

**CLOSURE:** The horizontal distance from the collar of the borehole to the survey point.

**NORTHING, EASTING, ELEV.:** Coordinates calculated from the collar coordinates. Collar coordinates furnished by Geodetic Surveys, Inc.. (\* Estimated coordinates have not been surveyed.)

**BOREHOLE: MF-254**  
**COLLAR COORDINATES**

NORTH 1,084,687.0 \*  
EAST 577,105.0 \*  
ELEVATION 6,082.0 \* (Estimated)

**Miner Flat Dam**

**Secondary Access Road - South End**

**BOREHOLE - AS DRILLED**

Surveyed By: DA

Date Surveyed: August 22, 1995

Time Surveyed: 10:15 am

Raw Data From Owl Technical Assoc.

HOLE DEPTH (ft.)	INCLINATION (from Vert.)	DIRECTION North=360 *	TRUE DEPTH (ft.)	DISTANCE NORTH (ft.)	DISTANCE EAST (ft.)	CLOSURE (ft.)	NORTHING (ft.)	EASTING (ft.)	ELEVATION (ft.)
0.0	0.0	0	0.00	0.00	0.00	0.00	1,084,687.0	577,105.0	6,082.0
10.0	0.9	86	10.00	0.01	0.08	0.08	1,084,687.0	577,105.1	6,072.0
20.0	0.5	200	20.00	-0.03	0.14	0.14	1,084,687.0	577,105.1	6,062.0
30.0	0.8	318	30.00	-0.02	0.08	0.08	1,084,687.0	577,105.1	6,052.0
40.0	0.4	23	40.00	0.06	0.05	0.08	1,084,687.1	577,105.0	6,042.0
50.0	0.8	195	50.00	0.03	0.04	0.05	1,084,687.0	577,105.0	6,032.0
60.0	1.1	225	60.00	-0.11	-0.04	0.11	1,084,686.9	577,105.0	6,022.0
70.0	1.0	304	69.99	-0.12	-0.18	0.22	1,084,686.9	577,104.8	6,012.0
80.0	0.9	297	79.99	-0.04	-0.33	0.33	1,084,687.0	577,104.7	6,002.0
90.0	0.7	322	89.99	0.04	-0.43	0.44	1,084,687.0	577,104.6	5,992.0
100.0	1.2	283	99.99	0.12	-0.57	0.58	1,084,687.1	577,104.4	5,982.0
110.0	1.2	277	109.99	0.15	-0.78	0.79	1,084,687.2	577,104.2	5,972.0
120.0	1.3	237	119.99	0.10	-0.98	0.98	1,084,687.1	577,104.0	5,962.0
130.0	1.1	206	129.98	-0.05	-1.12	1.12	1,084,687.0	577,103.9	5,952.0
140.0	1.1	220	139.98	-0.21	-1.22	1.24	1,084,686.8	577,103.8	5,942.0
150.0	0.1	142	149.98	-0.29	-1.28	1.31	1,084,686.7	577,103.7	5,932.0
160.0	1.3	251	159.98	-0.33	-1.38	1.42	1,084,686.7	577,103.6	5,922.0
170.0	0.9	291	169.98	-0.34	-1.56	1.59	1,084,686.7	577,103.4	5,912.0
180.0	1.1	276	179.98	-0.30	-1.73	1.75	1,084,686.7	577,103.3	5,902.0
190.0	1.1	277	189.97	-0.28	-1.92	1.94	1,084,686.7	577,103.1	5,892.0
200.0	0.5	133	199.97	-0.30	-1.98	2.00	1,084,686.7	577,103.0	5,882.0
210.0	0.4	109	209.97	-0.34	-1.92	1.95	1,084,686.7	577,103.1	5,872.0
220.0	1.0	291	219.97	-0.32	-1.96	1.99	1,084,686.7	577,103.0	5,862.0
230.0	1.1	268	229.97	-0.29	-2.14	2.16	1,084,686.7	577,102.9	5,852.0
240.0	1.1	290	239.97	-0.26	-2.33	2.34	1,084,686.7	577,102.7	5,842.0
250.0	0.8	304	249.97	-0.19	-2.48	2.48	1,084,686.8	577,102.5	5,832.0
260.0	1.2	280	259.96	-0.13	-2.64	2.64	1,084,686.9	577,102.4	5,822.0
270.0	1.1	292	269.96	-0.08	-2.83	2.83	1,084,686.9	577,102.2	5,812.0
280.0	1.0	298	279.96	-0.00	-3.00	3.00	1,084,687.0	577,102.0	5,802.0
290.0	1.1	281	289.96	0.06	-3.17	3.17	1,084,687.1	577,101.8	5,792.0
300.0	0.4	134	299.96	0.05	-3.24	3.24	1,084,687.1	577,101.8	5,782.0
310.0	0.8	162	309.96	-0.04	-3.19	3.19	1,084,687.0	577,101.8	5,772.0
320.0	1.2	261	319.96	-0.12	-3.27	3.27	1,084,686.9	577,101.7	5,762.0
325.1	1.5	270	325.05	-0.13	-3.39	3.39	1,084,686.9	577,101.6	5,756.9

**Error Analysis**

**Bottom Coordinates**

Missed the target of the designed hole (0.0°,0.0°) by:

0.1 ft. North

3.4 ft. East

0.0 ft. Higher

-----  
3.39 feet Total

1.04% Tolerance

Northing 1,084,686.9 +/- 0.3

Easting 577,101.6 +/- 0.0

Elevation 5,756.9 +/- 0.0

**EXPLANATION**

HOLE DEPTH: Length along the borehole axis from the collar to the point of the survey, NOT vertical depth.

INCLINATION: Equivalent to "Dip" with the exception that 0° INCLINATION = VERTICAL and 90° INCLINATION = HORIZONTAL. INCLINATION greater than 90° implies UP.

DIRECTION: Equivalent to "Bearing" of the borehole at the point of survey. Measured on a magnetic azimuth (0°-360°) system.

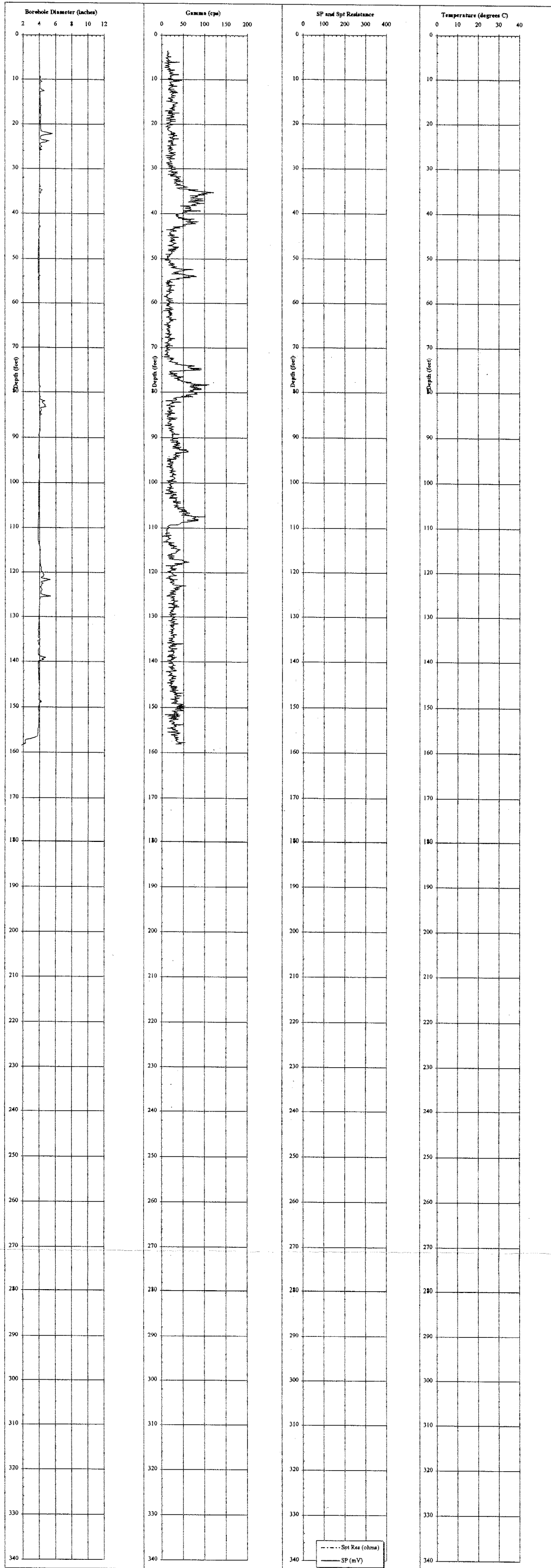
DISTANCE NORTH, DISTANCE EAST – Distances from the collar calculated on a TRUE NORTH-SOUTH grid.

CLOSURE: The horizontal distance from the collar of the borehole to the survey point.

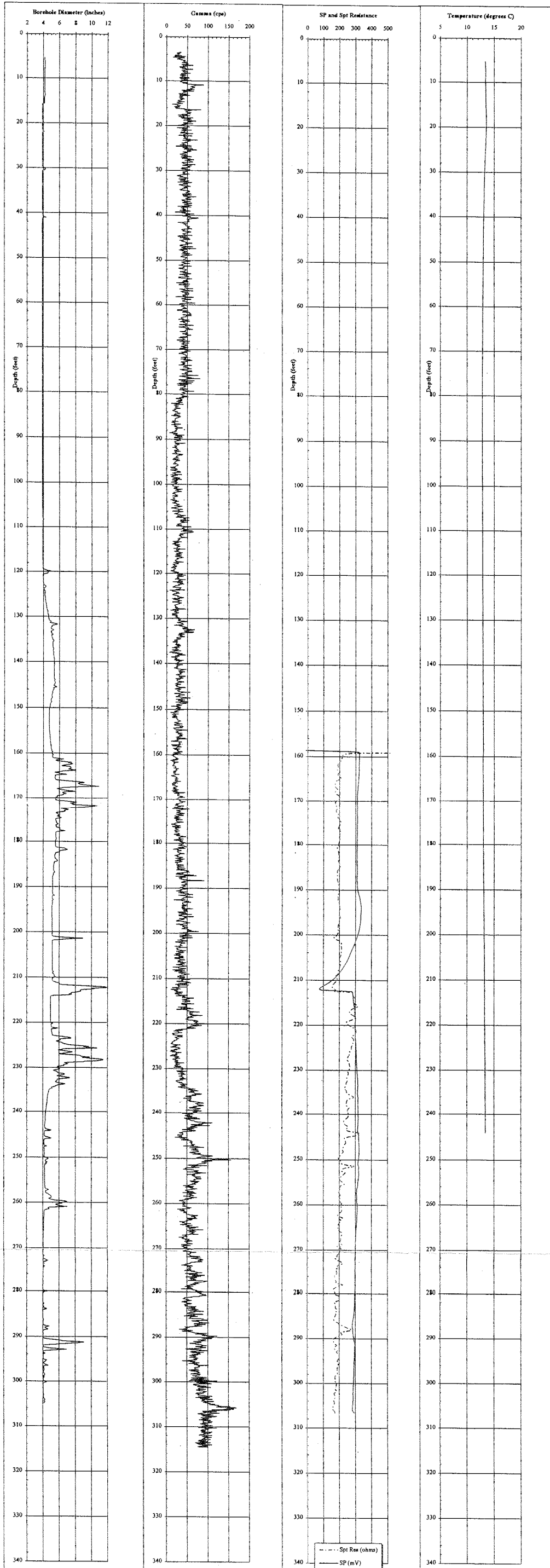
NORTHING, EASTING, ELEV.: Coordinates calculated from the collar coordinates. Collar coordinates furnished by Geodetic Surveys, Inc.. (\* Estimated coordinates have not been surveyed.)

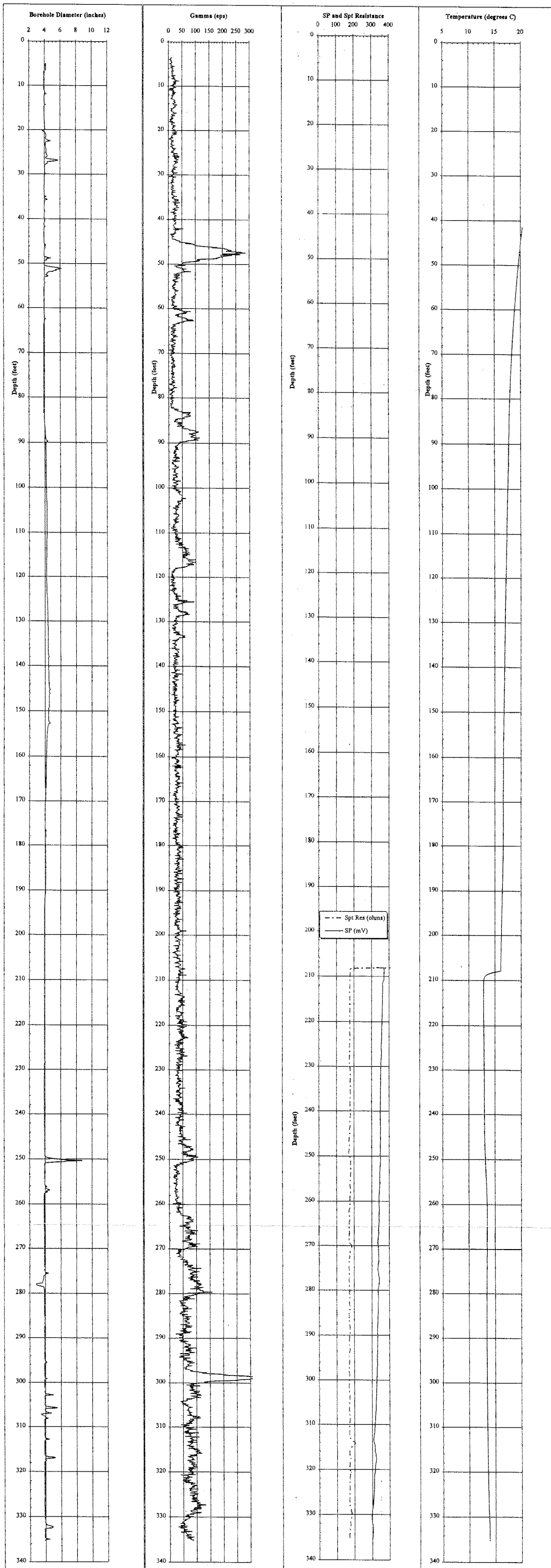
**APPENDIX D**

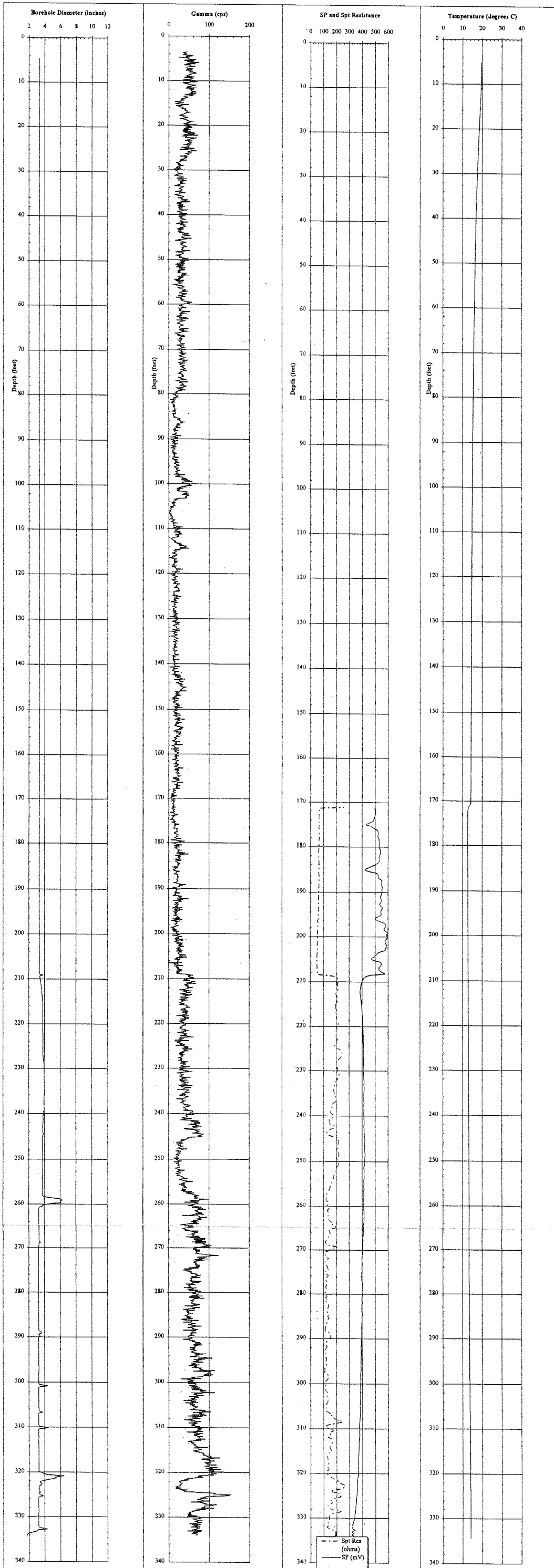
**GEOPHYSICS**

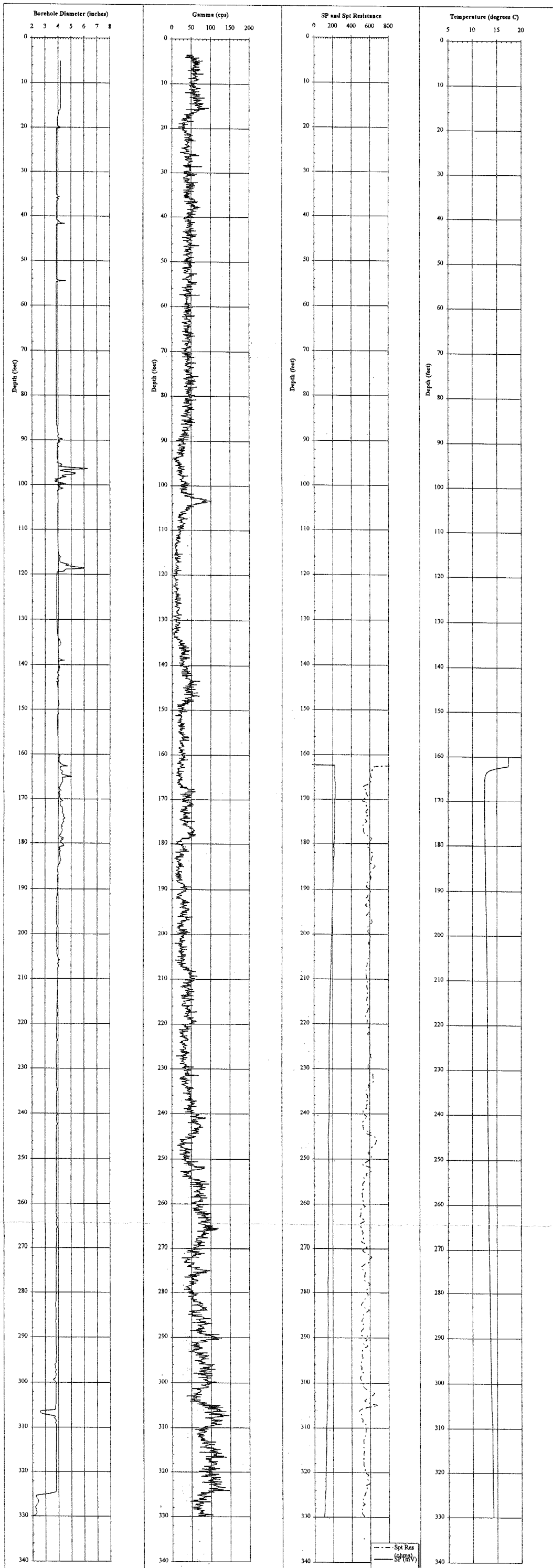


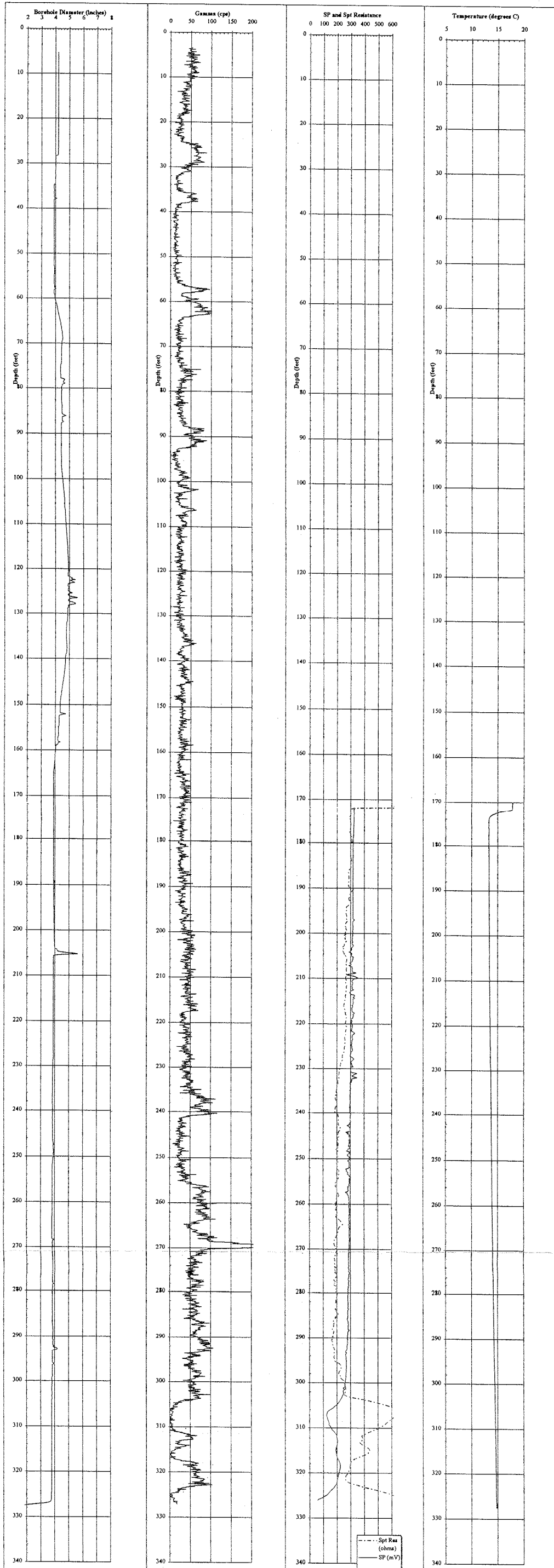


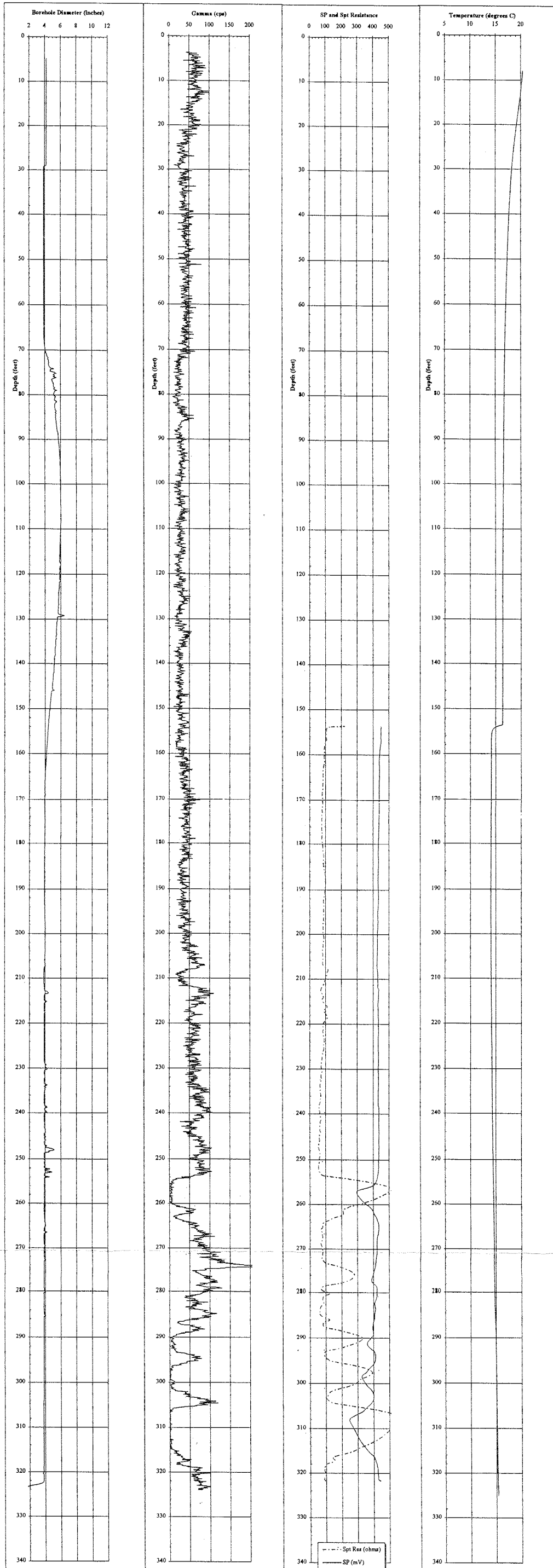












**APPENDIX E**

**PIEZOMETER INSTALLATIONS**

### PIEZOMETER CONSTRUCTION SUMMARY

Survey Coords:                       
 Northing: 1085006.50  
 Easting: 576834.81

Elevation Ground Level 6107.52 ft.  
 Top of PVC Casing 6108.42 ft.

**Drilling Summary:**

Total Depth 362.0 ft.  
 Borehole Diameter 3.65 in.  
 Casing Stickup Height 0.90 ft.  
 Driller B. Mathews

Rig Dresser DBS-25  
 Bit(s) RSG 3.65 in. Core Bit

Drilling Fluid Polymer/Water

Protective Casing 6 ft.-2 in., 4 in. dia. Steel

**Well Design & Specifications**

Basis: Geologic Log X Geophysical Log                       
 Casing string(s): C = Casing S = Screen

Depth	String(s)	Elevation
+ 0.90 - 206.00	C1	6108.42 - 5901.52
206.00 - 226.00	S1	5901.52 - 5881.52
- - - - -	- - - - -	- - - - -
- - - - -	- - - - -	- - - - -
- - - - -	- - - - -	- - - - -
- - - - -	- - - - -	- - - - -

Casing: C1 1.5 in. Schedule 80, Flush Threaded PVC

Screen: S1 1.5 in. Schedule 80, Flush Threaded  
0.020 Slot PVC

Filter Pack: 10-12 Silica Sand (196.0-228.0 ft.)

Grout Seal: Portland Type I-II Cement Slurry with 5%  
Quickgel Bentonite Powder (0.0-2.0 ft.)

Backfill: 3/4 in. Gravel (2.0-178.0 and  
243.0-362.0 ft.)

Bentonite Seal: Wyoming Grade 3/8 in. Bentonite  
Holeplug Bentonite Chips  
(178.0-196.0 and 228.0-243.0 ft.)

**Construction Time log:**

Task	Date	Start		Finish	
		Date	Time	Date	Time
Drilling	8/21/95	14:00	5/28/95	17:30	
Casing:					
S1	12/18/95	9:52	12/18/95	9:55	
C1	12/18/95	9:55	12/18/95	10:28	
Filter Placement:	12/18/95	10:28	12/18/95	10:55	
Cementing:	12/18/95	18:04	12/18/95	18:08	
Development:					

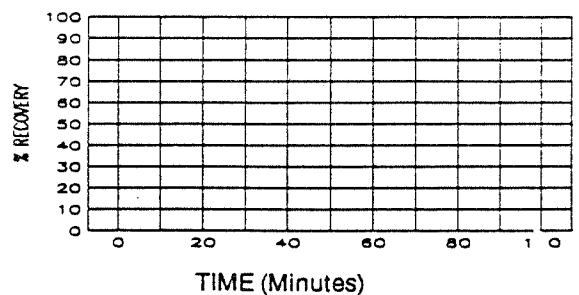
**Well Development**

Water Level on 12/18/95 = 149.95 FBTC

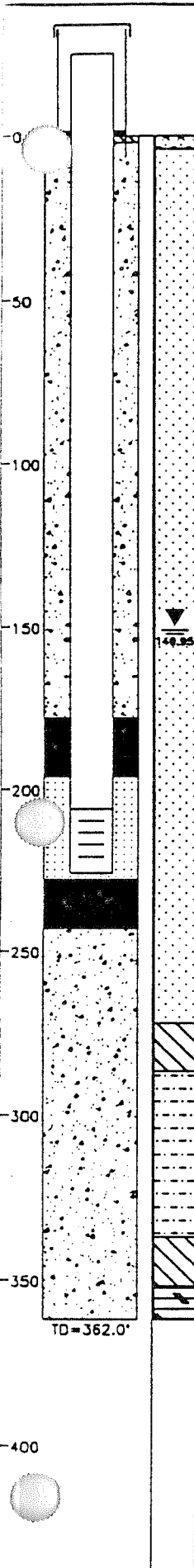
**Stabilization Test Data:**

Time	pH	Spec. Cond.	Temp (°C)

**Recovery Data:**



Comments: Used coupling (PVC) at 130.0 ft.; glued with Uni-Weld 1200 PVC Cement/ACE 44393  
PVC Pipe Cleaner. Upper backfilling took only ~18 gallons of gravel; may have bridged.



Not to Horiz. Scale

Supervised by B. Johnson  
 Job Number 943-27691.140

Site Miner Flat Dam  
 File Name MF-218A







### PIEZOMETER CONSTRUCTION SUMMARY

Survey Coords:                       
 Northing: 1085229.89  
 Easting: 578821.43

Elevation Ground Level 6074.16 ft.  
 Top of PVC Casing 6074.91 ft.

**Drilling Summary:**

Total Depth 332.7 ft.  
 Borehole Diameter 3.65 in.  
 Casing Stickup Height 0.75 ft.  
 Driller B. Mathews

Rig Dresser DBS-25  
 Bit(s) RSG 3.65 in. Core Bit

Drilling Fluid Polymer/Water

Protective Casing 20 ft., 4 in. dia. Steel

**Construction Time log:**

Task	Start		Finish	
	Date	Time	Date	Time
Drilling	8/15/96	7:30	8/2/96	11:30
Casing:				
S1	12/17/95	14:42	12/17/95	14:45
C1	12/17/95	14:45	12/17/95	15:30
Filter Placement:	12/17/95	16:00	12/17/95	16:30
Cementing:	12/18/95	16:32	12/18/95	16:40
Development:				

**Well Design & Specifications**

Basis: Geologic Log X Geophysical Log                       
 Casing string(s): C = Casing S = Screen

Depth	String(s)	Elevation
+ 0.75 - 160.00	C1	6074.91 - 5914.16
160.00 - 180.00	S1	5914.16 - 5894.16
- - -	-	- - -
- - -	-	- - -
- - -	-	- - -
- - -	-	- - -

Casing: C1 1.5 in. Schedule 80, Flush Threaded PVC

Screen: S1 1.5 in. Schedule 80, Flush Threaded 0.020 Slot PVC

Filter Pack: 10-12 Silica Sand (148.0-182.0 ft.)

Grout Seal: Portland Type I-II Cement Slurry with 5% Quickgel Bentonite Powder (0.0-10.0 ft.)

Backfill: 3/4 in. Gravel (10.0-130.0 and 197.0-332.7 ft.)

Bentonite Seal: Wyoming Grade 3/8 in. Bentonite Holeplug Bentonite Chips (130.0-148.0 and 182.0-197.0 ft.)

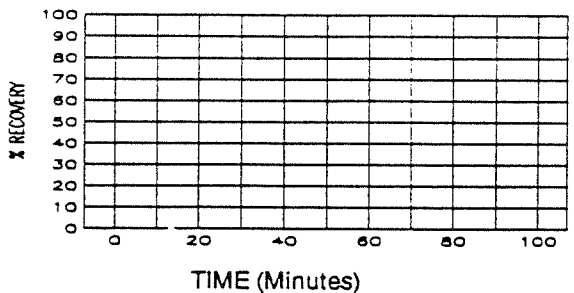
**Well Development**

Water Level on 12/18/95 = 161.11 FBTC

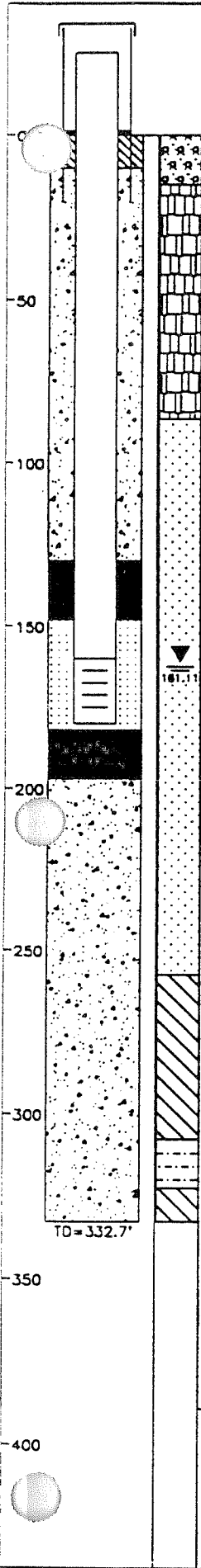
**Stabilization Test Data:**

Time	pH	Spec. Cond.	Temp (°C)

**Recovery Data:**



Comments: Some caving/bridging near top of upper bentonite seal, added additional 1/2 bag to ensure at least 15 ft. of seal; used coupling (galvanized steel) at 150.0 ft.



Not to Horiz. Scale

Supervised by B. Johnson  
 Job Number 943-27891.140

Site Miner Flat Dam  
 File Name MF-252

# PIEZOMETER CONSTRUCTION SUMMARY

Survey Coords:      Northing: 1084797.33  
 Easting: 578874.74

Elevation Ground Level 6071.41 ft.  
 Top of PVC Casing 6072.61 ft.

### Drilling Summary:

Total Depth 327.2 ft.  
 Borehole Diameter 3.65 in.  
 Casing Stickup Height 1.20 ft.  
 Driller B. Mathews

Rig Dresser DBS-25  
 Bit(s) RSG 3.65 in. Core Bit

Drilling Fluid Polymer/Water

Protective Casing 35 ft., 4 in. dia. Steel

### Well Design & Specifications

Basis: Geologic Log X      Geophysical Log \_\_\_\_\_  
 Casing string(s): C = Casing    S = Screen

Depth	String(s)	Elevation
+ 1.20 - 177.00	C1	6072.61 - 5894.41
177.00 - 197.00	S1	5894.41 - 5874.41
-	-	-
-	-	-
-	-	-
-	-	-
-	-	-

Casing: C1 1.5 in. Schedule 80, Flush Threaded PVC

Screen: S1 1.5 in. Schedule 80, Flush Threaded  
 0.020 Slot PVC

Filter Pack: 10-12 Silica Sand (167.0-199.0 ft.)

Grout Seal: Portland Type I-II Cement Slurry with 5%  
 Quickgel Bentonite Powder (0.0-10.0 ft.)

Backfill: 3/4 in. Gravel (10.0-152.0 and  
 215.0-327.2 ft.)

Bentonite Seal: Wyoming Grade 3/8 in. Bentonite  
 Holeplug Bentonite Chips  
 (152.0-167.0 and 199.0-215.0 ft.)

Comments: \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

### Construction Time log:

Task	Start		Finish	
	Date	Time	Date	Time
Drilling	6/3/95	14:42	8/21/95	13:50
Casing:				
S1	12/15/95	8:56	12/15/95	9:00
C1	12/15/95	9:00	12/15/95	9:08
Filter Placement:	12/15/95	9:10	12/15/95	11:30
Cementing:	12/18/95	15:45	12/18/95	15:50
Development:				

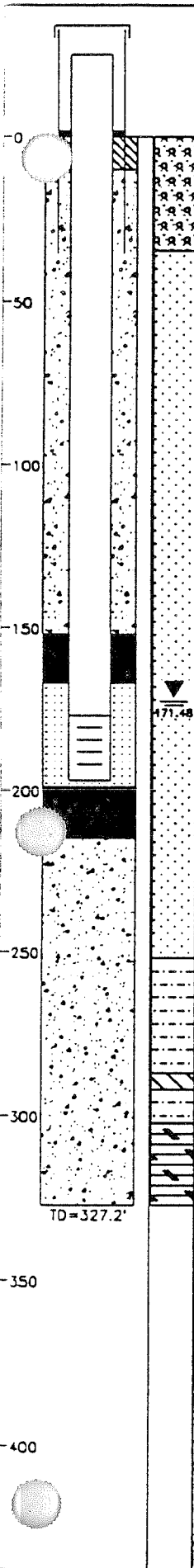
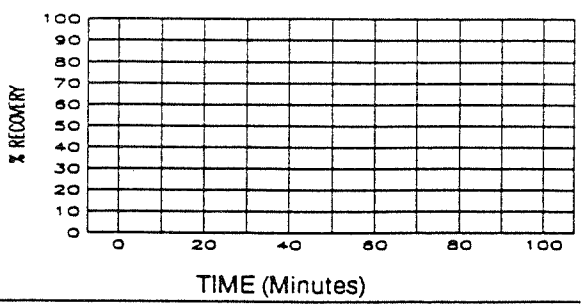
### Well Development

Water Level on 12/18/95 = 171.48 FBTC  
 \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

### Stabilization Test Data:

Time	pH	Spec. Cond.	Temp (°C)

### Recovery Data:



Not to Horiz. Scale

Supervised by B. Johnson  
 Job Number 943-27891.140

Site Miner Flat Dam  
 File Name MF-253

### PIEZOMETER CONSTRUCTION SUMMARY

Survey Coords: Northing: 1084655.39  
 Easting: 577063.74

Elevation Ground Level 6061.25 ft.  
 Top of PVC Casing 6062.05 ft.

**Drilling Summary:**

Total Depth 325.2 ft.  
 Borehole Diameter 3.65 in.  
 Casing Stickup Height 0.80 ft.  
 Driller B. Mathews

Rig Dresser DBS-25  
 Bit(s) RSG 3.65 in. Core Bit

Drilling Fluid Polymer/Water

Protective Casing 29.5 ft., 4 in. dia. Steel

**Construction Time log:**

Task	Date	Start		Finish	
		Date	Time	Date	Time
Drilling	8/13/95		12:45	8/8/95	11:30
Casing:					
S1	12/15/95		14:33	12/15/95	14:35
C1	12/15/95		14:35	12/15/95	14:50
Filter Placement:	12/15/95		14:50	12/15/95	15:30
Cementing:	12/18/95		15:22	12/18/95	15:30
Development:					

**Well Design & Specifications**

Basis: Geologic Log X Geophysical Log  
 Casing string(s): C = Casing S = Screen

Depth	String(s)	Elevation
+ 0.80 - 155.00	C1	6062.05 - 5908.25
155.00 - 175.00	S1	5908.25 - 5888.25
-	-	-
-	-	-
-	-	-
-	-	-
-	-	-

Casing: C1 1.5 in. Schedule 80, Flush Threaded PVC

Screen: S1 1.5 in. Schedule 80, Flush Threaded  
 0.020 Slot PVC

Filter Pack: 10-12 Silica Sand (145.0-177.0 ft.)

Grout Seal: Portland Type I-II Cement Slurry with 5%  
 Quickgel Bentonite Powder (0.0-10.0 ft.)

Backfill: 3/4 in. Gravel (10.0-130.0 and  
 192.0-325.2 ft.)

Bentonite Seal: Wyoming Grade 3/8 in. Bentonite  
 Holeplug Bentonite Chips  
 (130.0-145.0 and 177.0-192.0 ft.)

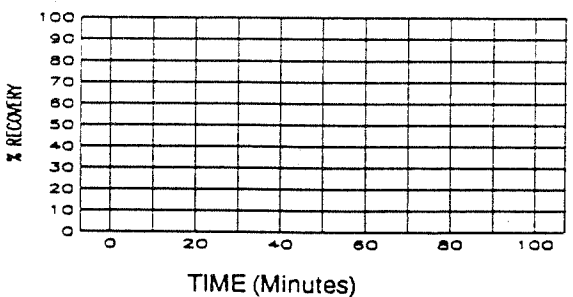
**Well Development**

Water Level on 12/18/95 = 155.01 FBTC

**Stabilization Test Data:**

Time	pH	Spec. Cond.	Temp (°C)

**Recovery Data:**



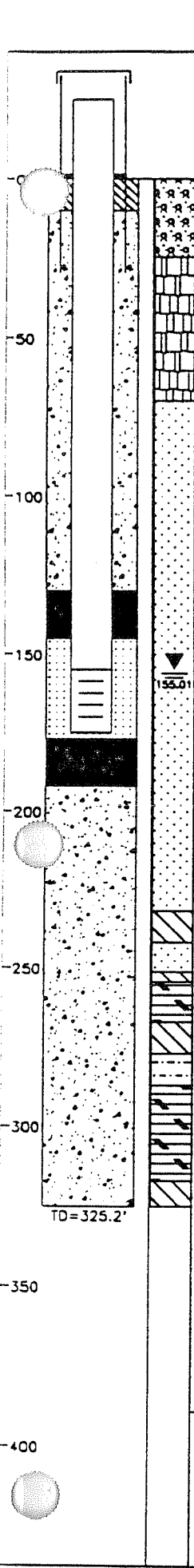
Comments:

\_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

Not to Horiz. Scale

Supervised by B. Johnson  
 Job Number 943-27891.140

Site Miner Flat Dam  
 File Name MF-254



### PIEZOMETER CONSTRUCTION SUMMARY

Survey Coords: Northing: 1085006.50  
 Easting: 576834.81

Elevation Ground Level 6107.52 ft.  
 Top of PVC Casing 6108.42 ft.

**Drilling Summary:**

Total Depth 362.0 ft.  
 Borehole Diameter 3.65 in.  
 Casing Stickup Height 0.90 ft.  
 Driller B. Mathews

Rig Dresser DBS-25  
 Bit(s) RSG 3.65 in. Core Bit

Drilling Fluid Polymer/Water

Protective Casing 6 ft. -2 in., 4 in. dia. Steel

**Well Design & Specifications**

Basis: Geologic Log X Geophysical Log  
 Casing string(s): C = Casing S = Screen

Depth	String(s)	Elevation
+ 0.90 - 206.00	C1	6108.42 - 5901.52
206.00 - 226.00	S1	5901.52 - 5881.52
-	-	-
-	-	-
-	-	-
-	-	-
-	-	-

Casing: C1 1.5 in. Schedule 80, Flush Threaded PVC

Screen: S1 1.5 in. Schedule 80, Flush Threaded  
 0.020 Slot PVC

Filter Pack: 10-12 Silica Sand (196.0-228.0 ft.)

Grout Seal: Portland Type I-II Cement Slurry with 5%  
 Quickgel Bentonite Powder (0.0-2.0 ft.)

Backfill: 3/4 in. Gravel (2.0-178.0 and  
 243.0-362.0 ft.)

Bentonite Seal: Wyoming Grade 3/8 in. Bentonite  
 Holeplug Bentonite Chips  
 (178.0-196.0 and 228.0-243.0 ft.)

**Construction Time log:**

Task	Start		Finish	
	Date	Time	Date	Time
Drilling	8/21/95	14:00	8/28/95	17:30
Casing:				
S1	12/18/95	9:52	12/18/95	9:55
C1	12/18/95	9:55	12/18/95	10:28
Filter Placement:	12/18/95	10:28	12/18/95	10:55
Cementing:	12/18/95	18:04	12/18/95	18:08
Development:				

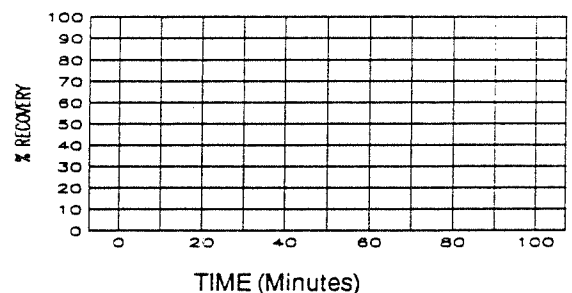
**Well Development**

Water Level on 12/18/95 = 149.95 FBTC

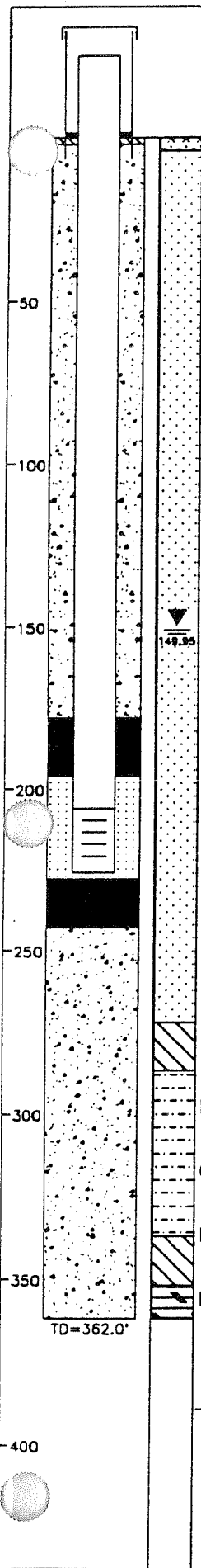
**Stabilization Test Data:**

Time	pH	Spec. Cond.	Temp (°C)

**Recovery Data:**



Comments: Used coupling (PVC) at 130.0 ft.; glued with Uni-Weld 1200 PVC Cement/ACE 44393  
 PVC Pipe Cleaner. Upper backfilling took only ~18 gallons of gravel; may have bridged.



Not to Horiz. Scale

Supervised by B. Johnson  
 Job Number 943-27691.140

Site Miner Flat Dam  
 File Name MF-218A

Well No. MF-250

Boring No. X-Ref: MF-250

### PIEZOMETER CONSTRUCTION SUMMARY

Survey Coords: Northing: 1085625.57  
Easting: 576826.38

Elevation Ground Level 6079.16 ft.  
Top of PVC Casing 6080.06 ft.

#### Drilling Summary:

Total Depth 331.3 ft.  
Borehole Diameter 3.65 in.  
Casing Stickup Height 0.90 ft.  
Driller B. Mathews

Rig Dresser DBS-25  
Bit(s) RSG 3.65 in. Core Bit

Drilling Fluid Polymer/Water

Protective Casing 15 ft., 4 in. dia. Steel

#### Well Design & Specifications

Basis: Geologic Log  Geophysical Log   
Casing string(s): C = Casing S = Screen

Depth	String(s)	Elevation
+ 0.90 - 148.00	C1	6080.06 - 5931.16
148.00 - 168.00	S1	5931.16 - 5911.16
-	-	-
-	-	-
-	-	-
-	-	-
-	-	-

Casing: C1 1.5 in. Schedule 80, Flush Threaded PVC

Screen: S1 1.5 in. Schedule 80, Flush Threaded 0.020 Slot PVC

Filter Pack: 10-12 Silica Sand (148.0-180.0 ft.)

Grout Seal: Portland Type I-II Cement Slurry with 5% Quickgel Bentonite Powder (0.0-10.0 ft.)

Backfill: 3/4 in. Gravel (10.0-133.0 and 190.0-331.3 ft.)

Bentonite Seal: Wyoming Grade 3/8 in. Bentonite Holeplug Bentonite Chips (133.0-148.0 and 180.0-190.0 ft.)

Comments: \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

#### Construction Time log:

Task	Start		Finish	
	Date	Time	Date	Time
Drilling	6/2/95	7:00	6/19/95	12:45
Casing:				
S1	12/16/95	9:50	12/16/95	9:58
C1	12/16/95	9:58	12/16/95	10:09
Filter Placement:	12/16/95	10:15	12/16/95	11:30
Cementing:	12/18/95	16:50	12/18/95	17:00
Development:				

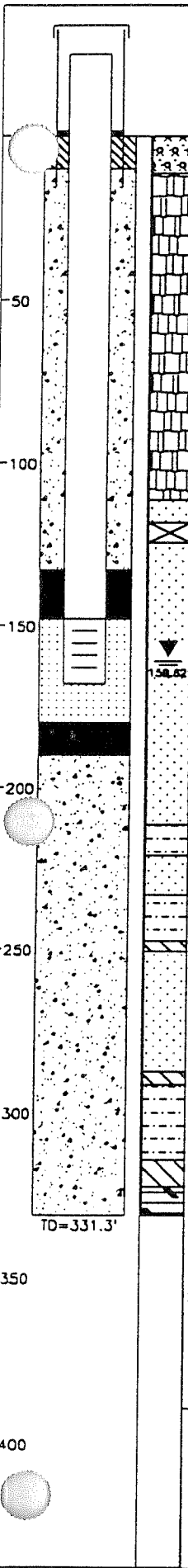
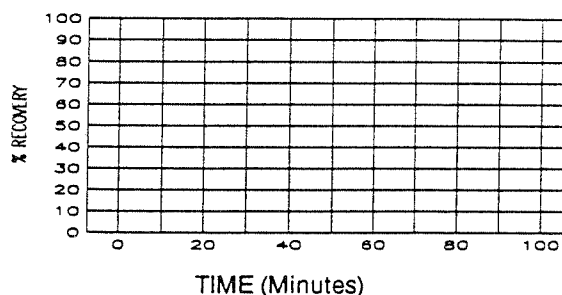
#### Well Development

Water Level on 12/18/95 = 159.62 FBTC

#### Stabilization Test Data:

Time	pH	Spec. Cond.	Temp (°C)

#### Recovery Data:



Not to Horiz. Scale

Supervised by B. Johnson  
Job Number 943-27691.140

Site Miner Flat Dam  
File Name MF-250

### PIEZOMETER CONSTRUCTION SUMMARY

Survey Coords: Northing: 1085422.42  
 Easting: 576870.86

Elevation Ground Level 6088.96 ft.  
 Top of PVC Casing 6089.96 ft.

#### Drilling Summary:

Total Depth 340.7 ft.  
 Borehole Diameter 3.65 in.  
 Casing Stickup Height 1.00 ft.  
 Driller B. Mathews  
 Rig Dresser DBS-25  
 Bit(s) RSG 3.65 in. Core Bit  
 Drilling Fluid Polymer/Water  
 Protective Casing 30 ft., 4 in. dia. Steel

#### Construction Time log:

Task	Date	Start		Finish	
		Date	Time	Date	Time
Drilling	6/2/95		17:57	7/23/95	17:30
Casing:					
S1	12/16/95		14:15	12/16/95	14:17
C1	12/16/95		14:17	12/16/95	14:37
Filter Placement:	12/16/95		14:37	12/16/95	14:50
Cementing:	12/18/95		17:05	12/18/95	17:15
Development:					

#### Well Design & Specifications

Basis: Geologic Log X Geophysical Log  
 Casing string(s): C = Casing S = Screen

Depth	String(s)	Elevation
+ 1.00 - 169.00	C1	6089.96 - 5919.96
169.00 - 189.00	S1	5919.96 - 5899.96
-	-	-
-	-	-
-	-	-
-	-	-
-	-	-

Casing: C1 1.5 in. Schedule 80, Flush Threaded PVC

Screen: S1 1.5 in. Schedule 80, Flush Threaded  
 0.020 Slot PVC

Filter Pack: 10-12 Silica Sand (159.0-191.0 ft.)

Grout Seal: Portland Type I-II Cement Slurry with 5%  
 Quickgel Bentonite Powder (0.0-10.0 ft.)

Backfill: 3/4 in. Gravel (10.0-144.0 and  
 205.0-340.7 ft.)

Bentonite Seal: Wyoming Grade 3/8 in. Bentonite  
 Holeplug Bentonite Chips  
 (144.0-159.0 and 191.0-205.0 ft.)

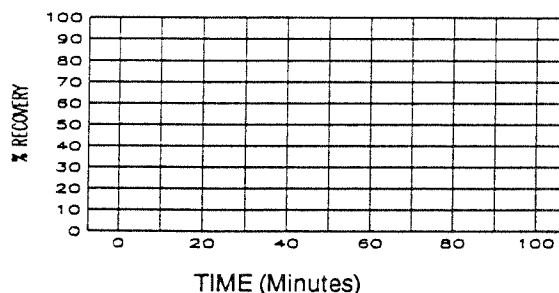
#### Well Development

Water Level on 12/18/95 = 170.84 FBTC

#### Stabilization Test Data:

Time	pH	Spec. Cond.	Temp (°C)

#### Recovery Data:



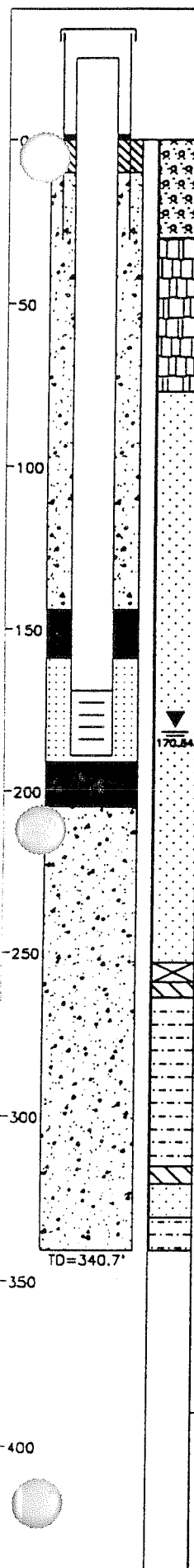
#### Comments:

\_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

Not to Horiz. Scale

Supervised by B. Johnson  
 Job Number 943-27691.140

Site Miner Flat Dam  
 File Name MF-251





Well No. MF-252

Boring No. X-Ref: MF-252

### PIEZOMETER CONSTRUCTION SUMMARY

Survey Coords: Northing: 1085229.89  
Easting: 576821.43

Elevation Ground Level 6074.16 ft.  
Top of PVC Casing 6074.91 ft.

#### Drilling Summary:

Total Depth 332.7 ft.  
Borehole Diameter 3.65 in.  
Casing Stickup Height 0.75 ft.  
Driller B. Mathews

Rig Dresser DBS-25  
Bit(s) RSG 3.65 in. Core Bit

Drilling Fluid Polymer/Water

Protective Casing 20 ft., 4 in. dia. Steel

#### Construction Time log:

Task	Start		Finish	
	Date	Time	Date	Time
Drilling	8/15/95	7:30	8/2/95	11:30
Casing:				
S1	12/17/95	14:42	12/17/95	14:45
C1	12/17/95	14:45	12/17/95	15:30
Filter Placement:	12/17/95	16:00	12/17/95	16:30
Cementing:	12/18/95	16:32	12/18/95	16:40
Development:				

#### Well Design & Specifications

Basis: Geologic Log X Geophysical Log  
Casing string(s): C = Casing S = Screen

Depth	String(s)	Elevation
+ 0.75 - 160.00	C1	6074.91 - 5914.16
160.00 - 180.00	S1	5914.16 - 5894.16
-	-	-
-	-	-
-	-	-
-	-	-

Casing: C1 1.5 in. Schedule 80, Flush Threaded PVC

Screen: S1 1.5 in. Schedule 80, Flush Threaded 0.020 Slot PVC

Filter Pack: 10-12 Silica Sand (148.0-182.0 ft.)

Grout Seal: Portland Type I-II Cement Slurry with 5% Quickgel Bentonite Powder (0.0-10.0 ft.)

Backfill: 3/4 in. Gravel (10.0-130.0 and 197.0-332.7 ft.)

Bentonite Seal: Wyoming Grade 3/8 in. Bentonite Holeplug Bentonite Chips (130.0-148.0 and 182.0-197.0 ft.)

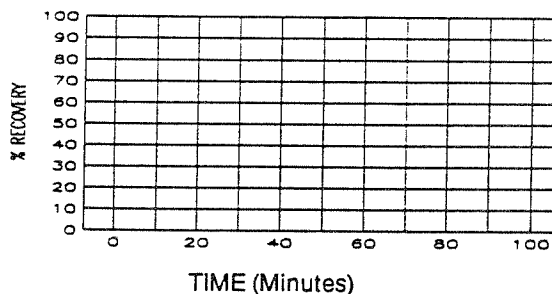
#### Well Development

Water Level on 12/18/95 = 161.11 FBTC

#### Stabilization Test Data:

Time	pH	Spec. Cond.	Temp (°C)

#### Recovery Data:

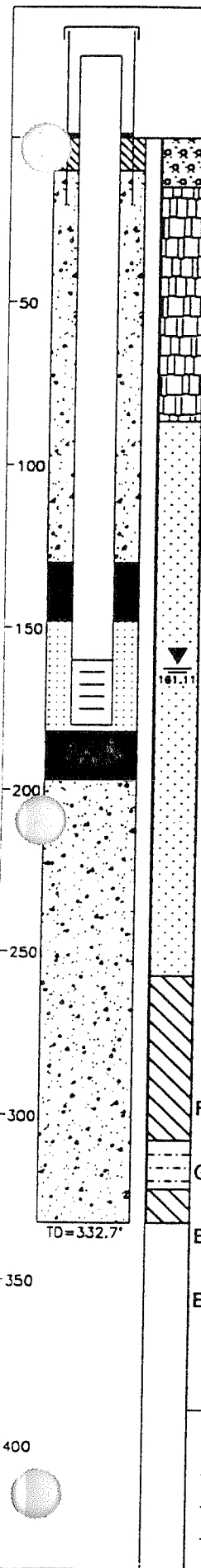


Comments: Some caving/bridging near top of upper bentonite seal, added additional 1/2 bag to ensure at least 15 ft. of seal; used coupling (galvanized steel) at 150.0 ft.

Not to Horiz. Scale

Supervised by B. Johnson  
Job Number 943-27691.140

Site Miner Flat Dam  
File Name MF-252



### PIEZOMETER CONSTRUCTION SUMMARY

Survey Coords: Northing: 1084797.33  
 Easting: 576874.74

Elevation Ground Level 6071.41 ft.  
 Top of PVC Casing 6072.61 ft.

#### Drilling Summary:

Total Depth 327.2 ft.  
 Borehole Diameter 3.65 in.  
 Casing Stickup Height 1.20 ft.  
 Driller B. Mathews  
 Rig Dresser DBS-25  
 Bit(s) RSG 3.65 in. Core Bit  
 Drilling Fluid Polymer/Water  
 Protective Casing 35 ft., 4 in. dia. Steel

#### Construction Time log:

Task	Start		Finish	
	Date	Time	Date	Time
Drilling	8/3/95	14:42	8/21/95	13:50
Casing:				
S1	12/15/95	8:56	12/15/95	9:00
C1	12/15/95	9:00	12/15/95	9:08
Filter Placement:	12/15/95	9:10	12/15/95	11:30
Cementing:	12/18/95	15:45	12/18/95	15:50
Development:				

#### Well Design & Specifications

Basis: Geologic Log X Geophysical Log  
 Casing string(s): C = Casing S = Screen

Depth	String(s)	Elevation
+ 1.20 - 177.00	C1	6072.61 - 5894.41
177.00 - 197.00	S1	5894.41 - 5874.41
-	-	-
-	-	-
-	-	-
-	-	-

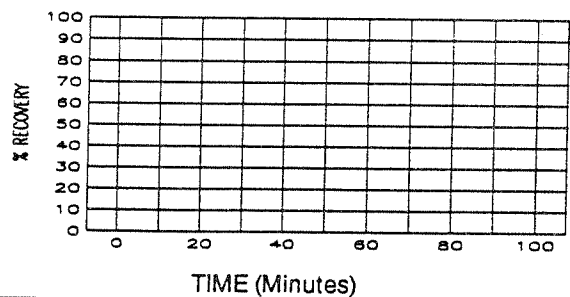
#### Well Development

Water Level on 12/18/95 = 171.48 FBTC

#### Stabilization Test Data:

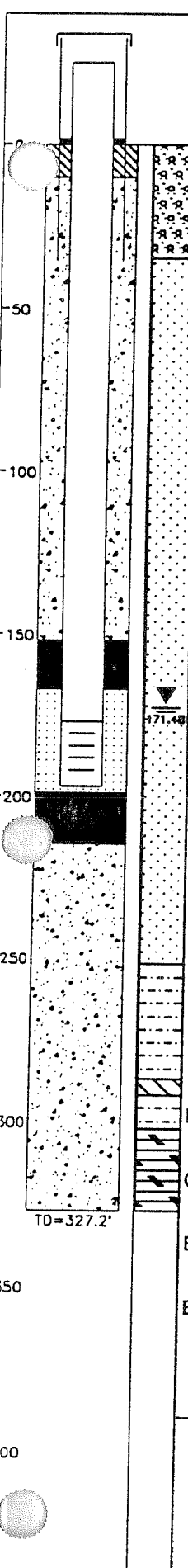
Time	pH	Spec. Cond.	Temp (°C)

#### Recovery Data:



Casing: C1 1.5 in. Schedule 80, Flush Threaded PVC  
 Screen: S1 1.5 in. Schedule 80, Flush Threaded 0.020 Slot PVC  
 Filter Pack: 10-12 Silica Sand (167.0-199.0 ft.)  
 Grout Seal: Portland Type I-II Cement Slurry with 5% Quickgel Bentonite Powder (0.0-10.0 ft.)  
 Backfill: 3/4 in. Gravel (10.0-152.0 and 215.0-327.2 ft.)  
 Bentonite Seal: Wyoming Grade 3/8 in. Bentonite Holeplug Bentonite Chips (152.0-167.0 and 199.0-215.0 ft.)

Comments:



Not to Horiz. Scale

Supervised by B. Johnson  
 Job Number 943-27691.140

Site Miner Flat Dam  
 File Name MF-253

Well No. MF-254

Boring No. X-Ref: MF-254

### PIEZOMETER CONSTRUCTION SUMMARY

Survey Coords: Northing: 1084655.39  
Easting: 577063.74

Elevation Ground Level 6061.25 ft.  
Top of PVC Casing 6062.05 ft.

#### Drilling Summary:

Total Depth 325.2 ft.  
Borehole Diameter 3.65 in.  
Casing Stickup Height 0.80 ft.  
Driller B. Mathews

Rig Dresser DBS-25  
Bit(s) RSG 3.65 in. Core Bit

Drilling Fluid Polymer/Water

Protective Casing 29.5 ft., 4 in. dia. Steel

#### Construction Time log:

Task	Date	Start		Finish	
		Date	Time	Date	Time
Drilling	8/13/95		12:45	8/8/95	11:30
Casing:					
S1	12/15/95		14:33	12/15/95	14:35
C1	12/15/95		14:35	12/15/95	14:50
Filter Placement:	12/15/95		14:50	12/15/95	15:30
Cementing:	12/18/95		15:22	12/18/95	15:30
Development:					

#### Well Design & Specifications

Basis: Geologic Log  Geophysical Log  
Casing string(s): C = Casing S = Screen

Depth	String(s)	Elevation
+ 0.80 - 155.00	C1	6062.05 - 5906.25
155.00 - 175.00	S1	5906.25 - 5886.25
-	-	-
-	-	-
-	-	-
-	-	-

Casing: C1 1.5 in. Schedule 80, Flush Threaded PVC

Screen: S1 1.5 in. Schedule 80, Flush Threaded 0.020 Slot PVC

Filter Pack: 10-12 Silica Sand (145.0-177.0 ft.)

Grout Seal: Portland Type I-II Cement Slurry with 5% Quickgel Bentonite Powder (0.0-10.0 ft.)

Backfill: 3/4 in. Gravel (10.0-130.0 and 192.0-325.2 ft.)

Bentonite Seal: Wyoming Grade 3/8 in. Bentonite Holeplug Bentonite Chips (130.0-145.0 and 177.0-192.0 ft.)

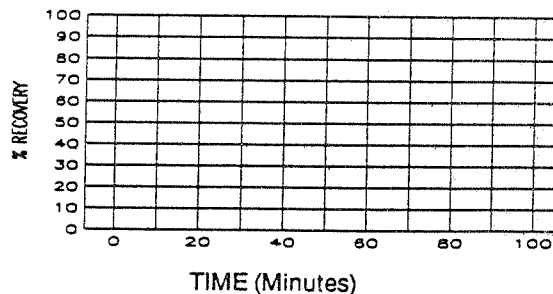
#### Well Development

Water Level on 12/18/95 = 155.01 FBTC

#### Stabilization Test Data:

Time	pH	Spec. Cond.	Temp (°C)

#### Recovery Data:



#### Comments:

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\_\_\_\_\_  
\_\_\_\_\_

Not to Horiz. Scale

Supervised by B. Johnson  
Job Number 943-27691.140

Site Miner Flat Dam  
File Name MF-254

